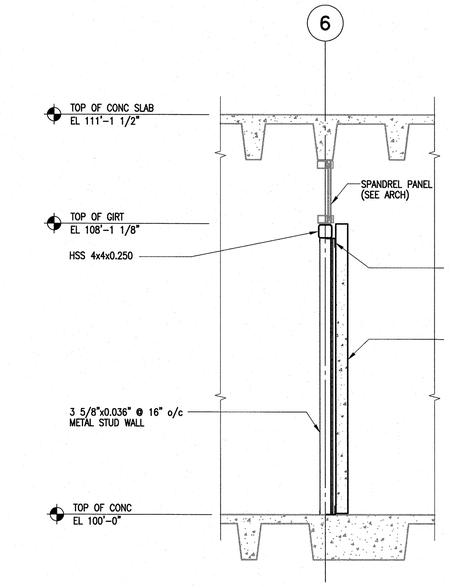
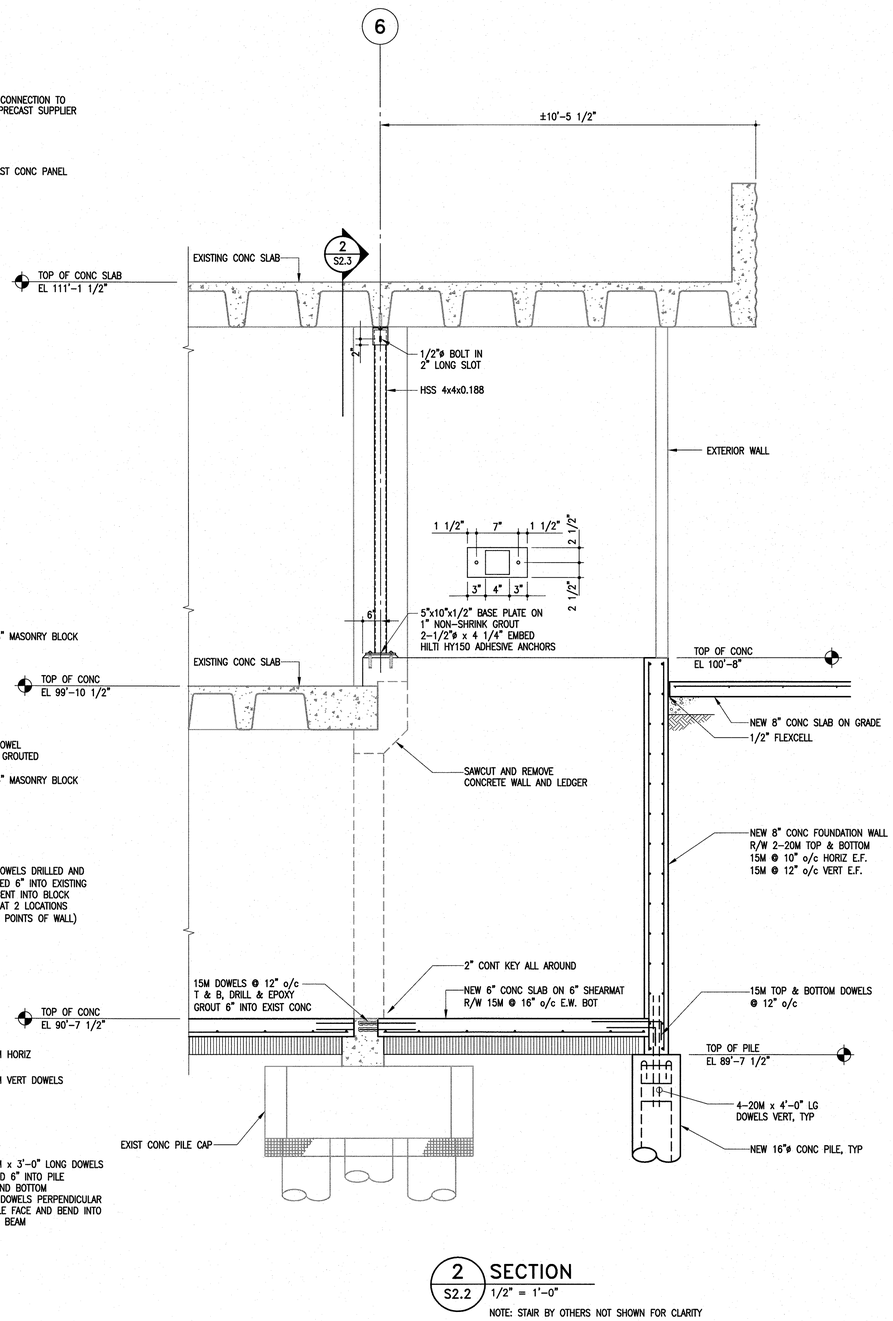
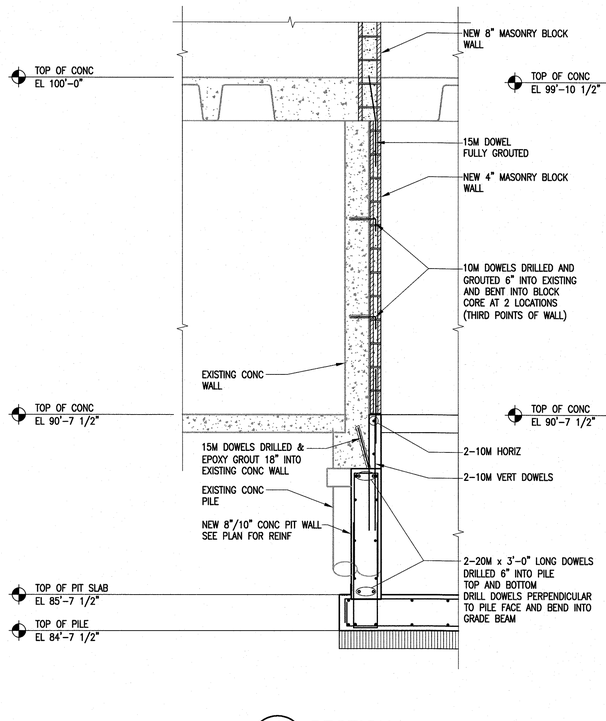


3 SECTION
S2.2 1/2" = 1'-0"



2 SECTION
S2.2 1/2" = 1'-0"

NOTE: STAIR BY OTHERS NOT SHOWN FOR CLARITY



1 SECTION
S2.2 1/2" = 1'-0"

GENERAL NOTES

1. STRUCTURAL DESIGN BASED ON THE NATIONAL BUILDING CODE OF CANADA 2010 EDITION.
   A) IMPORTANCE CATEGORY: NORMAL
   B) WIND LOAD: Q50 = 0.4 P.S.F.
   C) GROUND SNOW LOAD: SS = 39.6 P.S.F.
   D) ASSOCIATED RAIN LOAD: SR = 4.2 P.S.F.
2. SEISMIC SITE CLASSIFICATION: NOT APPLICABLE
3. DO NOT SCALE DRAWINGS.
4. DO NOT BACKFILL UNTIL GROUND FLOOR STRUCTURE IS IN PLACE AND BASEMENT SLABS HAVE BEEN POURED AND CURED.
5. ALL DIMENSIONS ARE TO BE VERIFIED WITH THE ARCHITECTURAL DRAWINGS AND EXISTING SITE CONDITIONS PRIOR TO CONSTRUCTION.
6. THESE STRUCTURAL DRAWINGS SHOW THE COMPLETED STRUCTURE AND DO NOT INDICATE ALL COMPONENTS NECESSARY FOR SAFETY DURING CONSTRUCTION. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SAFETY ON AND AROUND THE JOBSITE DURING CONSTRUCTION.

FOUNDATIONS

1. A COPY OF THE GEOTECHNICAL REPORT COMMISSIONED BY THE OWNER IS AVAILABLE FOR REVIEW AT THE OFFICES OF THE ARCHITECT CONSULTANT.
2. NOTWITHSTANDING THE INFORMATION PROVIDED IN THE GEOTECHNICAL REPORT THE FOUNDATION AND GENERAL CONTRACTORS SHALL SATISFY THEMSELVES AS TO THE PREVAILING CONDITIONS AT THE SITE AS NO EXTRAS SHALL BE GRANTED SHOULD CONDITIONS DIFFER FROM THOSE INDICATED.
3. ALL FRICTION PILES ARE DESIGNED ON AN ALLOWABLE SKIN FRICTION OF 300 P.S.F.. EFFECTIVE LENGTH OF FRICTION PILES IS TOTAL LENGTH AS SHOWN ON PLAN MINUS 10'-0" FOR PERIMETER AND EXTERIOR PILES AND A MINUS 5'-0" FOR INTERIOR PILES BELOW BASEMENT.
4. FRICTION PILE REINFORCING TO BE 20'-0" LONG UNLESS NOTED IN PLANS; 10M RINGS AT 48 IN. ON-CENTRE AND 3-10M RINGS AT 6" ON-CENTRE AT TOP. EXTEND VERTICAL PILE REINFORCING 1'-6" INTO BEAMS OR WALLS. PILE REINFORCING TO BE 5-10M FOR 16 IN. DIAMETER PILES, 6-10M FOR 18 IN., 5-15M FOR 20 IN., 5-15M FOR 22 IN., 6-15M FOR 24 IN..

CAST-IN-PLACE CONCRETE

I CONCRETE

1. ALL CONCRETE IS TO BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF CAN/CSA-A23.1-09 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION" AND CAN/CSA-A23.2-09 "METHOD OF TEST FOR CONCRETE".
2. PROVIDE CERTIFICATION THAT MIX PROPORTIONS SELECTED WILL PRODUCE CONCRETE OF QUALITY, YIELD AND STRENGTH AS SPECIFIED IN CONCRETE MIXES, AND WILL COMPLY WITH CAN/CSA-A23.1. CERTIFICATION LETTER TO BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA.
3. PROVIDE CERTIFICATION THAT PLANT, EQUIPMENT, AND MATERIALS TO BE USED IN CONCRETE COMPLY WITH REQUIREMENTS OF CAN/CSA-A23.1. CERTIFICATION LETTER TO BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA.
4. CONCRETE STRENGTHS AT 28 DAYS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

PILES:	32 MPA MIN. AT 56 DAYS. CLASS OF EXPOSURE: S-2 ENTRAINED AIR/CATEGORY: 2 (4% TO 7%) CEMENT TYPE: HS AGGREGATE: MAX. 20 MM CURING TYPE: TYPE 2 – ADDITIONAL TYPE 1 – BASIC SLUMP: MIN. 120 MM
EXTERIOR WALLS AND GRADE BEAMS:	25 MPA MIN. AT 28 DAYS CLASS OF EXPOSURE: F-2 ENTRAINED AIR/CATEGORY: 2 (4% TO 7%) AGGREGATE MAX. 20 MM CURING TYPE: TYPE 2 – ADDITIONAL
EXTERIOR SLABS-ON-GRADE:	25 MPA MIN. AT 28 DAYS CLASS OF EXPOSURE: C-2 F-1 F-2 ENTRAINED AIR/CATEGORY: 1 (5% TO 8%) 2 (4% TO 7%) AGGREGATE MAX. 20 MM CURING TYPE: TYPE 2 – ADDITIONAL TYPE 1 – BASIC
INTERIOR WALLS AND BEAMS:	25 MPA MIN. AT 28 DAYS CLASS OF EXPOSURE: N ENTRAINED AIR/CATEGORY: NONE (LESS THAN 3%) AGGREGATE MAX. 20 MM CURING TYPE: TYPE 2 – ADDITIONAL
INTERIOR STRUCTURAL SLABS:	25 MPA MIN. AT 28 DAYS CLASS OF EXPOSURE: N ENTRAINED AIR/CATEGORY: NONE (LESS THAN 3%) AGGREGATE MAX. 20 MM CURING TYPE: TYPE 2 – ADDITIONAL TYPE 1 – BASIC
MASONRY FILL:	20 MPA MIN. AT 28 DAYS CLASS OF EXPOSURE: F-2 N ENTRAINED AIR/CATEGORY: 2 NONE AIR CONTENT: 4% TO 7% LESS THAN 3% AGGREGATE MAX. 20 MM 14 MM SLUMP: 200 MM ± 40 MM

UNLESS INDICATED OTHERWISE THE GENERAL CONTRACTOR SHALL SPECIFY CONCRETE SLUMP APPROPRIATE WITH PLACEMENT METHODS AND SITE CONDITIONS. THE GENERAL CONTRACTOR SPECIFIED SLUMP MUST BE SHOWN ON THE CERTIFICATION LETTER AND CONCRETE DELIVERY TICKET.

5. UNLESS NOTED OTHERWISE CONCRETE CURING TO CONFORM TO THE LATEST EDITION OF CAN/CSA-A23.1-09 AS FOLLOWS:
   A) TYPE 1 – BASIC: 3 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN 40% OF THE SPECIFIED STRENGTH.
   B) TYPE 2 – ADDITIONAL: 7 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN 70% OF THE SPECIFIED STRENGTH.
   C) TYPE 3 – EXTENDED: 7 DAYS WET CURING ≥ 10°C.
6. AIR ENTRAINING ADMIXTURES SHALL CONFORM TO THE REQUIREMENTS OF ASTM C260/C260M-10A "STANDARD SPECIFICATION FOR AIR ENTRAINING ADMIXTURES FOR CONCRETE". SUPERPLASTICIZING ADMIXTURES SHALL CONFORM TO ASTM C494/C494M "STANDARD SPECIFICATION FOR CHEMICAL ADMIXTURES FOR CONCRETE" OR ASTM C1017/C1017M "STANDARD SPECIFICATION FOR CHEMICAL ADMIXTURES FOR USE IN PRODUCING FLOWING CONCRETE" WHEN FLOWING CONCRETE IS APPLICABLE. AIR ENTRAINED ADMIXTURES TO HAVE A DURABILITY FACTOR GREATER THAN 75, WHEN TESTED TO ASTM STANDARDS C666/C666M PROCEDURE A. SPACING FACTOR FOR ANY AIR ENTRAINING ADMIXTURE MUST BE 0.17MM OR LESS WHEN TESTED IN ACCORDANCE WITH ASTM C457 "STANDARD TEST METHOD FOR MICROSCOPICAL DETERMINATION OF PARAMETERS OF THE AIR-VOID SYSTEM IN HARDENED CONCRETE".

II REINFORCING STEEL

1. ALL REINFORCING STEEL TO BE CAN/CSA-G30.18M GRADE 400R DEFORMED BARS EXCEPT COLUMN TIES AND BEAM STIRRUPS WHICH SHALL BE GRADE 400W STEEL. ALL REINFORCING IS TO BE DETAILED IN ACCORDANCE WITH THE LATEST EDITION OF THE REINFORCING STEEL INSTITUTE OF CANADA – MANUAL OF STANDARD PRACTICE, EXCEPT OTHERWISE NOTED.
2. WELDED STEEL WIRE MESH SHALL BE TO ASTM A185/A185M-07, 400 MPA YIELD, FLAT SHEETS ONLY.
3. REINFORCING STEEL COVER IS TO CONFORM TO CAN/CSA A23.3-04 "DESIGN OF CONCRETE STRUCTURES FOR BUILDINGS" AND AS FOLLOWS:

EXTERIOR WALLS: EXPOSURE CLASS: F-2	1 1/2 IN. OUTSIDE FACE	3/4 IN. INSIDE FACE.	
INTERIOR WALLS: EXPOSURE CLASS: N	3/4 IN. EACH FACE		
INTERIOR STRUCTURAL SLABS: EXPOSURE CLASS: N	3/4 IN. TOP	3/4 IN. BOTTOM	
GRADE BEAMS: EXPOSURE CLASS: F-2	2 IN. BOTTOM TO TIES	1 1/2 IN. SIDES AND TOP TO TIES.	
INTERIOR BEAMS: EXPOSURE CLASS: N	1 1/4 IN. BOTTOM TO TIES	1 1/4 IN. SIDES AND TOP TO TIES.	
PILES: EXPOSURE CLASS: S-2	3 IN. TO TIES.		
EXTERIOR SLABS-ON-GRADE: EXPOSURE CLASS: F-2	1 1/2 IN. TOP	1 1/2 IN. BOTTOM	

4. IN WALLS AND GRADE BEAMS, BEND ALL TOP AND INTERMEDIATE HORIZONTAL STEEL 2'-0" AROUND CORNERS, OR USE EXTRA L BARS 4'-0" LONG. ALL OPENINGS IN WALLS TO HAVE 2-15M EACH SIDE AND 2-25M OVER, EXCEPT AS NOTED.
5. TOP STEEL IN BEAMS TO BE LAPPED AT CENTRE SPAN, BOTTOM STEEL TO BE BUTTED AT SUPPORT.
6. ALL REINFORCING TO BE HELD IN PLACE, AND TIED BY THE USE OF PROPER ACCESSORIES, SUCH AS HI-CHAIRS, SPACERS, ETC. TO BE SUPPLIED BY THE REINFORCING STEEL FABRICATOR. HI-CHAIRS TO HAVE 4 LEGS AND TO BE STAPLED OR NAILED TO THE FORMWORK.
7. ALL OPENINGS IN CAST-IN-PLACE CONCRETE FLATWORK TO BE TRIMMED WITH 2-15M ALL AROUND ON BOTH FACES, EXCEPT AS NOTED.
8. FOR ALL STRUCTURAL SLABS A MINIMUM OF 50% OF THE BOTTOM STEEL SHALL BE CONTINUED A MINIMUM DISTANCE OF 6 IN. INTO ALL SUPPORTING WALLS AND BEAMS. IF KEYS ARE USED AT JOINTS BETWEEN SLABS AND WALLS OR BEAMS, BOTTOM DOWELS EQUAL TO BOTTOM REINFORCEMENT OR 10M AT 12 IN. O/C SHALL BE PROVIDED WHICHEVER IS GREATER.
9. ALL MISCELLANEOUS CONCRETE PADS AND CURBS ARE TO BE REINFORCED WITH A MINIMUM OF 10M AT 16 IN. O/C EACH WAY, UNLESS NOTED.
10. WHEN CONCRETE BEAMS ARE CAST INTO A WALL CHASE, DOWELS SIZE AND NUMBER SAME AS BEAM REINFORCING ARE TO BE PROVIDED FROM WALL, UNLESS OTHERWISE SHOWN ON PLAN.
11. PROVIDE MINIMUM 2-10M BOTTOM INTEGRITY BARS THROUGHOUT STRUCTURES IN ACCORDANCE WITH CAN/CSA A23.3-04, CLAUSE 13.10.6.

III FORMWORK

1. SHEARMAT OR APPROVED CARDBOARD VOIDFORM WITH A MIN. DEPTH OF 6 IN. SHALL BE USED AS THE BOTTOM FORM FOR STRUCTURAL SLABS AT GRADE, GRADE BEAMS, AND WALLS IN CONTACT WITH SOIL. SELECT AND INSTALL IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
2. ACCESSORIES SUCH AS HI-CHAIRS, SPACERS, ETC. SHALL BE SUPPORTED BY PADS OF PLYWOOD OR TEMPERED HARDBOARD TO PREVENT PUNCTURING THE VOIDFORM.
3. UNLESS NOTED OTHERWISE PROVIDE SLIP JOINT ALL PAVING OR CONCRETE SLABS ON GRADE AGAINST STRUCTURAL MEMBERS WITH 1/2 IN. ASPHALT IMPREGNATED FIBREBOARD.
4. ALL CONSTRUCTION JOINT KEYS ARE TO BE A MINIMUM OF 1 1/2 IN. DEEP.
5. ALL STRUCTURAL SLABS FRAMING INTO BASEMENT WALLS ARE TO HAVE A MINIMUM KEY OF 1 1/2 IN.
6. ALL CONCRETE BEAMS FRAMING INTO CONCRETE WALLS ARE TO BE SUPPORTED BY A CHASE OF MINIMUM 4 IN. DEPTH AND THE HEIGHT AND WIDTH OF THE BEAM.
7. PLACE 10 MIL POLYETHYLENE UNDER ALL SLABS ON FILL AND OVER TOP OF VOIDFORM.
8. PROVIDE 6 IN. WIDE, RIBBED, PVC WATERSTOPS IN ALL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS IN ALL EXTERIOR WALLS BELOW GRADE AND PIT WALLS.

STRUCTURAL STEEL

1. THE STRUCTURAL STEEL FABRICATOR'S ENGINEER SHALL BE RESPONSIBLE FOR LOCATING AND DESIGNING PROVISIONS FOR ALL TEMPORARY FALL PROTECTION SYSTEMS REQUIRED DURING CONSTRUCTION TO MEET MANITOBA WORKPLACE HEALTH AND SAFETY REGULATIONS.
2. STRUCTURAL STEEL TO CONFORM TO CSA-G40.21, "STRUCTURAL QUALITY STEELS" AND CSA-G40.20 "GENERAL REQUIREMENTS FOR ROLLED OR WELDED STRUCTURAL QUALITY STEEL".
3. ALL ROLLED OR STEEL STRUCTURAL SECTIONS SHALL BE G40.21-50W. ALL HOLLOW STRUCTURAL SECTIONS TO BE G40.21-50W CLASS C. ALL ANGLES, CHANNELS AND PLATES SHALL BE G40.21-44W.
4. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE PERFORMED IN ACCORDANCE WITH CAN/CSA S16-09, "STEEL STRUCTURES FOR BUILDINGS".
5. ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF CSA W59, "WELDED STEEL CONSTRUCTION". FABRICATORS SHALL BE PROPERLY CERTIFIED IN ACCORDANCE WITH CSA W47.1, "CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL STRUCTURES".
6. ALL BOLTED CONNECTIONS TO USE A325 HIGH STRENGTH BOLTS. MINIMUM CONNECTION SHALL CONSIST OF 2 BOLTS.
7. ALL STRUCTURAL STEEL IS TO RECEIVE ONE COAT OF CISC/CPMA 1-73A QUICK DRYING SHOP PRIMER. STEEL IN CRAWLSPACES SHALL RECEIVE 2 COATS. STEEL TO BE CLEANED IN CONFORMANCE WITH SSPC-SP2. STEEL RECEIVING FINISH PAINTING TO HAVE ONE COAT OF CISC/CPMA 2-75 QUICK DRYING SHOP PRIMER. STEEL TO BE CLEANED IN CONFORMANCE WITH SSPC-SP7.
8. STRUCTURAL STEEL SUPPLIER TO PROVIDE 1 1/2 IN. X 3/16 IN. MASONRY ANCHORS BY 16 IN. LONG WITH 2 IN. HOOK AT 32 IN. O/C, ON ALL COLUMNS AND BEAMS IN CONTACT WITH MASONRY.
9. NO HOLES PERMITTED IN TOP FLANGE OF BEAMS AT COLUMNS WHERE BEAMS ARE CONTINUOUS OVER COLUMNS.
10. ALL BEAMS CONTINUOUS OVER COLUMNS ARE TO HAVE WEB STIFFENERS THE SAME SIZE AND ORIENTATION AS THE COLUMN BELOW, UNLESS OTHERWISE NOTED.
11. ANCHOR BOLTS TO BE GRADE ASTM A307 PROVIDED BY STEEL SUPPLIER AND SET BY THE GENERAL CONTRACTOR.
12. FABRICATOR TO NOTIFY ENGINEER OF ANY PROPOSED MEMBER SUBSTITUTIONS AND CHANGED CONNECTION DETAILS.
13. THE STRUCTURAL STEEL SUPPLIER SHALL PROVIDE AND BE RESPONSIBLE FOR ALL HOLES IN STEEL SECTIONS REQUIRED BY OTHER TRADES. SECTION SHALL BE STRENGTHENED WHERE REQUIRED TO GUARANTEE THE ORIGINAL STRENGTH OF THE BEAM. ANY CUTTING OF STEEL AT THE JOB SITE SHALL BE DONE ONLY AS DIRECTED AND APPROVED BY THE ENGINEER.
14. THE STRUCTURAL STEEL ERECTOR SHALL BE RESPONSIBLE FOR SUPPLYING AND ERECTING ALL TEMPORARY GUYING AND BRACING OF THE STEEL FRAMING TO PROVIDE STABILITY FOR THE STRUCTURE AS A WHOLE. THESE SHALL REMAIN IN PLACE UNTIL ALL STEEL DECKING IS ERECTED, WELDED IN PLACE AND ALL MASONRY/CONCRETE WALLS CONSTRUCTED.
15. STRUCTURAL STEEL SUPPLIER IS TO SUBMIT ENGINEERING DRAWINGS BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA COVERING THE DESIGN OF CONNECTIONS, TO THE PROJECT DESIGN ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONNECTION DESIGN TO INCLUDE FOR ALL ADJUSTABLE CONNECTIONS REQUIRED TO SUITE FABRICATION AND ERECTION PROCEDURES AND TOLERANCES.
16. STRUCTURAL STEEL WHICH SUPPORTS ARCHITECTURAL FINISHES MUST BE DESIGNED TO BE SUFFICIENTLY ADJUSTABLE TO MEET REQUIRED INSTALLATION TOLERANCES. SEE ARCHITECTURAL FOR REQUIRED FINISH TOLERANCES.

MISCELLANEOUS METAL – STEEL STAIR AND GUARDRAILS

1. STEEL STAIR AND GUARDRAIL SUPPLIER IS TO SUBMIT ENGINEERING DRAWINGS BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA FOR REVIEW BY THE PROJECT ENGINEER, PRIOR TO FABRICATION. ENGINEERING SHOP DRAWINGS SHALL INCLUDE DESIGN LOADS, LAYOUT PLAN, CONNECTION DETAILS, AND ALL OTHER PERTINENT INFORMATION.
2. STEEL STAIR AND GUARDRAIL SUPPLIER/DESIGNER SHALL PROVIDE A FINAL INSPECTION AND A LETTER SEALED BY THE ENGINEER RESPONSIBLE FOR THE STAIR AND GUARDRAIL DESIGN, CERTIFYING THAT STAIRS AND GUARDRAILS ARE CONSTRUCTED AND INSTALLED AS PER DESIGN ASSUMPTIONS AND INSTALLATION REQUIREMENTS.

METAL DECK

1. ROOF DECK SHALL BE 1 1/2 IN. DEEP PROFILE, 0.030 IN., WITH RIB SPACING OF 5.91 IN.
2. DECK SHALL BE MINIMUM GRADE A WITH A MINIMUM GALVANNEAL ZINC COATING TO Z275.
3. DECK SHALL BE ARC SPOT WELDED TO BEARING SUPPORTS AT 12 IN. O/C. WELDS SHALL BE 3/4 IN. DIAMETER.
4. SIDE LAPS SHALL BE MECHANICALLY FASTENED AT 24 IN. ON-CENTRE.
5. TOUCH UP DECK WITH ZINC RICH PAINT WHERE ZINC COATING HAS BEEN BURNED BY WELDING.

MASONRY

1. CONCRETE BLOCKS TO CONFORM TO CAN/CSA-A165-04 SERIES "STANDARDS FOR CONCRETE MASONRY UNITS".
   A) STANDARD HOLLOW MASONRY UNITS SHALL BE H/2100/A/M.
   B) STANDARD SOLID MASONRY UNITS SHALL BE S/2100/A/M.
   C) LIGHTWEIGHT HOLLOW MASONRY UNITS SHALL BE H/2100/C/M.
   D) LIGHTWEIGHT SOLID MASONRY UNITS SHALL BE S/2100/C/M. (COMPRESSIVE STRENGTH IS BASED ON NET AREA).
2. EXTERIOR AND LOAD BEARING WALLS ARE TO BE BUILT WITH TYPE "S" MORTAR HAVING A MINIMUM STRENGTH OF 12 MPA AT 28 DAYS. MORTAR SHALL CONFORM TO THE LATEST EDITION OF CSA A179, "MORTAR AND GROUT FOR UNIT MASONRY".
3. USE DUR-O-WAL OR EQUAL EVERY SECOND COURSE. EVERY COURSE FOR STACK BOND.
4. THE TOP COURSE OF ALL BLOCK WALLS IS TO BE A "U" BLOCK WITH 2-10M CONTINUOUS CENTRED AND FILLED WITH 20 MPA CONCRETE UNLESS NOTED ON PLAN.
5. ALL MASONRY WALLS TO BE PROPERLY BRACED UNTIL STRUCTURE IS CLOSED IN AND WALL PERMANENTLY SUPPORTED.
6. ALL BLOCK WALLS RECEIVING BEAMS TO HAVE 2 COURSES HIGH, 16 IN. LONG FILLED WITH 20 MPA CONCRETE UNLESS NOTED ON DRAWINGS.
7. DOOR LINTELS IN BLOCK WALLS SHALL BE AS FOLLOWS UNLESS NOTED ON DRAWINGS:

UP TO 4'-0"	8 IN. HIGH "U" BLOCK 20 MPA CONCRETE FILL 2-10M BOTTOM
4'-4" TO 8'-0"	16 IN. HIGH "U" BLOCK 20 MPA CONCRETE FILL 2-15M BOTTOM

THIS DRAWING MUST NOT BE SCALED.

THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS, DATUMS, AND LEVELS PRIOR TO COMMENCEMENT OF WORK. ALL ERRORS AND OMISSIONS TO BE REPORTED TO NUMBER TEN ARCHITECTURAL GROUP BEFORE PROCEEDING.

VARIATIONS AND MODIFICATIONS TO WORK SHOWN ON THESE DRAWINGS SHALL NOT BE CARRIED OUT WITHOUT WRITTEN PERMISSION OF NUMBER TEN ARCHITECTURAL GROUP.

THIS DRAWING IS THE EXCLUSIVE PROPERTY OF NUMBER TEN ARCHITECTURAL GROUP AND THE COPYRIGHT IN THE SAME BEING RESERVED TO THEM. IT MAY BE REPRODUCED ONLY WITH THE PERMISSION OF NUMBER TEN ARCHITECTURAL GROUP, IN WHICH CASE THE REPRODUCTION MUST BEAR THEIR NAME.

REVISED/ ISSUED/ PLOTTED	DATE
ISSUED FOR TENDER	NOV 21, 2011

APEGM
Certificate of Authorization
Crosier Kilgour & Partners Ltd.
No. 235 Date: Nov 21, 2011

numberTEN
architectural group
winnipeg MB 204 942.0981 regina SK 306 721.1501 victoria BC 250 360.2106
architecture ▪ interior design ▪ graphic design

project
PEGUIS PAVILION
UPGRADES AND ENTRY ADDITION
BIDDING #913-2011

sheet title
SECTIONS AND GENERAL NOTES

consultant
300-275 Carlton Street
Winnipeg, Manitoba R3C 5R6
T 204. 943. 7501
F 204. 943. 7507
2011-0492
Crosier Kilgour & Partners Ltd.
CONSULTING STRUCTURAL ENGINEERS

seal
PROVINCE OF MANITOBA
T.K. MALKIEWICZ
Nov 21/11
REGISTERED PROFESSIONAL ENGINEER

drawn by: MS
checked by: TKM
project no. 2011026

scale: AS NOTED
date: November 21, 2011
sheet no. S2.2