

### LEGEND

	CONCRETE		GROUT
	CUT AND REMOVE EXISTING CONCRETE/CMU CUT IN PLAN OR SECTION		RIGID INSULATION
	ROCK		EARTH OR FINISHED GRADE
	CHECKERED PLATE		CAPILLARY WATERPROOFING
	HINGE SIDE		CHEMICAL RESISTANT COATING
	GRATING		WELDED WIRE FABRIC
	SPAN		SLOPE DIRECTION
			STANDARD DETAIL NUMBER
			VOID FORM

### GENERAL NOTES

- DIMENSIONS IN MILLIMETRES.
- ELEVATIONS IN METRES.
- READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH ALL RELATED CIVIL, ARCHITECTURAL, MECHANICAL, PROCESS, AND ELECTRICAL DRAWINGS, EXISTING DRAWINGS AND OTHER CONTRACT DOCUMENTS.
- DESIGN DETAILS, SECTIONS, AND STANDARD DETAILS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS AND LOCATIONS OCCURRING THROUGHOUT THE PROJECT, WHETHER OR NOT THEY ARE INDIVIDUALLY CALLED OUT ON DRAWINGS.
- TYPICAL STRUCTURAL DETAILS SHOWN ON DRAWINGS SHALL GOVERN THE WORK. IF DETAILS DIFFER ON OTHER DRAWINGS OR SPECIFICATIONS, THE MOST STRINGENT SHALL GOVERN.
- DESIGN LOADS INDICATED ON DRAWINGS ARE SERVICES LOADS (UNFACTORED). DESIGN LOADS INDICATED ON DRAWINGS WITH SUBSCRIPT "f" ARE FACTORED LOADS.
- DO NOT EXCEED DESIGN LOADS NOTED ON DRAWINGS DURING CONSTRUCTION.
- SEE OTHER CONTRACT DRAWINGS AND COORDINATE FOR ACTUAL SIZES, LOCATIONS AND DETAILS OF OPENINGS FOR PIPES, SLEEVES, DUCTS, FLOOR DRAINS, CONDUITS, AND OTHER PENETRATIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE MECHANICAL AND/OR PROCESS DRAWINGS AND COORDINATE FOR ACTUAL SIZES, LOCATIONS AND DETAILS OF EQUIPMENT BASES, SLUICE GATES, SLIDE GATES, IRRIGATION GATES, STOP LOG GUIDES, AND SIMILAR ITEMS.
- SEE ELECTRICAL DRAWINGS FOR SIZES, REINFORCING, AND LOCATIONS OF CONCRETE ENCASED CABLES, CONDUITS, DUCT BANKS, AND CONCRETE PADS FOR ELECTRICAL EQUIPMENT NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS FOR SIZES AND LOCATIONS OF CONCRETE CURBS, RAILING, SHELF ANGLES, LOOSE LINTELS, ABRASIVE STAIR NOSINGS, REGLETS, INSERTS, AND THRESHOLDS NOT SHOWN ON STRUCTURAL DRAWINGS.
- STRUCTURAL MEMBERS SHALL NOT BE CUT OR MODIFIED UNLESS SPECIFICALLY DETAILED OR APPROVED IN WRITING BY THE CONTRACT ADMINISTRATOR.
- ALL LIQUID RETAINING STRUCTURES AND STRUCTURES BELOW GRADE SHALL BE WATERTIGHT.
- DIMENSIONS, ELEVATIONS AND DETAILS OF EXISTING STRUCTURES ARE BASED ON PREVIOUS CONTRACT DRAWINGS AND DO NOT NECESSARILY REPRESENT THE AS-CONSTRUCTED CONDITIONS. FIELD VERIFY ALL DIMENSIONS, ELEVATIONS AND DETAILS OF EXISTING STRUCTURES PRIOR TO FABRICATION OF ADJACENT OR CONNECTING WORK. REPORT TO CONTRACT ADMINISTRATOR ANY DISCREPANCIES OR UNSATISFACTORY CONDITIONS WHICH MAY ADVERSELY AFFECT PROPER COMPLETION OF THE WORK BEFORE PROCEEDING.

### DESIGN CRITERIA

- APPLICABLE CODE: 2010 NATIONAL BUILDING CODE WITH 2011 MANITOBA AMENDMENTS (NBC).
- IMPORTANCE CATEGORY: POST DISASTER
 

Is (ULS) = 1.25	Is (SLS) = 0.9
Iw (ULS) = 1.25	Iw (SLS) = 0.75
- SITE LOCATION: WINNIPEG, MANITOBA
- SNOW LOAD DATA:
 

GROUND SNOW LOADING	S <sub>s</sub> = 1.90 kPa
ASSOCIATED RAIN LOADING	S <sub>r</sub> = 0.20 kPa
- RAIN LOAD DATA: ONE DAY RAINFALL 108 mm
- SEISMIC DATA: N/A
- WIND LOAD DATA:
 

1 IN 50 YEAR HOURLY WIND PRESSURE EXPOSURE CATEGORY	0.45 kPa OPEN TERRAIN
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### CONCRETE SURFACE FINISH LEGEND

(SEE SPECIFICATION 03 39 00.3.9 FOR FINISH DESCRIPTION)

	S-1	STEEL TROWEL FINISH
	S-2	WOOD FLOAT FINISH
	S-3	NON SLIP FINISH
	S-4	BROOM FINISH
	S-5	SCRATCH FINISH
	S-6	BONDED CONCRETE TOPPING
	S-7A	CAPILLARY WATERPROOFING
	S-7B	FLOOR HARDENER
	S-8	BONDED CONCRETE TOPPING
	W-1	ORDINARY WALL FINISH
	W-2	SMOOTH WALL FINISH
	W-3	CONTROLLED PERMEABILITY FORM LINER
	W-4	SMOOTH RUBBED FINISH
	W-5	SACK RUBBED FINISH
	W-6	ABRASIVE BLAST FINISH

### FOUNDATIONS

- REFER TO GEOTECHNICAL REPORT "SEWPPCC UPGRADE/EXPANSION/CIVIL/GEOTECH GEOTECHNICAL INVESTIGATION REPORT BY KGS GROUP, DATED OCTOBER 2014."
- DESIGN GROUND WATER LEVEL ELEVATION: 227.800m, EXCEPT AT TOP OF WEEPING TILE WHERE WEEPING TILE IS SHOWN. STRUCTURES ARE NOT DESIGNED FOR GROUND WATER LOADS.
- DESIGN SURCHARGE: 5.0 kPa EXCEPT WHERE NOTED OTHERWISE
- BACK FILL:
 

UNIT WEIGHT:	19 kPa
1/3 DENSITY OF BACKFILL MATERIAL ABOVE DESIGN GROUND WATER LEVEL	19 kN/m <sup>3</sup>
2/3 DENSITY OF BACKFILL MATERIAL BELOW DESIGN GROUND WATER LEVEL	22 kN/m <sup>3</sup>
EARTH PRESSURE (AT REST COEFFICIENT)	K <sub>o</sub> (0.42)
EARTH PRESSURE (ACTIVE COEFFICIENT)	K <sub>a</sub> (0.27)
- PRESTRESSED CONCRETE PILES (HEXAGONAL).
 

PILE DIAMETER (D)	SLS PILE CAPACITY
300mm	555 kN
350mm	780 kN
400mm	1050 kN
- SEE PILE DETAILS FOR TYPICAL CUT-OFF REQUIREMENTS.
- NOTIFY CONTRACT ADMINISTRATOR IF FINAL ELEVATION OF TOP OF INSTALLED PILE OR LOCATION OF PILE DIFFERS FROM CONTRACT DOCUMENTS. SEE PILE DETAILS FOR PILE EXTENSION.
- PROTECT FOUNDATION INCLUDING: SLAB ON GRADE, GRADE BEAMS AND FOOTINGS AGAINST FREEZING AND FROST ACTION DURING CONSTRUCTION.
- PROTECT EXISTING FOUNDATIONS, WALLS, PILES AND SHORING TO REMAIN FROM DAMAGE.
- BACKFILL EVENLY ALL AROUND STRUCTURES, IN LIFTS NOT EXCEEDING 150mm.

### CONCRETE AND CONCRETE REINFORCING

- CONCRETE COMPRESSIVE STRENGTH (MIN):
 

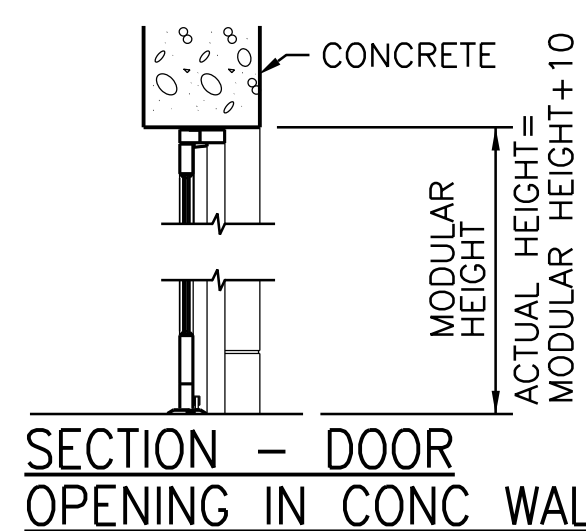
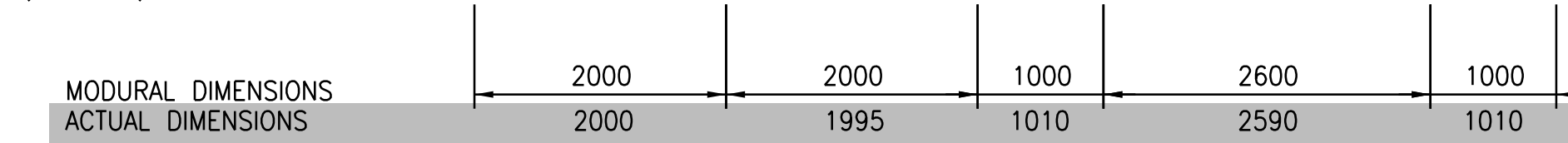
TYPE - A	STRENGTH (MPa)	CLASS OF EXPOSURE
TYPE - B	30 AT 56 DAYS	S-3
TYPE - C	15	N
PRECAST	32 AT 28 DAYS	C-2
	35 AT 28 DAYS	S-3
- REINFORCING BARS: CAN/CSA-G30.18-09, GRADE 400R, GRADE 400W FOR WELDED REBARS.
- REFERENCE CODES AND STANDARDS:
  - CSA A23.1-09 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION
  - CSA A23.2-09 METHODS OF TEST AND STANDARD PRACTICES FOR CONCRETE
  - CSA A23.3-09 DESIGN OF CONCRETE STRUCTURES
  - ACI 350M-06, ACI 350.1-10 AND ACI 350.3-06 FOR LIQUID RETAINING STRUCTURES
- FABRICATE AND PLACE REINFORCING STEEL IN ACCORDANCE WITH RSIC MANUAL OF STANDARD PRACTICE, UNLESS NOTED OTHERWISE.
- FOR CONCRETE COVER TO REINFORCING STEEL, BENDS, LAPS AND ADDITIONAL REINFORCEMENT BELOW GRADE FOR WATER TIGHTNESS SEE STANDARD DETAIL.
- MINIMUM REINFORCING FOR ALL CONCRETE WALLS AND SLABS UNLESS NOTED OTHERWISE:
 

WALL THICKNESS (mm)	REINF EACH WAY	LOCATION
150	15M@300	CENTRED
200	15M@300	EACH FACE; EACH WAY
250	15M@300	EACH FACE; EACH WAY
300	20M@300	EACH FACE; EACH WAY
- CONTINUOUS WATERSTOP AS SPECIFIED SHALL BE INSTALLED IN ALL CONSTRUCTION JOINTS IN WALLS OF WATER HOLDING BASINS, CHANNELS, AND BELOW GRADE STRUCTURES EXCEPT WHERE SPECIFICALLY NOTED OTHERWISE.
- CONSTRUCTION JOINTS INDICATED ARE SUGGESTED LOCATIONS. CONTRACTOR MAY REVISE LOCATIONS OF JOINT IN ACCORDANCE WITH SPECIFIED REQUIREMENTS. CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED FOR REVIEW.
- COORDINATE PLACEMENT OF OPENINGS, CURBS, DOWELS, SLEEVES CONDUITS, BOLTS, INSERTS, ETC. PRIOR TO PLACEMENT OF CONCRETE.
- PROVIDE 200mm HIGH MINIMUM CONCRETE CURBS BELOW MASONRY WALLS UNLESS NOTED OTHERWISE. WHEN EXPOSED TO VIEW, PROVIDE SETBACKS FOR BOTH LOAD BEARING AND PARTITION MASONRY WALLS AS SHOWN ON ARCHITECTURAL DRAWINGS.
- PROVIDE CHAMFER TO ALL EXPOSED CORNERS.
- PROVIDE DOWELS FROM PILES, COLUMNS/PIERS, FOOTINGS, SLABS, WALLS OR BEAMS TO COLUMNS/PIERS OR WALLS SIMILAR IN NUMBERS, SIZE AND SPACING TO THE VERTICAL STEEL IN THE COLUMNS/PIERS OR WALLS ABOVE UNLESS NOTED OTHERWISE.
- LOCATE SLAB AND BEAM TOP BAR SPLICES AT MID-SPAN AND BOTTOM BAR SPLICES AT SUPPORTS.
- CONDUITS SHALL NOT BE PLACED PARALLEL WITH BEAM OR COLUMN REINFORCING UNLESS SPECIFICALLY INDICATED IN DRAWINGS.

### MODULAR DIMENSIONING

- UNLESS OTHERWISE NOTED: ALL DIMENSIONS ON DRAWINGS ARE INDICATED AS MODULAR.
- FOR HORIZONTAL DIMENSIONS, THE ACTUAL WIDTH OF OPENING SHALL BE OPENING DIMENSION AS INDICATED ON PLAN DRAWINGS PLUS 10 mm; COMPENSATION OF 5mm ON EACH SIDE OF DOOR OPENING.
- FOR VERTICAL DIMENSIONS, THE ACTUAL HEIGHT OF OPENING SHALL BE OPENING HEIGHT DIMENSION AS INDICATED ON SECTION DRAWINGS PLUS 10 mm.
- MODULAR DIMENSIONS DO ONLY APPLY FOR DOOR OPENINGS. THEY DO NOT APPLY TO OPENINGS OF WINDOWS, LOUVERS, ROLLING DOOR ETC.

### PLAN - MODULAR/ACTUAL DOOR OPENINGS (EXAMPLE)



**APEGM**  
Certificate of Authorization  
CH2M HILL Canada Ltd.  
No. 1441

NO.	REVISIONS	DATE	DESIGN	CHECK
05	ISSUED FOR ADD. 2 - B.O. 899-2015	02/2016	C.R.	J.C.
04	ISSUED FOR TENDER - B.O. 899-2015	01/2016	G.O.	J.C.
03	ISSUED FOR 60% PRELIMINARY REVIEW	12/2015	G.O.	J.C.
02	ISSUED FOR CONSTRUCTION	10/2015	G.O.	J.C.
01	ISSUED FOR 60% PRELIMINARY REVIEW	09/2015	G.O.	J.C.
00	ISSUED FOR TENDER	08/2015	G.O.	J.C.

<b>CH2MHILL</b>	
DESIGNED BY: J. THANNICKAL	CHECKED BY: E. LUI
DRAWN BY: G.OMORI	APPROVED BY: H.T. FREIHAMMER
SCALE: NTS	ISSUED FOR CONSTRUCTION BY: T. TURZAK
DATE: 2015/08/13	DATE: 2016/01/29
CONSULTANT NO.: 474248	

ENGINEER'S SEAL  
24/02/2016  
PROVINCE OF MANITOBA  
S.L.E. LUI  
Member 37003  
REGISTERED PROFESSIONAL ENGINEER

**THE CITY OF WINNIPEG**  
Winnipeg WATER AND WASTE DEPARTMENT  
SOUTH END WATER POLLUTION CONTROL CENTRE  
SEWPPCC UPGRADE/EXPANSION PROJECT  
STRUCTURAL  
LEGEND AND GENERAL NOTES  
CITY DRAWING NUMBER: 1-0102-SDTL-A001  
SHEET: 05  
REV: A1  
SIZE: A1

### MASONRY

- MORTAR: CSA A179-04 PORTLAND CEMENT LIME, TYPE S
- GROUT: CSA A179-04 12 MPa
- CONCRETE MASONRY UNITS: CAN/CSA A165.1-04, HOLLOW TYPE H/15/A/M
- VERTICAL REINFORCING STEEL: CAN/CSA G30.18-92, GRADE 400R
- HORIZONTAL REINFORCING: CSA A370-04, EXTRA HEAVY DUTY 4.8mm DIAMETER TRUSS TYPE
- DESIGN f'm OF THE FINISHED ASSEMBLY SHALL BE 7.5 MPa.
- THE MINIMUM REINFORCING FOR REINFORCING CONCRETE BLOCK WALLS:
 

WALL THICKNESS (mm)	VERTICAL REINFORCING	LOCATION	HORIZONTAL REINFORCING
200	15M@1200	CENTERED	EVERY OTHER COURSE
250	15M@1200	CENTERED	EVERY OTHER COURSE
300	15M@1200	EACH FACE	EVERY OTHER COURSE
- PLACE NO CONDUIT IN CELLS CONTAINING REINFORCEMENT.

### STRUCTURAL STEEL AND METAL FABRICATIONS

- MATERIAL SHALL CONFORM TO THE FOLLOWING:
 

STRUCTURAL STEEL	CAN/CSA G40.20/G40.21-04, GRADE 350W FOR ALL W AND H SECTIONS, GRADE 300W FOR OTHERS
HOLLOW STRUCTURAL SECTION	CAN/CSA G40.20/G40.21-04, GRADE 350W, CLASS C
HIGH STRENGTH BOLTS	ASTM A325 OR ASTM A490
SHEAR STUDS	20mm DIAMETER, F <sub>y</sub> =350MPa
ALUMINUM	CAN/CSA S157-05/S157.1-05, ALLOY 6351-T6 FOR STRUCTURAL EXTRUDED SHAPES
STAINLESS STEEL	ASTM A666-03, TYPE 316/316L WITH MINIMUM YIELD STRESS F <sub>y</sub> = 207 MPa (30 ksi)
ANCHOR BOLTS	ASTM A307-04; GRADE 248 MPa
- REFERENCE CODES:
 

STRUCTURAL STEEL	CAN/CSA-S16-09 LIMIT STATES DESIGN OF STEEL STRUCTURES
ALUMINUM	CAN/CSA S157-05/S157.1-05 STRENGTH DESIGN IN ALUMINUM/ COMMENTARY ON CSA S157-05
STAINLESS STEEL	ASTM A666-03 STANDARD SPECIFICATION FOR ANNEALED OR COLD-WORKED AUSTENITIC STAINLESS STEEL SHEET, STRIP, PLATE, AND FLAT BAR ASTM A276-08 STANDARD SPECIFICATION FOR STAINLESS STEEL BARS AND SHAPES CSA S136-07 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
COLD FORMED STEEL	
WELDING	CSA W59-03 WELD STEEL CONSTRUCTION (METAL ARC WELDING) CSA W59.2-11991(R2008) WELDED ALUMINUM CONSTRUCTION AMERICAN WELDING SOCIETY AWS D1.6/D1.6M:2007 STRUCTURAL WELDING CODE - STAINLESS STEEL
- SHOP CONNECTIONS SHALL BE WELDED. FIELD CONNECTIONS SHALL BE WELDED OR BOLTED USING HIGH STRENGTH BOLTS, BEARING TYPE WITH THREADS INCLUDED IN THE SHEAR PLANE.
- BEAM CONNECTIONS SHALL BE C.I.S.C. DOUBLE ANGLE CONNECTIONS USING A325 BOLTS AND FILLET WELDS UNLESS NOTES OTHERWISE. MINIMUM SIZE OF BOLTS SHALL BE 19mm DIAMETER.
- ALL MOMENT CONNECTIONS SHALL BE DESIGNED FOR 90% MOMENT CAPACITY OF THE MEMBER. THE WEB CONNECTIONS SHALL BE DESIGNED FOR THE SHEAR CAPACITY OF THE MEMBER.
- FOR SHEAR AT NON-COMPOSITE SIMPLE SPAN CONNECTIONS, PROVIDE FOR HALF THE TOTAL FACTORED LOAD ON THAT SPAN AS TABULATED IN THE C.I.S.C. STEEL HANDBOOK BEAM LOAD TABLES, BUT NOT LESS THAN 50% OF THE SHEAR CAPACITY OF THE BEAMS.
- ALL COLUMN SPLICES, DIAGONAL BRACING CONNECTIONS, AND MOMENT CONNECTIONS SHALL BE PRETENSIONED BEARING TYPE CONNECTIONS USING HIGH STRENGTH BOLTS.
- SPLICES SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF THE MEMBER AT THE POINT OF SPLICE. MEMBER SHALL NOT BE SPLICED AT POINT OF MAXIMUM STRESS.
- BRACING MEMBERS SHALL BE CONNECTED FOR THE FOLLOWING (WHICHEVER IS LARGER):
  - 50% OF THE FACTORED TENSILE RESISTANCE OF THE MEMBER BASED ON THE GROSS AREA OF THE MEMBER
  - FORCES AS SHOWN ON THE DRAWINGS
  - PROVIDE A MINIMUM OF TWO BOLTS
  - CROSS BRACE SHALL BE CONNECTED AT CENTRE
- FORCES ARE DESIGNATED BY (+) FOR TENSION AND (-) FOR COMPRESSION.
- CONNECTION FOR BEAMS SUBJECT TO AXIAL FORCES SHALL BE DESIGNED FOR THE AXIAL FORCES IN ADDITION TO THE SHEAR AND/OR MOMENT FORCES.
- PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES OF THE WEB OF BEAMS AT POINT OF CONCENTRATED LOAD INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER TOPS OF COLUMNS. MINIMUM STIFFENER PLATES THICKNESS SHALL BE 10mm OR FLANGE THICKNESS OF THE COLUMN ABOVE OR BELOW, WHICHEVER IS GREATER. MINIMUM SIZE OF WELD SHALL BE 5mm DOUBLE FILLET WELDS OR SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE STIFFENER, WHICHEVER IS GREATER.
- ALL EXTERIOR EXPOSED STEEL SHALL BE HOT-DIP GALVANIZED.