



1021-2011 ADDENDUM 1

STURGEON ROAD BRIDGE REPLACEMENT

URGENT

**PLEASE FORWARD THIS DOCUMENT TO
WHOEVER IS IN POSSESSION OF THE BID
OPPORTUNITY**

ISSUED: January 20, 2012
BY: Kevin Amy, M.Sc., P.Eng.
TELEPHONE NO. (204) 488-5743

**THIS ADDENDUM SHALL BE INCORPORATED
INTO THE BID OPPORTUNITY AND SHALL
FORM A PART OF THE CONTRACT
DOCUMENTS**

Template Version: A20070419

Please note the following and attached changes, corrections, additions, deletions, information and/or instructions in connection with the Bid Opportunity, and be governed accordingly. Failure to acknowledge receipt of this Addendum in Paragraph 10 of Form A: Bid may render your Bid non-responsive.

PART A – BID SUBMISSION

Replace: 1021-2011_Bid_Submission with 1021-2011_Addendum_1-Bid_Submission. The following is a summary of changes incorporated in the replacement Bid Submission:

Form B(R1): Numerous changes.

Page numbering on some forms may be changed as a result.

Replace: 1021-2011_Form_B-Excel with 1021-2011_Addendum_1-Form_B-Excel

PART B – BIDDING PROCEDURES

Revise: B2.1 to read: The Submission Deadline is 12:00 noon Winnipeg time, February 8, 2012.

PART D – SUPPLEMENTAL CONDITIONS

Revise: D2.2(d) to read: Construction of two (2) new southbound lanes of concrete pavement from Portage Avenue to Hallonquist Drive (2013)

Revise: D15.4 to read: The City intends to award this Contract by March 30, 2011

Revise: D20.1 to read: The Contractor shall achieve Substantial Performance by August 27, 2013.

Revise: D21.1 to read: The Contractor shall achieve Total Performance by September 30, 2013.

PART E – SPECIFICATIONS

Revise: E4.7.1(b) to read: 60% during construction.

Revise: E12.1(a) to read: This Specification shall cover all operations related to excavation for the abutments, approach slabs and lightweight cellular concrete.

Revise: E12.4.1(a) to read: Excavation: The excavation of material to a depth as shown on the Drawings for the abutments, approach slab and lightweight cellular concrete.

Add: E13.5.5(b) All clay borrow and suitable site materials placed on the cellular concrete fill shall be supplied in accordance with CW 3170-R3.

Revise: E14.4.5 to read: Abutment Pile Casing
(a) The abutment pile casing as detailed on the Drawings, are to 750mm diameter, ArmTec BOSS 2000 HDPE pipe with a stiffness of 260 kPa or approved equivalent in accordance with section B6.

Add: E14.6.1(b) All items of work associated with dynamic testing of steel piles are considered incidental to Driving of Steel Piles.

Revise: E15.4.11(a) to read: The top surfaces of box girders shall be finished to produce even indentations at right angles to the longitudinal centreline of the girders. The indentations shall be 3 to 5 mm, full amplitude, and spaced not greater than 15 mm apart.

Add: E15.4.11(i) The concrete surfaces of the continuous shear keys shall be rough, clean, and free of laitance with a full amplitude of approximately 6 mm. The method for roughening and cleaning the above surfaces shall be approved by the Contract Administrator.

Revise: E17.5.3 to read: Concrete
(a) Concrete materials susceptible to frost damage shall be protected from freezing.
(b) Concrete shall have nominal compressive strengths (f'c) and meet the requirements for hardened concrete as specified in the following Table 17.1.

TABLE 17.1 REQUIREMENTS FOR HARDENED CONCRETE							
Type of Concrete	Location	Nominal Compressive Strength MPa	Class of Exposure	Air Content Category	Max Aggregate Size	Special Requirements	Minimum Post Residual Cracking Index
Type 1	Caissons	40 @ 28 Days	C-1	1	20 mm	-	-
Type 2	Pier Caps	40 @ 28 Days	C-1	1	20 mm	Synthetic Fibers	0.15
Type 3	Structural Deck, Abutments, Approach Slabs, Reinforced Road Slab	45 @ 28 Days	C-1	1	20 mm	Synthetic Fibers	0.15
Type 4	Sidewalk, Traffic Barriers,	35 @ 28 Days	C-1	1	20 mm	Synthetic Fibers	0.15
Type 6	Leveling Pad, Coping	35 @ 28 Days	C-1	1	20 mm	-	-

(c) Type 6 concrete is incidental to "MSE Precast Concrete Panels".

Add: E17.5.31 Void Form

(a) Void form shall be placed as indicated on the Drawings and beneath the abutment bearing seats and wingwalls. The Contractor shall be responsible for the maintenance of the void form throughout construction and replace any damaged void form as required by the Contract Administrator.

Add: E17.6.3(b) Refer to section E17.7.13(d) for structural deck finish requirements.

Revise: E17.6.5(a) to read: Screed to Approach Slab Concrete, refer to section E17.6.3(b).

Delete: E17.6.5(b)

Revise: E17.6.5(c)(i) The Contractor shall provide a movable Work Bridge, spanning the approach slab at right angles to the centreline of roadway in order to facilitate the approach slab finish as required by section E17.7.13(d), the application of curing compound, the inspection of the freshly-placed concrete, and any remedial Work required to be done to the screeded surface, including filling in any holes left by the screed bars. After the surface has been screeded, all further Work that may be required shall be done from the Work Bridge.

Delete: E17.7.11

Add: E17.7.13(g) Type 5 Finish – Deck, Approach Slab and Structural Roadway Slab Finish for Traffic Barriers and Sidewalks

(a) The concrete finish of the of the deck, approach slab and structural roadway slab on which the sidewalks and traffic barriers are to be placed shall be broom finished or have a roughened amplitude of 3mm (maximum).

Revise: E17.9.2 to read: Moveable Deck Hoarding
Supplying, setting up, operating, and removing of the moveable deck hoarding will not be measured and will be paid for at the Contract Lump Sum Price for “Supply and Install Moveable Hoarding for Deck Concrete”, which price shall be payment in full for performing all operations herein described and all other items incidental to the Work included in this Specification and accepted by the Contract Administrator.

Delete: E18.7.3

Delete: E18.8

Delete: E18.8.2

Revise: E24.3.1 to read: The locations of the reinforced silt fence barriers shall be such that the requirements of section E3 and the regulatory requirements are satisfied. The final locations of the silt fence barriers will be dependent on site conditions, the Contractor’s activities and methods of construction and on direction of the Contract Administrator.

Revise: E25 to read: **MODULAR BLOCK RETAINING WALL AND MSE RETAINING WALL**

Revise: E25.2.1(b) to read: Design, supply and installation of all materials required for the MSE Retaining Wall.

Add: E25.2.1(b)(ii) The material limits pertaining to the MSE retaining wall shall be the precast panels, and miscellaneous related material items, the steel reinforcing strips, concrete coping and concrete leveling pad.

The design, completed by the Contractor, of the MSE Retaining Wall shall utilize the fill materials as shown on the Drawings.

All excavated materials and all fill materials in front of and behind the MSE retaining wall shall be as shown on the Drawings and shall be referenced to, and measured and paid by, their applicable specification section.

Revise: E25.2.2 to read: Contractor shall design and implement the required connections or details between the Precast Concrete Panels and the temporary grade separation wall to facilitate the phasing and construction of the Work as described, measured and paid in E11 “Temporary Grade Separation Wall”.

Add: E25.2.3 The retaining walls shall be designed to support the maintenance vehicle from CSA-S6-06, Figure 3.4.

The design loading on the active transportation trail for the purpose of the design of the modular block wall is 2.5kPa.

- Add: E25.3.5 MSE Precast Concrete Panels
- (a) The fabrication of the precast concrete panels shall conform to the requirements for E15 "Precast Concrete Girders". Precast concrete panel concrete strength to be determined by the Contractor.
 - (b) The exposed surfaces of the precast concrete panels shall have a surface finish of "Fractured Fin" as per Atlantic Industries Limited or approved equivalent as per section B6.
- Add: E25.3.6 Cast-in-Place Concrete Levelling Pad
- (a) The concrete levelling pad shall be in accordance with E17 "Structural Concrete".
- Add: E25.3.7 Cast-in-Place Concrete Coping
- (a) The concrete coping shall be in accordance with E17 "Structural Concrete".
- Add: E25.3.8 Steel Reinforcing Strips
- (a) Steel reinforcing strips to be hot dip galvanized in accordance with CSA G164.
- Revise: E25.4.1 to read: Contractor to submit shop drawings and calculations, sealed by a Professional Engineer Registered in Manitoba experienced in the design of modular block retaining wall and precast concrete panel wall, to Contract Administrator for review no later than twenty (20) days prior to wall installation. Details, as applicable, to include:
- (a) Plan of entire length of wall;
 - (b) Elevation of wall indicating top and bottom of wall elevations;
 - (c) Sections through walls;
 - (d) All vertical and horizontal break points and running dimensions to break points;
 - (e) Designation of type and size of blocks including caps;
 - (f) Designation of type and size of panels including cast-in-place coping;
 - (g) Limits and extent of reinforced fill volume;
 - (h) Length, size, type and elevation of every layer of geogrid reinforcement and/or steel reinforcing strips;
 - (i) Running dimension to changes in length of the geogrid reinforcement and/or steel reinforcing strips;
 - (j) The original and final ground elevations;
 - (k) Location of drain lines within geogrid reinforcement and/or steel reinforcing strips;
 - (l) Without written consent from the Contract Administrator the modular block wall and precast concrete panel material shall be as noted in this Specification and on the Drawings. All other materials, type of materials, dimensions of materials and configurations shall be confirmed and determined by the Contractor. This information shown on the Drawings was determined through a preliminary design and shall not be considered final.

- (m) The Geotechnical Report will be supplied to Contractor such that the modular block wall design can be completed.
- (n) The top of wall elevations are to be as shown on the Drawings unless approved, in writing, by the Contract Administrator.
- (o) Construction methods required for the construction of the modular block wall and/or the precast concrete panels.
- (p) General notes required for construction.

Samples of modular concrete blocks and soil reinforcing materials to be submitted twenty (20) days prior to wall installation.

- Revise: E25.5(f) to read: Tracked construction equipment shall not be operated directly upon geogrid reinforcing and/or steel reinforcing strips on within 1 m of concrete units. Minimum fill thickness of 150 mm is required prior to operation of tracked vehicles over geogrid and/or steel reinforcing strips. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing fill and damaging the geogrid and/or steel reinforcing strips.
- Revise: E25.5(g) to read: Any damage to the geogrid and/or steel reinforcing strips reinforcing or other components of the wall caused the Contractor shall be repaired at the Contractor's expense.
- Add: E25.6.2 Supplying and placing the MSE retaining wall will not be measured and will be paid for at the Contract Lump Sum Price for "MSE Precast Concrete Panels", which price shall be payment in full for performing all operations herein described and all other items, incidental to the Work included in this Specification and accepted by the Contract Administrator.
- Revise: E26.4.4(a) to read: 914mm outside diameter x 9.5 mm thick permanent casing as indicated on the Drawings, conforming to ASTM A252 Grade 3. Steel casings shall be hot dip galvanized to the limits shown on the Drawings.
- Revise: E26.4.4(b) to read: The steel casing shall be seamless or spirally welded.
- Revise: E26.6.3 to read: Dewatering
- (a) Any water present within the caisson holes shall be pumped out and removed from site.
 - (b) The caisson hole shall be safe and dewatered to facilitate inspection by the Contract Administrator.
 - (c) If the caisson hole cannot be dewatered using reasonable effort, as accepted by the Contract Administrator, the Contractor shall supply and operate a down hole camera to facilitate the inspection of the caisson hole by the Contract Administrator.
 - (d) All costs associated with the dewatering shall be incidental to the appropriate Contract Lump Sum Price for "Supply and Install Rock-Socketed Caissons".
- Revise: E27.1.1 to read: This Specification covers the supply, installation, and maintenance of erosion control blanket to be installed on areas disturbed during construction, seeded areas and as directed by the Contract Administrator.
- Revise: E27.4.1 to read: The Contractor shall supply all ECB materials required and store them on site. The installation and maintenance of all ECB will be as directed by the Contract Administrator.

Revise: E45.4.2 to read: The shear key excavation shall be excavated to the depths and widths, and in the locations shown on the Drawings. An adequate volume of rockfill for backfilling shall be on-site prior to excavation of each incremental length of the trench shear key. The excavation shall proceed in a timely manner and rockfill must be placed as soon as excavation takes place, as practical. Stockpiling of excavated material on the riverbank will not be permitted, and shall be removed immediately upon excavation. The maximum open length of the shear key shall be 2 metres along the bottom of the excavation. The Contractor shall be required to prevent surface water from entering the excavation.

In no case shall the shear key be left at the end of the day. The shear key must be filled with rockfill at the end of each day.

After placement of the rockfill to the required dimensions shown on the Drawings, the impervious clay cap shall be placed in layers not exceeding 200 millimetres, and compacted to a minimum of 95% of the Standard Proctor maximum dry density. The clay cap shall be located within undisturbed native material surrounding the trench. Care shall be taken to ensure that an effective seal results between the wall of the excavation and the clay material placed to protect against water infiltration into the trench, as approved by the Contract Administrator.

Water infiltration and bottom blow out may occur as the bottom of the trench approaches the till. The top elevation of the trench excavation shall be located to minimize the risk of river water entering the excavation. Discharge of water contained within the trench excavation from displacement of the rockfill during backfill will be acceptable. The Contractor shall be responsible to contain and direct any displaced water such that it will not affect other construction work or cause excessive erosion of the native riverbank soils. The control of the water shall be the responsibility of the Contractor and shall be considered incidental work.

Excavation material shall consist of clay, silt, sand, cobbles, boulders, organic material, concrete rubble, and all other materials that may be reasonably excavated using conventional excavation equipment. During the excavation in the vicinity of the existing bridge abutments, it will be necessary to completely remove the abutment piles prior to the placement of rockfill. The existing pile removal shall be considered incidental to the shear key excavation and no separate measurement or payment shall be made.

As outlined within other areas of this Specification, the existing bridge removal and new bridge construction will be performed in two Phases. Upon the Phase 1 removal of the existing bridge, all rockfill trench shear key shown on the drawings located east of the west limit of Phase 1 construction shall be installed prior to approach slab fill placement or retaining segmental block wall fill placement as part of Sturgeon Road northbound lanes bridge replacement.

For the Phase 2 construction, all rockfill trench shear key construction at the limits shown on the drawings shall be completed prior to Sturgeon Road Southbound Lanes approach slab fill placement or any fill placement for segmental block retaining wall construction.

Delete: E46

Revise: E52.2(a) to read: The Contractor will investigate the area with the Contract Administrator to identify and mark required limits of work along riverbanks to retain and minimize damage of native grass areas and limits will remain marked throughout the construction period.

Add: E52.3.3 Grass Seed

The seed supplied will be free of disease and mixed by percentage (%) of weight to meet the following mixture:

Species	Latin name	Percentage by weight
big bluestem	<i>Andropogon gerardii</i>	30
Canada wild rye	<i>Elymus canadensis</i>	10
slough grass	<i>Beckmannia syzigachne</i>	5
slender wheatgrass	<i>Agropyron trachycaulum</i>	5
green needlegrass	<i>Stipa viridula</i>	20
switch grass	<i>Panicum virgatum</i>	10
fowl blue grass	<i>Poa palustris</i>	10
side-oats grama	<i>Bouteloua curtipendula</i>	10
		100

The mixture specified above will grow differently than regular turf mixture, therefore this vegetation should be rough cut at a height greater than 100 mm and only need to be mowed once or twice during the first growing season. Best seeding time is before July 1st. If it is after July 1st the site should be kept weed free and seeded in the fall. Spot spraying or rough cutting through one growing season is required to eliminate weeds such as Canada thistle.

Add: E53.

LIGHTWEIGHT CELLULAR CONCRETE

Add: E53.1

Description

Add: E53.1.1

The Work to be done by the Contractor under this Specification shall include the furnishing of all superintendence, overhead, labour, materials, equipment, tools, supplies, and all other things necessary or and incidental to the satisfactory performance and completion of all Work as hereinafter specified.

Add: E53.2

References:

- CAN/CSA A3001, Cementitious Materials for Use in Concrete
- CSA A23.1, Concrete Materials and Methods of Concrete Construction
- ASTM C 869, Standard Specification for Foaming Agents Used in Making Preformed Foam for Cellular Concrete
- ASTM C 796, Standard Test Method for Foaming Agents for Use in Producing Cellular Concrete Using Preformed Foam
- ASTM C 495-99a, Standard Test Method for Compressive Strength of Lightweight Insulating Concrete

Add: E53.3

Qualification:

Add: E53.3.1

The Contractor is to submit the qualifications of the Subcontractor that is to produce and place the cellular concrete for review and approval by the Contract Administrator.

Add: E53.3.2

The approved Subcontractor producing and placing cellular concrete shall have a record of experience and quality of work that is satisfactory to the Contract Administrator, and shall be capable of developing a mix design, batching, mixing, handling, and placing cellular concrete. The Subcontractor shall be certified by the manufacturer of the foaming agent and regularly engaged in the production and placement of cellular concrete. The Subcontractor shall have an adequate number of fully qualified workers who are thoroughly trained and experienced in the production and placement of cellular concrete.

Add: E53.4

Equipment

Add: E53.4.1

The specialized batching, mixing, and placing equipment shall be automated and certified for the purpose by the manufacturer of the cellular concrete material. Dry-mix equipment must be able to receive bulk cement and produce over 100 cubic metres per hour on-site,

continuously, from one piece of equipment, and pump through hoses or pipes up to a flat lineal distance of 1000 metres. Bulk cement shall be weighed on a scale that operates within a tolerance of one and one-half percent (1.5%) per batch. Wet-mix equipment must be able to receive slurry on-site into the equipment and process it continuously during ready-mix supply, and pump through hoses or pipes up to a flat lineal distance of 200 metres.

- Add: E53.4.2 Cellular concrete must be pumped by a positive displacement pump (Peristaltic or similar). A foam generator shall be used to continuously produce pre-formed foam, which shall be injected and mixed with the cementitious slurry downstream of the positive displacement slurry pump. The equipment shall be calibrated to produce a precise and predictable volumetric rate of foam with stable uniform microbubbles.
- Add: E53.5 Materials and Testing
- Add: E53.5.1 Cellular concrete shall be CEMATRIX CMEF-475 lightweight engineered fill with the following properties:
- (a) Minimum unconfined compressive strength at 28 days of 0.5 MPa.
 - (b) Wet cast density of 475 kg/m³ (+/-10%).
- Add: E53.5.2 Portland cement shall conform to the requirements of CSA Standard CAN/CSA A3001, Type GU or HE. Supplementary cementing materials shall conform to the requirements of CSA Standard CAN/CSA A3001.
- Add: E53.5.3 Mixing water shall conform to the requirements of CSA Standard A23.1. Water of questionable quality shall not be used unless proven to produce specimens whose 28-day compressive strength is at least 90 % of those made with known acceptable water and an identical material mix.
- Add: E53.5.4 Foaming agents shall conform to the requirements of ASTM C 869 when tested in accordance with the provisions of ASTM C 796. CEMATRIX CF-1 or PROVOTON foaming agents shall be used. The Subcontractor shall be pre-qualified and approved in writing by the foaming agent manufacturer, referencing this Project. A copy of the written approval is to be submitted to the Contract Administrator prior to the commencement of the work.
- Add: E53.5.5 The fresh cellular concrete density shall be measured and recorded once per production run, or once for every 50 cubic metres, or once per 30 minutes, whichever is more frequent. The density shall be maintained within +/- 10 % of the design density.
- Add: E53.5.6 Cellular concrete samples must be captured, cured, and tested to verify the compressive strength requirement is satisfied. One sample is comprised of one set of six cellular concrete cylinders. One sample should be taken for each placement, or every 100m³, whichever is more frequent. Cylinders are cast in 75mm by 150mm cylindrical plastic molds. The sample mold must be lined with "freezer paper" with the plastic side against the cellular concrete. Cellular concrete cylinders shall be cured and tested as per ASTM C495-99a, modified to represent the field curing conditions for geotechnical applications.
- Add: E53.6 Subgrade Conditions and Site Preparation
- Add: E53.6.1 The sub-grade shall be cleared of vegetation, soft, wet, muddy, loose soil and other deleterious material, and graded and compacted to the lines and grades shown on the relevant drawings. The prepared subgrade shall be good competent level ground with nominal compaction to provide a firm base. The placement area shall be free of standing water during placement of cellular concrete and until backfill is placed on top of the cellular concrete. Snow and ice must be removed from the area prior to placement.
- Add: E53.7 Installation

- Add: E53.7.1 The Quality Control & Quality Assurance Manual Cematrix Cellular Concrete, Document Number: QCS-007, Last Updated: September 29, 2011 shall apply to the work.
- Add: E53.7.2 Any items to be fully or partially encased in the cellular concrete shall be properly set and stable prior to the installation of the cellular concrete.
- Add: E53.7.3 Where required, formwork should be designed and installed to withhold cellular concrete, and may require lining with poly sheeting or similar impermeable membrane to prevent leakage.
- Add: E53.7.4 Cellular concrete may be placed during freezing conditions, provided measures are taken to prevent damage to the cellular concrete until sufficient strength has been attained. Care should be taken to avoid freezing before initial set. Cellular concrete must not be placed during heavy or prolonged precipitation.
- Add: E53.7.5 Once mixed, the cellular concrete shall be conveyed promptly to the location of placement without excessive handling.
- Add: E53.7.6 The Constructor shall determine the maximum lift thickness based on density and any other considerations that may impact placement. Cellular concrete shall be cast in a formed area within 1 to 2 hours, to permit an undisturbed setting.
- Add: E53.7.7 Finished surface elevation shall be within ± 25 mm of the design grades shown on the drawings. Cellular Concrete can be placed with a maximum slope of 1%. Slopes greater than 1% will require profiling by creating steps for the Cellular Concrete with formwork.
- Add: E53.7.8 Loading of, or traffic on the cellular concrete shall be prevented until the material has attained sufficient strength to withstand the loads with no damage. Backfill can commence with cellular concrete supports foot traffic without leaving an indentation.
- Add: E53.8 Measurement and Payment
- Add: E53.8.1 Cellular concrete will be measured on a volume basis, as determined from the Drawings, and will be paid at the Contract Unit Price per cubic metre for "Cellular Concrete" which price shall be payment in full for performing all operations herein described and all other items incidental to the Work included in this Specification and accepted by the Contract Administrator.
- Add: E54 **RAKED ASPHALT**
- Add: E54.1 Description
- Add: E54.1.1 The installation of raked asphalt (raked asphalt pavement) shall be applied to the centre median(s) on Sturgeon Road as shown on the drawings or as directed by the Contract Administrator.
- Add: E54.2 Materials
- Add: E54.2.1 Asphaltic concrete shall be Type 1A asphalt in accordance with CW 3410.
- Add: E54.3 Construction Methods
- Add: E54.3.1 The contractor shall place a prime coat on the base-course prior to the application of raked asphalt median cover and ensure good coverage on the base surface with no over-splash on adjacent concrete work.
- Add: E54.3.2 The contractor shall place median cover (raked asphalt pavement) in the reconstructed medians as shown on the drawings or as directed by the Contract Administrator.
- Add: E54.3.3 Asphalt shall be placed on the centre medians and spread evenly by hand rakes to the back-of-curbs to a final finished surface depth of 50mm.

- Add: E54.3.4 The final finished surface shall be rough corrugations from the raking operation and shall be in a pre-set pattern as directed by the Contract Administrator.
- Add: E54.4 Measurement and Payment
- Add: E54.4.1 Raked Asphalt will be measured on a weight basis and will be paid for at the Contract Unit Price per tonne for "Raked Asphalt Pavement", which price shall be payment in full for performing all operations herein described and all other items incidental to the Work included in this Specification and accepted by the Contract Administrator.

DRAWINGS

- Replace: 1021-2011_Drawing_B120-12-005_Sht5-R0 with 1021-2011_Addendum_1-Drawing_B120-12-005_Sht5-R1
- 1021-2011_Drawing_B120-12-006_Sht6-R0 with 1021-2011_Addendum_1-Drawing_B120-12-006_Sht6-R1
- 1021-2011_Drawing_B120-12-007_Sht7-R0 with 1021-2011_Addendum_1-Drawing_B120-12-007_Sht7-R1
- 1021-2011_Drawing_B120-12-008_Sht8-R0 with 1021-2011_Addendum_1-Drawing_B120-12-008_Sht8-R1
- 1021-2011_Drawing_B120-12-012_Sht12-R0 with 1021-2011_Addendum_1-Drawing_B120-12-012_Sht12-R1
- 1021-2011_Drawing_B120-12-015_Sht15-R0 with 1021-2011_Addendum_1-Drawing_B120-12-015_Sht15-R1
- 1021-2011_Drawing_B120-12-016_Sht16-R0 with 1021-2011_Addendum_1-Drawing_B120-12-016_Sht16-R1
- 1021-2011_Drawing_B120-12-026_Sht26-R0 with 1021-2011_Addendum_1-Drawing_B120-12-026_Sht26-R1
- 1021-2011_Drawing_B120-12-027_Sht27-R0 with 1021-2011_Addendum_1-Drawing_B120-12-027_Sht27-R1
- 1021-2011_Drawing_B120-12-028_Sht28-R0 with 1021-2011_Addendum_1-Drawing_B120-12-028_Sht28-R1
- 1021-2011_Drawing_B120-12-029_Sht29-R0 with 1021-2011_Addendum_1-Drawing_B120-12-029_Sht29-R1
- 1021-2011_Drawing_B120-12-030_Sht30-R0 with 1021-2011_Addendum_1-Drawing_B120-12-030_Sht30-R1
- 1021-2011_Drawing_B120-12-040_Sht40-R0 with 1021-2011_Addendum_1-Drawing_B120-12-040_Sht40-R1
- 1021-2011_Drawing_B120-12-041_Sht41-R0 with 1021-2011_Addendum_1-Drawing_B120-12-041_Sht41-R1
- 1021-2011_Drawing_B120-12-042_Sht42-R0 with 1021-2011_Addendum_1-Drawing_B120-12-042_Sht42-R1
- 1021-2011_Drawing_B120-12-045_Sht45-R0 with 1021-2011_Addendum_1-Drawing_B120-12-045_Sht45-R1
- 1021-2011_Drawing_B120-12-047_Sht47-R0 with 1021-2011_Addendum_1-Drawing_B120-12-047_Sht47-R1
- 1021-2011_Drawing_B120-12-052_Sht52-R0 with 1021-2011_Addendum_1-Drawing_B120-12-052_Sht52-R1
- 1021-2011_Drawing_B120-12-057_Sht57-R0 with 1021-2011_Addendum_1-Drawing_B120-12-057_Sht57-R1
- 1021-2011_Drawing_B120-12-058_Sht58-R0 with 1021-2011_Addendum_1-Drawing_B120-12-058_Sht58-R1
- 1021-2011_Drawing_B120-12-059_Sht59-R0 with 1021-2011_Addendum_1-Drawing_B120-12-059_Sht59-R1
- 1021-2011_Drawing_B120-12-060_Sht60-R0 with 1021-2011_Addendum_1-Drawing_B120-12-060_Sht60-R1
- 1021-2011_Drawing_B120-12-062_Sht62-R0 with 1021-2011_Addendum_1-Drawing_B120-12-062_Sht62-R1
- 1021-2011_Drawing_P3331-20_Sht82-R0 with 1021-2011_Addendum_1-Drawing_P3331-20_Sht82-R1

