APPENDIX 'A' GEOTECHNICAL REPORT

CITY OF WINNIPEG

2019 LOCAL STREET RENEWALS - 19-R-03 CONTRACT 1 GEOTECHNICAL REPORT

APRIL 29, 2019 ORIGINAL





2019 LOCAL STREET RENEWALS - 19-R-03 CONTRACT 1 GEOTECHNICAL REPORT

CITY OF WINNIPEG

ORIGINAL

WSP PROJECT NO.: 18M-01969-00

DATE: APRIL 29, 2019

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PROFESSION

ENGINEERS
GEOSCIENTISTS
MANITOBA

Certificate of Authorization

WSP Canada Inc.

No. 5750 Date: 2019-04-29

REVIEWED BY

Dana Bredin, P.Eng. Project Engineer

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1 INTRODUCTION

A geotechnical investigation was conducted by WSP Canada Inc. for the proposed 2019 Local Street Renewals – Contract 1 (Project # 19-R-03) in Winnipeg, Manitoba. The purpose of this investigation was to assess the general subsurface conditions with respect to identifying the existing pavement structure and the underlying soil profile.

Three (3) streets were cored and two (2) streets were cored and drilled, which includes the following:

- 1. Dowker Ave from Pembina Hwy to Crowson Bay (east leg) drilled and cored
- 2. Mulvey Ave from Lilac St to Arbuthnot St drilled and cored
- 3. **Maureen St** from Barron Dr to Assiniboine Ave cored only
- 4. Whitegate Cres from Barron Dr to Maureen St cored only
- 5. **Ness Ave** from Pebblewood Ln to Sturgeon Rd cored only

2 SUB-SURFACE INVESTIGATION AND TESTING

The field investigation commenced on March 20, 2019 and was completed on April 11, 2019. A total of 11 testholes and 20 pavement cores were completed by Maple Leaf Drilling. The testholes were drilled to a depth of 3.05 m below the road surface using a B40 truck-mounted rig equipped with a 125 mm solid stem auger. The pavement was cored using a 150 mm diameter coring press. All testholes were backfilled with auger cuttings and bentonite after the completion of the drilling and patched with hot mix asphalt. All pavement cores were patched with hot mix asphalt. Testhole and pavement core locations are noted on the testhole logs, and within the testhole and pavement core summary tables.

The soils encountered were visually classified to the full extent of the test hole. Representative soil samples were recovered at regular intervals starting from 0.1 m below pavement structure and every 0.3 m thereafter to a maximum depth of approximately 2.4 m below grade (mbg). All of the soil samples were tested for their moisture contents and selected soil samples were submitted for grain size analysis and Atterberg limits (minimum one per street). The pavement cores were measured for their thickness and each core was photographed. Any groundwater seepage or sloughing that was encountered in any of the test holes during drilling was noted.

The photos of the pavement cores, detailed descriptions of the soil profiles for each test hole, the material test results and the testhole maps are included in Appendices, organized by street.

3 TESTHOLE SUMMARY TABLES

Table 3-1 – Dowker Avenue

TEST HOLE	TESTHOLE LOCATION		EMENT RFACE	PAVEN STRUCTURE		SOIL DESCRIPTION	BOREHOLE DEPTH (m)	No. of Samples
NO.		Type	Thickness (mm)	Туре	Thickness (mm)			Taken
TH-01	UTM 14N: 5522364.9 m N, 632961.2 m E Eastbound lane near 1392 Pembina Hwy, 31.7 m northeast of Pembina Hwy, 1.7 m northwest of south curb	Asphalt	70	Granular Fill (Crushed Limestone, 20 mm)	1170	Clay	3.05	8
TH-02	UTM 14N: 5522393.5 m N, 633002.5 m E Westbound lane near 1 Kenneth St, 80.7 m northeast of Pembina Hwy, 6.8 m northwest of south curb	5522393.5 m N, 633002.5 m E Westbound lane near 1 Kenneth St, 80.7 m northeast of Pembina Hwy, 6.8 m northwest of					3.05	8
TH-03	UTM 14N: , 5522413.5 m N 633048.6 m E Eastbound lane near 2 Kenneth St, 131.7 m northeast of Pembina Hwy, 2.3 m northwest of south curb	Asphalt	85	Granular Fill (Crushed Limestone, 20 mm)	410	Clay	3.05	8
TH-04	UTM 14N: 5522442.1 m N, 633089.9 m E, Westbound lane in front of 959 Dowker Ave, 180.9 m northeast of Pembina Hwy, 7.2 m northwest of south curb	Asphalt	60	Granular Fill (Crushed Limestone, 20 mm)	560	Clay	1.52	5

TH-05	UTM 14N: 5522462.0 m N, 633136.0 m E Eastbound lane in front of 947 Dowker Ave, 232.0 m northeast of Pembina Hwy, 2.1 m northwest of south curb	Asphalt	130	Granular Fill (Crushed Limestone, 20 mm)	380	Clay	3.05	8
TH-06	UTM 14N: 5522490.6 m N, 633177.3 m E Westbound lane in front of 931 Dowker Ave, 280.0 m northeast of Pembina Hwy, 7.1 m northwest of south curb	Asphalt	100	Granular Fill (Crushed Limestone, 20 mm)	360	Clay, Silt, Clay	3.05	8
TH-07	UTM 14N: 5522510.5 m N, 633223.5 m E Eastbound lane in front of 919 Dowker Ave, 331.9 m northeast of Pembina Hwy, 2.0 m northwest of south curb	Asphalt	75	Granular Fill (Crushed Limestone, 20 mm)	250	Clay	3.05	8
TH-08	UTM 14N: 5522539.2 m N, 633264.7 m E Westbound lane in front of 909 Dowker Ave, 380.8 m northeast of Pembina Hwy, 7.3 m northwest of south curb	Asphalt	100	Granular Fill (Crushed Limestone, 20 mm)	360	Clay	3.05	8

Table 3-2 – Mulvey Street

TEST HOLE	TESTHOLE LOCATION		EMENT FACE	PAVEN STRUCTURE		SOIL DESCRIPTION	BOREHOLE DEPTH (m)	No. of Samples Taken	
NO.		Туре	Thickness (mm)	Туре	Thickness (mm)				
TH-09	UTM 14N: 5525512.6 m N, 632591.5 m E Eastbound lane in front of 775 Mulvey Ave, 26.5 m northeast of Lilac St, 1.6 m northwest of south curb	Asphalt & Concrete	50 & 200	Granular Fill (Crushed Limestone, 20 mm)	50	Fill, Clay, Silty Clay & Clay	3.05	8	
TH-10	UTM 14N: 5525512.6 m N, 632591.5 m E Westbound lane in front of 761 Mulvey Ave, 81.4 m northeast of Lilac St, 5.2 m northwest of south curb	Asphalt & Concrete	50 & 150	Granular Fill (Crushed Limestone, 20 mm)	30	Silt, Silty Clay, Clay	3.05	8	
TH-11	UTM 14N: 5525512.6 m N, 632591.5 m E Eastbound lane in front of 748 Mulvey Ave, 131.5 m northeast of Lilac St, 1.8 m northwest of south curb	Asphalt & Concrete	50 & 125	Granular Fill (Crushed Limestone, 20 mm)	20	Clay	3.05	8	

Table 3-3 – Maureen Street

PAVEMENT	PAVEMENT CORE LOCATION	PAVEMENT SURFACE				
CORE NO.		Type	Thickness (mm)			
PC-01	UTM 14N: 5525229.6 m N, 621888.3 m E Northbound lane near 3335 Assiniboine Ave, 16.0 m north of Assiniboine Ave, 1.8 m west of east curb	Concrete	150 (40 mm intact, 110 mm broken)			
PC-02	UTM 14N: 5525279.7 m N, 621886.8 m E Southbound lane in front of 118 Maureen St, 65.6 m north of Assiniboine Ave, 5.3 m west of east curb	Concrete	150			
PC-03	UTM 14N: 5525329.5 m N, 621892.5 m E Northbound lane in front of 130 Maureen St, 116.5 m north of Assiniboine Ave, 1.8 m west of east curb	Concrete	150			
PC-04	UTM 14N: 5525374.6 m N, 621890.8 m E Southbound lane in front of 142 Maureen St, 160.6 m north of Assiniboine Ave, 6.7 m west of east curb	Concrete	150			
PC-05	UTM 14N: 5525434.4 m N, 621897.0 m E Northbound lane in front of 158 Maureen St, 221.5 m north of Assiniboine Ave, 1.8 m west of east curb	Concrete	200			
PC-06	UTM 14N: 5525479.5 m N, 621895.3 m E Southbound lane in front of 170 Maureen St, 265.6 m north of Assiniboine Ave, 5.5 m west of east curb	Concrete	150			
PC-07	UTM 14N: 5525529.3 m N, 621901.0 m E Northbound lane near 197 Barron Dr, 316.5 m north of Assiniboine Ave, 1.9 m west of east curb	Concrete	150			

Table 3-4 – Whitegate Cresent

PAVEMENT	PAVEMENT CORE LOCATION	PAVEME	NT SURFACE
CORE NO.		Type	Thickness (mm)
PC-08	UTM 14N: 5525293.1 m N, 621918.4 m E	Concrete	150
	Eastbound lane in front of 103 Whitegates Cres, 26.2 m east of Maureen St, 1.9 m north of south curb		
PC-09	UTM 14N: 5525281.6 m N, 621972.3 m E	Concrete	150
	Westbound lane in front of 118 Whitegates Cres, 82.1 m east of Maureen St, 5.6 m north of south curb		
PC-10	UTM 14N: 5525266.0 m N, 622014.7 m E	Concrete	150
	Eastbound lane in front of 131 Whitegates Cres, 126.2 m east of Maureen St, 2.2 m north of south curb		
PC-11	UTM 14N: 552530.0 m N, 622038.9 m E	Concrete	150
	Southbound lane in front of 152 Whitegates Cres, 238.7 m south of Barron Dr, 1.8 m east of west curb		
PC-12	UTM 14N: 5525354.8 m N, 622044.9 m E	Concrete	150
	Northbound lane in front of 163 Whitegates Cres, 183.7 m south of Barron Dr, 6.0 m east of west curb		
PC-13	UTM 14N: 5525399.9 m N, 622043.4 m E	Concrete	150
	Southbound lane in front of 176 Whitegates Cres, 138.7 m south of Barron Dr, 1.9 m east of west curb		
PC-14	UTM 14N: 5525449.7 m N, 622049.2 m E	Concrete	150
	Southbound lane near 7 West Ave, 88.8 m south of Barron Dr, 5.6 m east of west curb		
PC-15	UTM 14N: 5525499.8 m N, 622047.8 m E	Asphalt &	100 & 150
	Southbound lane in front of 199 Whitegates Cres, 38.8 m south of Barron Dr, 2.1 m east of west curb	Concrete	

Table 3-5 - Ness Avenue

PAVEMENT	PAVEMENT CORE LOCATION	PAVEMENT SURFACE				
CORE NO.		Type	Thickness (mm)			
PC-16	UTM 14N: 5527592.6 m N, 623432.9 m E Eastbound lane near 15 Pebblewood Ln, 230.9 m west of Sturgeon Rd, 2.4 m north of south curb	Asphalt & Concrete	60 & 180			
PC-17	UTM 14N: 5527598.1 m N, 623478.0 m E Westbound lane in front of 3046 Ness Ave, 186.1 m west of Sturgeon Rd, 8.5 m north of south curb	Asphalt & Concrete	60 & 150			
PC-18	UTM 14N: 5527589.6 m N, 623527.9 m E Eastbound lane in front of 3034 Ness Ave, 135.9 m west of Sturgeon Rd, 1.7 m north of south curb	Asphalt & Concrete	75 & 180			
PC-19	UTM 14N: 5527599.9 m N, 632588.0 m E Westbound lane near 809 Setter St, 76.2m west of Sturgeon Rd, 15.7 m north of south curb	Asphalt & Concrete	100 & 180			
PC-20	UTM 14N: 5527588.5 m N, 623632.9 m E Eastbound lane near 809 Setter St, 30.9 m west of Sturgeon Rd, 4.7 m north of south curb	Asphalt & Concrete	90 & 180			

4 CLOSURE

The findings and recommendations provided in this report were prepared by WSP Canada Inc. (the Consultant) in accordance with generally accepted professional engineering principles and practices. The recommendations are based on the results of field and laboratory investigations and are reflective only of the actual test hole(s) and/or excavation(s) examined. If conditions encountered during construction appear to be different than those shown by the test hole(s) and/or excavation(s) at this site, the Consultant should be notified immediately in order that the recommendations can be reviewed and modified as necessary to address actual site conditions.

This report is limited in scope to only those items that are specifically referenced in this report. There may be existing conditions that were not recorded in this report. Such conditions were not apparent to the Consultant due to the limitations imposed by the scope of work. The Consultant, therefore, accepts no liability for any costs incurred by the Client for subsequent discovery, manifestation or rectification of such conditions.

This report is intended solely for the Client named as a general indication of the visible or reported physical condition of the items addressed in the report at the time of the geotechnical investigation. The material in this report reflects the Consultant's best judgment in light of the information available to it at the time of preparation.

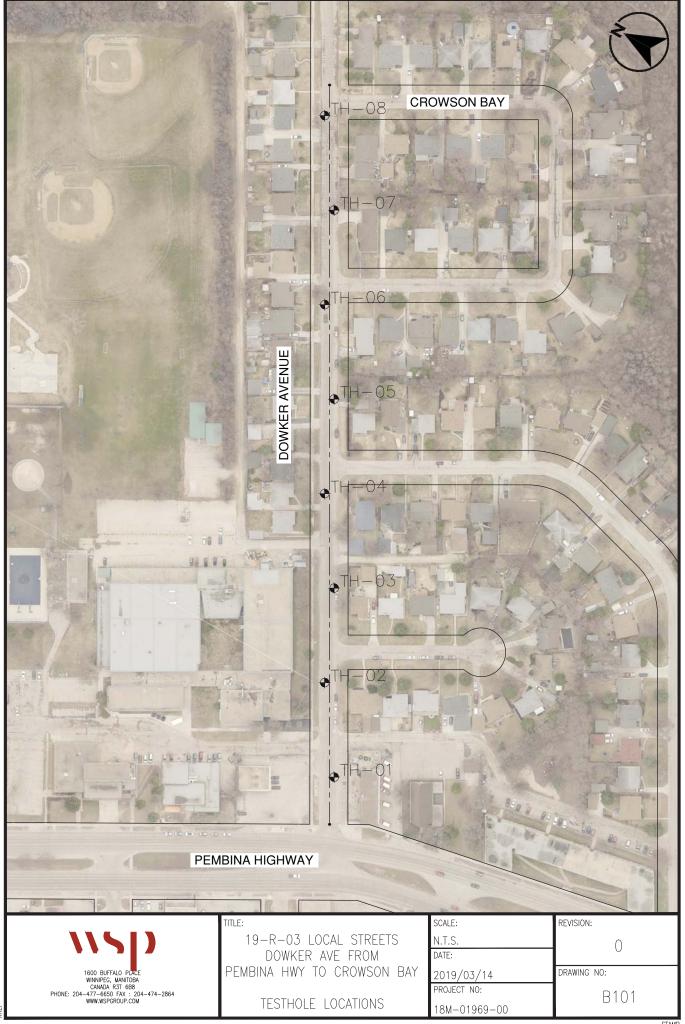
This report and the information and data contained herein are to be treated as confidential and may be used only by the Client and its officers and employees in relation to the specific project that it was prepared for. Any use a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. The Consultant accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

The report has been written to be read in its entirety, do not use any part of this report as a separate entity.

All files, notes, source data, test results and master files are retained by the Consultant and remain the property of the Consultant.

APPENDIX

A DOWKER AVE



STAMP

CLIENT _City of Winnipeg							
PROJECT NUMBER 18M-01969-00	PROJECT LOCATION _Dowker between Pembina/Crowson Bay (E Leg) _						
DATE STARTED 4/11/19 COMPLETED 4/11/19							
	GROUND WATER LEVELS:						
DRILLING METHOD Solid Stem Auger - B40 Truck Rig							
LOGGED BY Jason Dunn CHECKED BY Dana Bredin							
NOTES 632961.2 m E, 5522364.9 m N	AFTER DRILLING						
DEPTH (m)	SAMPLE TYPE NUMBER NU						
ASPHALT - 70mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down.	© GB S1 16 •						
1.0 98.78	My GB 9 ● My GB 83 8 ●						
CLAY - Brown, frozen, trace gravel, some silt Frost penetration at 1.68m below grade Moist, stiff below 1.68m.	07 GB 7						
	GB S7 65 •						
- Testhole ended at 3.05m below grade No seepage encountered Hole open to 2.29m below grade Testhole backfilled with bentonite and auger cutting	ngs.						

CLIENT				•		_				Street Renewals embina/Crowson Bay (E Leg)		
				1/19 COMPLETED 4/11/19 GRO						• • •		
					-							
				Dunn CHECKED BY Dana Bredin								
				E, 5522393.5 m N			G					
										▲ SPT N VALUE ▲		
DEPTH (m)	GRAPHIC LOG	ELEV. (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP Su (KPa) Torvane ## 50 100 150 200		
>	XX	99.93	\neg	ASPHALT - 70mm thick, intact.	$\overline{}$							
				GRANULAR FILL - Crushed limestone, 20mm down.		GB S1			10	•		
		99.39										
				CLAY - Brown, frozen, some silt - Frost penetration at 1.52m below grade Moist, stiff below 1.52m.		GB S2			23	•		
1.0						GB S3			34	•		
						GB S4			28	•		
1.5 						GB S5			22	•		
2.0						GB S6			36	•		
			Ā			GB S7			54	•		
- 2.5						GB S8			53	•		
3.0												
		96.95		- Testhole ended at 3.05m below grade. - Water measured at 2.13m below grade. - Hole open to 2.59m below grade. - Testhole backfilled with bentonite and auger cuttings.								

CLIENT _C	City of V	Vinr	ipeg	PROJECT	NAME _	19-R-03 - 0	Contrac	ct 1 - S	Street Renewals
			18M-01969-00						embina/Crowson Bay (E Leg)
	_		1/19 COMPLETED 4/11/19					HOLI	E SIZE 125mm
			Maple Leaf Drilling						
			Solid Stem Auger - B40 Truck Rig			LLING _2.4			
			Dunn CHECKED BY Dana Bredin						
NOTES 6	33048.	o m	E, 5522413.5 m N	AFIER	DRILLING	·			
DEPTH (m) GRAPHIC		WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP Su (kPa) Torvane
 	99.92		ASPHALT - 85mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down.		My GB				
0.5	99.54				S1			13	•
			CLAY - Brown to black, frozen, some silt Frost penetration at 1.52m below grade Moist, stiff below 1.52m.		GB S2			29	•
- 1.0					GB S3			32	•
- - - 1.5					GB S4			29	•
 					GB S5			36	•
2.0					GB S6			39	•
 		∇			♥ S7			42	•
2.5	96.95				GB S8			42	
			 Testhole ended at 3.05m below grade. Water measured at 2.44m below grade. Hole open to 2.74m below grade. Testhole backfilled with bentonite and auger cutting 	ngs.					

CLIENT City of Winnipeg					PROJECT NAME 19-R-03 - Contract 1 - Street Renewals						
PROJE	CT NU	IMBEI	R _	18M-01969-00	PROJEC	LOCA	TION Dowke	er betv	veen P	embina/Crowson Bay (E Leg)	
DATES	START	ED _	4/1	1/19 COMPLETED 4/11/19	GROUND ELEVATION 100 m HOLE SIZE 125mm						
DRILLI	NG CO	NTR/	VC.	FOR Maple Leaf Drilling	GROUND WATER LEVELS:						
DRILLI	NG ME	THO)	Solid Stem Auger - B40 Truck Rig	AT TIME OF DRILLING						
LOGGE	ED BY	Jas	on	Dunn CHECKED BY Dana Bredin	AT EN	OF DR	ILLING				
NOTES	633	089.9	m	E, 5522442.1 m N	AFTER	DRILLII	NG				
DEPTH (m)	GRAPHIC LOG	ELEV. (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (KPa)	MOISTURE CONTENT (%)	A SPT N VALUE A 20 40 60 80 PL MC LL 20 40 60 80 PP Su (kPa) Torvane	
		99.94		ASPHALT - 60mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down.		GB S1	-		13	•	
				CLAY - Brown, frozen, some silt Frost penetration at 1.52m below grade Moist, stiff below 1.52m.		GB S2			21	•	
1.0						GB S3			25	•	
- }						GB S4			42	•	
1.5		98.48		Tootholo anded at 1.52m below grade due to not	antial provimity	GB S5			44	•	
I				- Testhole ended at 1.52m below grade due to pote	endal proximity						

- No seepage or sloughing encountered.
 Testhole backfilled with bentonite and auger cuttings.

CLIEN	T _Ci	ty of V	√inr	nipeg	PROJEC1	NAME	19-R-03 - 0	Contrac	ct 1 - S	Street Renewals
PROJE	ECT N	UMBE	R	18M-01969-00	PROJEC1	LOCATI	ON Dowke	er betw	een P	embina/Crowson Bay (E Leg)
DATE	STAR	TED _	4/1	1/19 COMPLETED 4/11/19	GROUND ELE	VATION	100 m		HOLI	E SIZE 125mm
DRILL	ING C	ONTR	AC	TOR Maple Leaf Drilling	GROUND WA	TER LEV	ELS:			
DRILL	ING M	ETHO	D _	Solid Stem Auger - B40 Truck Rig	AT TIM	E OF DRI	LLING			
LOGG	ED BY	_Jas	on	Dunn CHECKED BY Dana Bredin	AT END	OF DRII	LLING			
NOTES	S 63	3136.0) m	E, 5522462.0 m N	AFTER	DRILLIN	G			
DEPTH (m)	GRAPHIC LOG	ELEV. (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP Su (kPa) Torvane PP 50 100 150 200
		99.87		ASPHALT - 130mm thick, intact.						
 0.5		99.54		GRANULAR FILL - Crushed limestone, 20mm down.		GB S1			12	•
 				- CLAY, sandy, silty, trace gravel, brown, frozen.- 36.3% clay, 31.2% sand, 27.4% silt, 5.1% gravel a	t 0.9 m	GB S2			28	•
 1.0						m GB			41	⊢ ●───-1
		98.93		CLAY		S3				
 				- Brown, some silt Frost penetration at 1.68m below grade Moist, stiff below 1.68m.		GB S4			34	•
1.5 						GB S5			32	•
2.0						GB S6			38	•
 2.5						GB S8			36	•
3.0		96.95		- Testhole ended at 3.05m below grade.						
				No seepage or sloughing encountered. Testhole backfilled with bentonite and auger cuttin	gs.					

CLIENT City of Winnipeg	PROJECT NAME 19-R-03 - Contract 1 - Street Renewals
PROJECT NUMBER 18M-01969-00	PROJECT LOCATION Dowker between Pembina/Crowson Bay (E Leg)
DATE STARTED 4/11/19 COMPLETED 4/11/19	
	GROUND WATER LEVELS:
DRILLING METHOD Solid Stem Auger - B40 Truck Rig LOGGED BY Jason Dunn CHECKED BY Dana Bredin	
NOTES 633177.3 m E, 5522490.6 m N	
NOTES _033177.5111 E, 3322490.0111 N	AFTER DRILLING
MATERIAL DESCRIPTION (m) (m) (m) (m) (m) (m) (m) (m) (m) (m	SAMPLE TYPE NUMBER TYPE NUMBER TYPE NUMBER TYPE SAMPLE TYPE NUMBER TYPE SAMPLE TYPE SAMPLE TYPE SOUTH TYPE SO
99.90 ASPHALT - 100mm thick, intact.	
GRANULAR FILL - Crushed limestone, 20mm down.	© GB 11 •
CLAY - Brown, frozen, some silt, trace gravel Frost penetration at 1.52m below grade Moist, stiff below 1.52m.	© GB S2 ■ 32 ●
- 1.0 	6B S3 ●
	© GB S4 31 ●
	60 S5 32 ●
98.02 SILT - 197.87 - Tan-brown, moist, soft, some clay, trace sand.	6B S6 43 ●
CLAY - Brown, moist, stiff.	60 GB S7 48 ●
2.5	6B S8 52 •
- Testhole ended at 3.05m below grade No seepage or sloughing encountered Testhole backfilled with bentonite and auger cutting	gs.

CLIEN.				•		_				Street Renewals
				18M-01969-00						embina/Crowson Bay (E Leg)
				1/19					HOL	E SIZE 125mm
				TOR Maple Leaf Drilling						
				Solid Stem Auger - B40 Truck Rig						
				Dunn CHECKED BY Dana Bredin						
NOTES	03.	3223.	ווו כ	E, 5522510.5 m N	AFIER	DRILLING	<u></u>			
DEPTH (m)	GRAPHIC LOG	(m) (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	PL MC LL 20 40 60 80 PL MC LL 20 40 60 80 PP SU (kPa) Torvane
		99.95	\neg	ASPHALT - 75mm thick, intact.						
				GRANULAR FILL	/					
 _ 0.5		99.70		- Crushed limestone, 20mm down. CLAY - Brown to black, frozen, some silt Frost penetration at 1.52m below grade Moist, stiff below 1.52m.		GB S1			10	•
 				- Moist, still below 1.52m.		GB S2			30	•
 1.0 						GB S3			37	•
 						GB S4			36	•
1.5 						GB S5			37	•
 2.0						GB S6			44	•
 						GB S7			51	•
2.5 						GB S8			51	•
3.0		96.95								
				 Testhole ended at 3.05m below grade. No seepage or sloughing encountered. Testhole backfilled with bentonite and auger cuttir 	gs.					· · ·

CLIEN										Street Renewals
				18M-01969-00						embina/Crowson Bay (E Leg)
				1/19 COMPLETED 4/11/19					HOL	E SIZE 125mm
				Maple Leaf Drilling						
				Solid Stem Auger - B40 Truck Rig						
				Dunn CHECKED BY Dana Bredin						
NOTE	63	3264.	m	E, 5522539.2 m N	_ AFIER	DRILLIN	G			
DEPTH (m)	GRAPHIC LOG	ELEV. (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP SU (KPa) Torvane 50 100 150 200
 0.5		99.90 99.54		ASPHALT - 100mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down. CLAY		GB S1			6	•
 				 Brown to black, frozen, some silt. Frost penetration at 1.68m below grade. Moist, stiff below 1.68m. 		GB S2			35	•
1.0 						GB S3 GB S4			37 40	•
1.5 						GB S5			41	•
2.0						GB S6			46	•
 						GB S7			52	•
2.5 3.0		00.05				GB S8			54	•
		96.95		Testhole ended at 3.05m below grade. No seepage or sloughing encountered. Testhole backfilled with bentonite and auger cutt	ings.			ı		<u> </u>



Figure 1 – TH-01 Dowker Ave



Figure 2 – TH-02 Dowker Ave



Figure 3 – TH-03 Dowker Ave



Figure 4 – TH-04 Dowker Ave



Figure 5 – TH-05 Dowker Ave



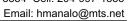
Figure 6 – TH-06 Dowker Ave



Figure 7 – TH-07 Dowker Ave

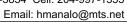


Figure 8 – TH-08 Dowker Ave



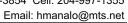


CLIENT: WSP Canada	Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906
PROJECT: 18M-01969-00) - Phase 802-1	DATE SAMPLED	: 8-Apr-2019	SAMPLED BY: Client	
PROJECT CONTACT:	Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh
TEST LOCATION:	Phase 802-1				
Description	TH 1	TH 1	TH 1	TH 1	TH 1
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	151.10	153.40	158.10	156.00	153.90
Wt Dry Sample + Tare	131.40	141.00	147.30	146.40	128.50
Wt Water	19.70	12.40	10.80	9.60	25.40
Wt Tare	4.60	4.40	4.30	4.20	4.30
Wt Dry Sample	126.80	136.60	143.00	142.20	124.20
Moisture Content (%)	15.5	9.1	7.6	6.8	20.5
Description	TH 1	TH 1	TH 1		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	134.10	123.60	130.60		
Wt Dry Sample + Tare	92.20	85.30	81.00		
Wt Water	41.90	38.30	49.60		
Wt Tare	4.20	4.20	4.30		
Wt Dry Sample	88.00	81.10	76.70		
Moisture Content (%)	47.6	47.2	64.7		
Description	TH 2	TH 2	TH 2	TH 2	TH 2
Sample	S1	S2	S 3	S4	S 5
Wt Wet Sample + Tare	164.90	158.90	126.10	124.20	124.10
Wt Dry Sample + Tare	151.00	130.00	95.40	98.30	102.50
Wt Water	13.90	28.90	30.70	25.90	21.60
Wt Tare	4.20	4.20	4.20	4.10	4.10
Wt Dry Sample	146.80	125.80	91.20	94.20	98.40
Moisture Content (%)	9.5	23.0	33.7	27.5	22.0
Description	TH 2	TH 2	TH 2		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	126.70	123.50	128.70		
Wt Dry Sample + Tare	94.50	81.60	85.70		
Wt Water	32.20	41.90	43.00		
Wt Tare	4.20	4.20	4.30		
Wt Dry Sample	90.30	77.40	81.40		
Moisture Content (%)	35.7	54.1	52.8		





CLIENT: WSP Canada	Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906
PROJECT: 18M-01969-0	0 - Phase 802-1	DATE SAMPLED	: 8-Apr-2019	SAMPLED BY:	Client
PROJECT CONTACT:	Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh
TEST LOCATION:	Phase 802-1				
Description	TH 3	TH 3	TH 3	TH 3	TH 3
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	174.60	134.60	124.00	121.90	126.10
Wt Dry Sample + Tare	154.80	105.40	94.80	95.20	94.10
Wt Water	19.80	29.20	29.20	26.70	32.00
Wt Tare	4.60	4.40	4.60	4.30	4.20
Wt Dry Sample	150.20	101.00	90.20	90.90	89.90
Moisture Content (%)	13.2	28.9	32.4	29.4	35.6
Description	TH 3	TH 3	TH 3		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	126.80	125.10	127.10		
Wt Dry Sample + Tare	92.50	89.50	91.10		
Wt Water	34.30	35.60	36.00		
Wt Tare	4.20	4.40	4.70		
Wt Dry Sample	88.30	85.10	86.40		
Moisture Content (%)	38.8	41.8	41.7		
Description	TH 4	TH 4	TH 4	TH 4	TH 4
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	163.10	151.40	126.90	121.90	124.10
Wt Dry Sample + Tare	145.10	126.10	102.30	87.40	87.50
Wt Water	18.00	25.30	24.60	34.50	36.60
Wt Tare	4.50	4.20	4.30	4.30	4.30
Wt Dry Sample	140.60	121.90	98.00	83.10	83.20
Moisture Content (%)	12.8	20.8	25.1	41.5	44.0
Description	TH 5	TH 5	TH 5	TH 5	TH 5
Sample	S1	S2	S 3	S4	S5
Wt Wet Sample + Tare	164.80	128.30	441.90	121.20	123.00
Wt Dry Sample + Tare	147.40	101.00	317.70	91.50	94.50
Wt Water	17.40	27.30	124.20	29.70	28.50
Wt Tare	4.30	4.60	14.20	4.50	4.10
Wt Dry Sample	143.10	96.40	303.50	87.00	90.40
Moisture Content (%)	12.2	28.3	40.9	34.1	31.5





CLIENT: WSP Canada	Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906
PROJECT: 18M-01969-00) - Phase 802-1	DATE SAMPLED	: 8-Apr-2019	SAMPLED BY: Client	
PROJECT CONTACT:	Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh
TEST LOCATION:	Phase 802-1				
Description	TH 5	TH 5			
Sample	S6	S8			
Wt Wet Sample + Tare	123.20	127.80			
Wt Dry Sample + Tare	90.50	95.20			
Wt Water	32.70	32.60			
Wt Tare	4.20	4.30			
Wt Dry Sample	86.30	90.90			
Moisture Content (%)	37.9	35.9			
Description	TH 6	TH 6	TH 6	TH 6	TH 6
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	158.10	124.50	127.60	122.30	126.10
Wt Dry Sample + Tare	143.40	95.50	95.70	94.40	96.50
Wt Water	14.70	29.00	31.90	27.90	29.60
Wt Tare	4.40	4.30	4.30	4.50	3.90
Wt Dry Sample	139.00	91.20	91.40	89.90	92.60
Moisture Content (%)	10.6	31.8	34.9	31.0	32.0
Description	TH 6	TH 6	TH 6		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	121.20	128.80	120.10		
Wt Dry Sample + Tare	86.30	88.60	80.40		
Wt Water	34.90	40.20	39.70		
Wt Tare	4.20	4.20	4.20		
Wt Dry Sample	82.10	84.40	76.20		
Moisture Content (%)	42.5	47.6	52.1		
Description	TH 7	TH 7	TH 7	TH 7	TH 7
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	179.40	127.30	125.60	122.00	127.60
Wt Dry Sample + Tare	163.30	98.80	93.10	91.00	94.60
Wt Water	16.10	28.50	32.50	31.00	33.00
Wt Tare	4.20	4.20	4.20	4.30	4.20
Wt Dry Sample	159.10	94.60	88.90	86.70	90.40
Moisture Content (%)	10.1	30.1	36.6	35.8	36.5





CLIENT: WSP Canada	Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906
PROJECT: 18M-01969-0	0 - Phase 802-1	DATE SAMPLED:	8-Apr-2019	SAMPLED BY:	Client
PROJECT CONTACT:	Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh
TEST LOCATION:	Phase 802-1				
Description	TH 7	TH 7	TH 7		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	121.90	123.50	124.10		
Wt Dry Sample + Tare	85.90	83.20	83.90		
Wt Water	36.00	40.30	40.20		
Wt Tare	4.20	4.20	4.30		
Wt Dry Sample	81.70	79.00	79.60		
Moisture Content (%)	44.1	51.0	50.5		
Description	TH 8	TH 8	TH 8	TH 8	TH 8
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	154.10	121.70	122.10	125.60	124.00
Wt Dry Sample + Tare	145.30	91.40	90.60	90.80	89.30
Wt Water	8.80	30.30	31.50	34.80	34.70
Wt Tare	4.20	4.20	4.30	4.30	4.20
Wt Dry Sample	141.10	87.20	86.30	86.50	85.10
Moisture Content (%)	6.2	34.7	36.5	40.2	40.8
Description	TH 8	TH 8	TH 8		
Sample	S6	S7	S8		
Wt Wet Sample + Tare	122.30	128.00	124.60		
Wt Dry Sample + Tare	85.30	85.60	82.20		
Wt Water	37.00	42.40	42.40		
Wt Tare	4.20	4.30	4.20		
Wt Dry Sample	81.10	81.30	78.00		
Moisture Content (%)	45.6	52.2	54.4		



H. MANALO CONSULTING LTD.

1402 Notre Dame Avenue, Winnipeg, MB R3E 3G5 Phone: 204 697 3854 Cell: 204 997-1355

hmanalo@mts.net

PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT: WSP PROJECT NO. 103-1906

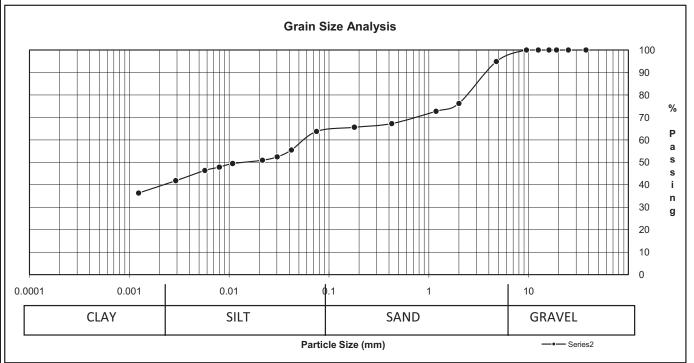
1600 Buffalo Place Test No:

Winnipeg, MB R3T 6B8 Lab No: HM 48-1P

ATTENTION: Dana Bredin

PROJECT: 18M-01969-00 Phase 802-1

1 1100201.	10101 0 1000	00111000 002 1					
Date Sampled:	17-Apr-19	Date Received: 17-Apr-19	Sieve Ar	nalysis	Hydrometer Analysis		
Sampled By:	Client	Date Tested: 18-Apr-19	Sieve (mm)	% Passing	Diameter	% Finer	
			50.00	100.0			
			37.50	100.0			
			25.00	100.0			
			19.00	100.0			
			16.00	100.0			
Material Identific	cation		12.50	100.0	0.0420	55.5	
B.H./T.H. No.		TH 5, S3	9.50	100.0	0.0302	52.4	
Sample No.		HM 48-1P	4.75	94.9	0.0215	50.9	
Sample Source		Various City Street	2.00	76.2	0.0109	49.4	
Specific Gravity	of Material:	2.65	1.18	72.7	0.0080	47.9	
			0.425	67.2	0.0057	46.3	
			0.180	65.6	0.0029	41.8	
			0.075	63.7	0.0012	36.3	



SOIL DESCRIPTION	% Composition		D10	
SOIL DESCRIPTION	5.1	Gravel	D30	
	31.2	Sand	D60	0.05500
	27.4	Silt	Cu	
	36.3	Clay	Сс	

Remarks: Test Method: ASTM D422, D2216, D4318

Technician: Navi

Honoralo

Reviewed by: Hermie Manalo



H. MANALO CONSULTING LTD.

1402 Notre Dame Avenue, Winnipeg, MB R3E 5 PHONE: 204 697-3854 CELL: 204 997-1355

hmanalo@mts.net

ATTERBERG LIMITS

CLIENT: WSP Canada Group Limited PROJECT NO.: 103-1906

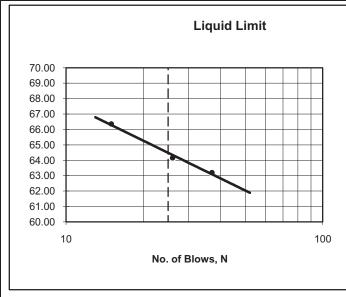
1600 Buffalo Place TEST NO.: 1

Winnipeg, MB R3T 6B8 LAB NO.: HM 48-1

ATTENTION: Dana Bredin

PROJECT: 18M-01969-00 Phase 802-1

Liquid Limit Determination								
Dish No.:	1	2	3		Liquid Limit			
Wet Soil + Dish:	14.82	14.7	13.11		25 Blows			
Dry Soil + Dish:	10.75	10.62	9.6					
Moisture:	4.07	4.08	3.51					
Dish:	4.31	4.26	4.31					
Dry Soil:	6.44	6.36	5.29					
% Moisture:	63.20	64.15	66.35					
No. of Blows:	37	26	15					
Liquid Limits:	66.27	64.46	62.37		64			



Material Identification:

T.H. No. **TH 5, S3**

Depth: 3'

Liquid Limit, %: 64
Plastic Limit, %: 32
Plasticity Index: 32

(LL-PL)

Dish No.:	1	2	3		
Wet Soil + Dish:	6.81	5.68	6.18		
Dry Soil + Dish:	6.2	5.32	5.72		
Moisture:	0.61	0.36	0.46		
Dish:	4.26	4.23	4.25		
Dry Soil:	1.94	1.09	1.47		
% Moisture:	31.44	33.03	31.29		
Average:					32

Test Method: ASTM: D4318, D2216

HMCL Tech: Navi
Date Tested: 23-Apr-19

Reviewed by: Hermie Manalo

Spraralo

APPENDIX

B MULVEY AVE



CLIENT City of Win	nipeg	_ PROJEC	Г NAME	19-R-03 - 0	Contract	1 - Street	Renewals
PROJECT NUMBER			T LOCAT	ION Mulve	y betwee	en Lilac/Ar	buthnot
DATE STARTED 4/	8/19 COMPLETED 4/8/19	_ GROUND ELI	EVATION	100 m	i	HOLE SIZI	E _125 mm
DRILLING CONTRAC	TOR Maple Leaf Drilling	_ GROUND WA	TER LEV	/ELS:			
DRILLING METHOD	Solid Stem Auger - B40 Truck Rig	AT TIM	E OF DR	ILLING			
LOGGED BY _Jasor	Dunn CHECKED BY Dana Bredin						
NOTES 632591.5 n	n E, 5525512.6 m N	AFTER	DRILLIN	IG			
							▲ SPT N VALUE ▲
DEPTH (m) GRAPHIC LOG ELEV. (m) WATER LEVEI	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	CONTENT	20 40 60 80 PL MC LL 20 40 60 80 PP Su (KPa) Torvane 50 100 150 200
99.95	ASPHALT						50 100 150 200
0.5 99.75 0.5 99.75 0.5 99.75 0.6 99.75 98.48	- 50mm thick, broken. CONCRETE - 200mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down. FILL - Clayey, black, frozen. CLAY - Grey, frozen, trace sand, some silt Frost penetration to 1.52m below grade. - 69.7% clay, 26.0% silt, 4.3% sand. SILT - Tan-brown, moist, soft, some clay. CLAY - Brown, moist, stiff, trace gravel.		GB S3 GB S4 GB S4 GB S5 GB S6 GB S7 GB S7			33 35 33 33 35 43 46 50	
GENERAL BH PLOTS - WSP 19-R-03-C1 - MULVEY.GPJ GINT STD CANADA.GDT 3-C1 - MULVEY.GPJ GINT STD GINT ST	- Testhole ended at 3.05m below grade No seepage or sloughing encountered Test hole backfilled with bentonite and auger cut	tings.					

WSP 1600 Buffalo Place Winnipeg, MB R3T 6B8 Telephone: (204)-477-6650

GENERAL BH PLOTS - WSP 19-R-03-C1 - MULVEY.GPJ GINT STD CANADA.GDT 4/29/19

		nipeg 18M-01969-00	PROJECT NAME 19-R-03 - Contract 1 - Street Renewals PROJECT LOCATION Mulvey between Lilac/Arbuthnot							
DATE STA	ARTED	4/8	completed 4/8/19	GROUND ELEVATION 100 m HOLE SIZE 125 mm						
DRILLING	AC	TOR Maple Leaf Drilling								
DRILLING	METHO	D .	Solid Stem Auger - B40 Truck Rig	AT TIM	IE OF DRI	LLING				
LOGGED	BY Ja	son	Dunn CHECKED BY Dana Bredin	_ AT EN	OF DRII	LING				
NOTES _	632635.	1 m	E, 5525540.6 m N	AFTER	DRILLIN	G				
DEPTH (m)	LOG (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP Su (kPa) Torvane ## 50 100 150 200	
0.5	99.95	$ \ \ $	ASPHALT - 50mm thick, intact. CONCRETE - 150mm thick, intact. GRANULAR FILL - Crushed limestone, 20mm down. SILT - Tan-brown, frozen, some clay Trace organics above 0.5m.		GB S1			36	•	
1.0	98.78				GB S3			17	•	
			SILTY CLAY - Brown, frozen, some silt Frost penetration to 1.52m below grade Moist, firm below 1.52m Silty below 1.98m.		S4 M GB S5			33		
2.0	97.71	.71 CLAY			S6 GB S7			45	•	
2.5	96.95		- Brown, moist, stiff, some silt.		M GB S8			47		
			 Testhole ended at 3.05m below grade. No seepage or sloughing encountered. Test hole backfilled with bentonite and auger cutt 	dings.						

PAGE 1 OF 1

WSP 1600 Buffalo Place Winnipeg, MB R3T 6B8 Telephone: (204)-477-6650

GENERAL BH PLOTS - WSP 19-R-03-C1 - MULVEY.GPJ GINT STD CANADA.GDT 4/29/19

CLIEN	T Cit	ty of V		nipeg	PROJECT	Г NAME	19-R-03 - (Contra	ct 1 - S	Street Renewals	
		-		18M-01969-00	PROJECT LOCATION Mulvey between Lilac/Arbuthnot						
DATE STARTED 4/8/19 COMPLETED 4/8/19					GROUND ELEVATION 100 m HOLE SIZE 125 mm						
DRILLI	NG C	ONTR	AC	TOR Maple Leaf Drilling							
DRILLI	NG M	ETHO	D _	Solid Stem Auger - B40 Truck Rig	$_{-}$ $_{ar{ar{ar{ar{ar{ar{ar{ar{ar{ar$	E OF DRI	LLING 1.8	33 m /	Elev 9	8.17 m	
LOGG	ED BY	/ Jas	on	Dunn CHECKED BY Dana Bredin	AT EN	OF DRII	LING				
NOTES	63	2683.	5 m	E, 5525563.1 m N	AFTER	DRILLIN	G				
DEPTH (m)	GRAPHIC LOG	ELEV. (m)	WATER LEVEL	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	POCKET PEN. (kPa)	MOISTURE CONTENT (%)	20 40 60 80 PL MC LL 20 40 60 80 PP SU (kPa) Torvane 50 100 150 200	
	0 4 4	99.95	$ \cdot $	ASPHALT - 50mm thick, intact.							
		99.83 99.78		CONCRETE - 125mm thick, intact.							
 0.5				GRANULAR FILL - Crushed limestone, 20mm down.		GB S1			40	•	
				CLAY - Brown, frozen, some silt, trace sand and gravel.							
 				Frost penetration to 1.37m below grade.Moist, stiff below 1.37m.Silty at 2.59m.		GB S2			40	•	
1.0						GB S3			33	•	
 						GB S4			34	•	
 1.5						m, GB			42	•	
- 			∇			S5					
2.0			-			GB S6			45	•	
 						GB S7			46	•	
2.5						GB S8			40	•	
- - -											
3.0											
		96.95		- Testhole ended at 3.05m below grade. - Water measured at 1.83m below grade. - Hole open to 2.29m below grade. - Test hole backfilled with bentonite and auger cut	ings.					<u> </u>	



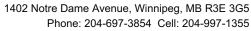
Figure 1 – TH-09 Mulvey Ave



Figure 2 – TH-10 Mulvey Ave



Figure 3 – TH-11 Mulvey Ave (asphalt not shown)



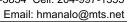
Email: hmanalo@mts.net



MOISTURE CONTENT OF SOIL (ASTM D2216)

CLIENT: WSP Canada Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906
PROJECT: 18M-01969-00 - Phase 802-1	DATE SAMPLED:	8-Apr-2019	SAMPLED BY:	Client
PROJECT CONTACT: Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh
TEST LOCATION: Phase 802-1				

Description	TH 9				
Sample	S1	S2	S3	S4	S5
Wt Wet Sample + Tare	149.20	241.50	343.70	126.80	122.90
Wt Dry Sample + Tare	113.60	183.30	262.40	96.30	92.50
Wt Water	35.60	58.20	81.30	30.50	30.40
Wt Tare	4.20	14.50	13.80	4.10	4.30
Wt Dry Sample	109.40	168.80	248.60	92.20	88.20
Moisture Content (%)	32.5	34.5	32.7	33.1	34.5





MOISTURE CONTENT OF SOIL (ASTM D2216)

CLIENT: WSP Canada	Group Limited	TEST NO:	19- 001	PROJECT NO:	103-1906		
PROJECT: 18M-01969-00) - Phase 802-1	DATE SAMPLED:	8-Apr-2019	SAMPLED BY:	Client		
PROJECT CONTACT: Dana Bredin DATE TESTED: 18-Apr-2019 TESTED BY: Viet Linh							
TEST LOCATION:	Phase 802-1						
Description	TH 9	TH 9	TH 9				
Sample	S6	S7	S8				
Wt Wet Sample + Tare	122.50	122.20	128.70				
Wt Dry Sample + Tare	86.70	85.10	87.00				
Wt Water	35.80	37.10	41.70				
Wt Tare	4.20	4.10	4.20				
Wt Dry Sample	82.50	81.00	82.80				
Moisture Content (%)	43.4	45.8	50.4				
Description	TH-10	TH-10	TH-10	TH-10	TH-10		
Sample	S1	S2	S 3	S4	S 5		
Wt Wet Sample + Tare	130.80	122.90	124.00	122.80	126.00		
Wt Dry Sample + Tare	97.60	102.20	106.30	104.50	95.80		
Wt Water	33.20	20.70	17.70	18.30	30.20		
Wt Tare	4.20	4.20	4.60	4.30	4.20		
Wt Dry Sample	93.40	98.00	101.70	100.20	91.60		
Moisture Content (%)	35.5	21.1	17.4	18.3	33.0		
Description	TH-10	TH-10	TH-10				
Sample	S6	S 7	S8				
Wt Wet Sample + Tare	120.40	125.30	123.90				
Wt Dry Sample + Tare	87.20	87.60	85.40				
Wt Water	33.20	37.70	38.50				
Wt Tare	5.90	4.30	4.20				
Wt Dry Sample	81.30	83.30	81.20				
Moisture Content (%)	40.8	45.3	47.4				
Description	TH-11	TH-11	TH-11	TH-11	TH-11		
Sample	S1	S2	S3	S4	S5		
Wt Wet Sample + Tare	128.60	126.60	123.50	123.30	121.10		
Wt Dry Sample + Tare	92.90	91.50	93.80	93.30	86.60		
Wt Water	35.70	35.10	29.70	30.00	34.50		
Wt Tare	4.20	4.30	4.10	4.20	4.10		
Wt Dry Sample	88.70	87.20	89.70	89.10	82.50		
Moisture Content (%)	40.2	40.3	33.1	33.7	41.8		





MOISTURE CONTENT OF SOIL (ASTM D2216)

CLIENT: WSP Canada 0	TEST NO:	19- 001	PROJECT NO:	103-1906					
PROJECT: 18M-01969-00	- Phase 802-1	DATE SAMPLED:	8-Apr-2019	SAMPLED BY:	Client				
PROJECT CONTACT:	Dana Bredin	DATE TESTED:	18-Apr-2019	TESTED BY:	Viet Linh				
TEST LOCATION:	TEST LOCATION: Phase 802-1								
Description	TH-11	TH-11	TH-11						
Sample	S6	S7	S8						
Wt Wet Sample + Tare	121.70	123.90	127.40						
Wt Dry Sample + Tare	85.30	86.00	92.20						
Wt Water	36.40	37.90	35.20						
Wt Tare	4.20	4.20	4.20						
Wt Dry Sample	81.10	81.80	88.00						
Moisture Content (%)	44.9	46.3	40.0						



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PARTICLE SIZE ANALYSIS OF SOILS TEST REPORT

CLIENT: WSP PROJECT NO. 103-1906

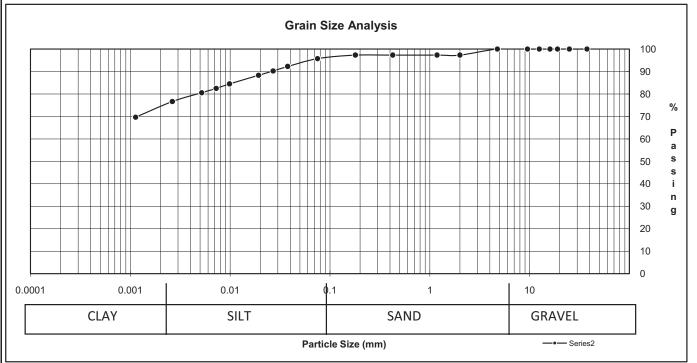
1600 Buffalo Place Test No: 2

Winnipeg, MB R3T 6B8 Lab No: HM 48-2P

ATTENTION: Dana Bredin

PROJECT: 18M-01969-00 Phase 802-1

1100201:	10101 0 1000	00 1 11030 002 1					
Date Sampled: 17-Apr-19		Date Received: 17-Apr-19	Sieve Ar	nalysis	Hydrometer Analysis		
Sampled By:	Client	Date Tested: 18-Apr-19	Sieve (mm)	% Passing	Diameter	% Finer	
			50.00	100.0			
			37.50	100.0			
			25.00	100.0			
			19.00	100.0			
			16.00	100.0			
Material Identific	ation		12.50	100.0	0.0378	92.2	
B.H./T.H. No.		TH 9, S2	9.50	100.0	0.0270	90.3	
Sample No.		HM 48-2P	4.75	100.0	0.0193	88.3	
Sample Source		Various City Street	2.00	97.3	0.0098	84.4	
Specific Gravity o	f Material:	2.65	1.18	97.3	0.0073	82.5	
			0.425	97.3	0.0052	80.5	
			0.180	97.3	0.0026	76.6	
			0.075	95.7	0.0011	69.7	



SOIL DESCRIPTION	% Composition		D10	
SOIL DESCRIPTION		Gravel	D30	
	4.3	Sand	D60	
	26.0	Silt	Cu	
	69.7	Clav	Сс	

Remarks: Test Method: ASTM D422, D2216, D4318

Technician: Navi

Reviewed by: Hermie Manalo

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ATTERBERG LIMITS

CLIENT: WSP Canada Group Limited PROJECT NO.: 103-1906

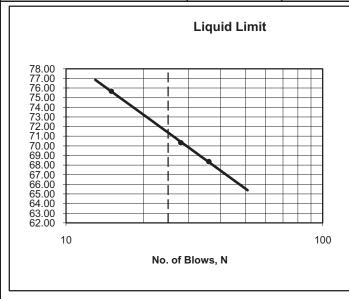
1600 Buffalo Place TEST NO.: 2

Winnipeg, MB R3T 6B8 LAB NO.: HM 48-2

ATTENTION: Dana Bredin

PROJECT: 18M-01969-00 Phase 802-1

Liquid Limit Determination								
Dish No.:	1	2	3		Liquid Limit			
Wet Soil + Dish:	13	11.28	13.47		25 Blows			
Dry Soil + Dish:	9.5	8.46	9.52					
Moisture:	3.5	2.82	3.95					
Dish:	4.38	4.45	4.3					
Dry Soil:	5.12	4.01	5.22					
% Moisture:	68.36	70.32	75.67					
No. of Blows:	36	28	15					
Liquid Limits:	71.44	71.30	71.13		71			



Material Identification:

T.H. No. **TH 9, S2**

Depth: 2'

Liquid Limit, %: 71
Plastic Limit, %: 31
Plasticity Index: 40

(LL-PL)

Plastic Limit Determination							
Dish No.:	1	2	3				
Wet Soil + Dish:	7.2	6.03	5.54				
Dry Soil + Dish:	6.49	5.61	5.22				
Moisture:	0.71	0.42	0.32				
Dish:	4.19	4.19	4.22				
Dry Soil:	2.3	1.42	1				
% Moisture:	30.87	29.58	32.00				
Average:					31		

Test Method: ASTM: D4318, D2216

HMCL Tech: Navi
Date Tested: 23-Apr-19

Reviewed by: Hermie Manalo

Spraralo

APPENDIX

C MAUREEN ST



XREF



Figure 1 – PC-01 Maureen St (broken concrete shown, actual core size 150 mm)



Figure 2 – PC-02 Maureen St



Figure 3 – PC-03 Maureen St



Figure 4 – PC-04 Maureen St



Figure 5 – PC-05 Maureen St



Figure 6 – PC-06 Maureen St



Figure 7 – PC-07 Maureen St

APPENDIX

D WHITEGATE CRES

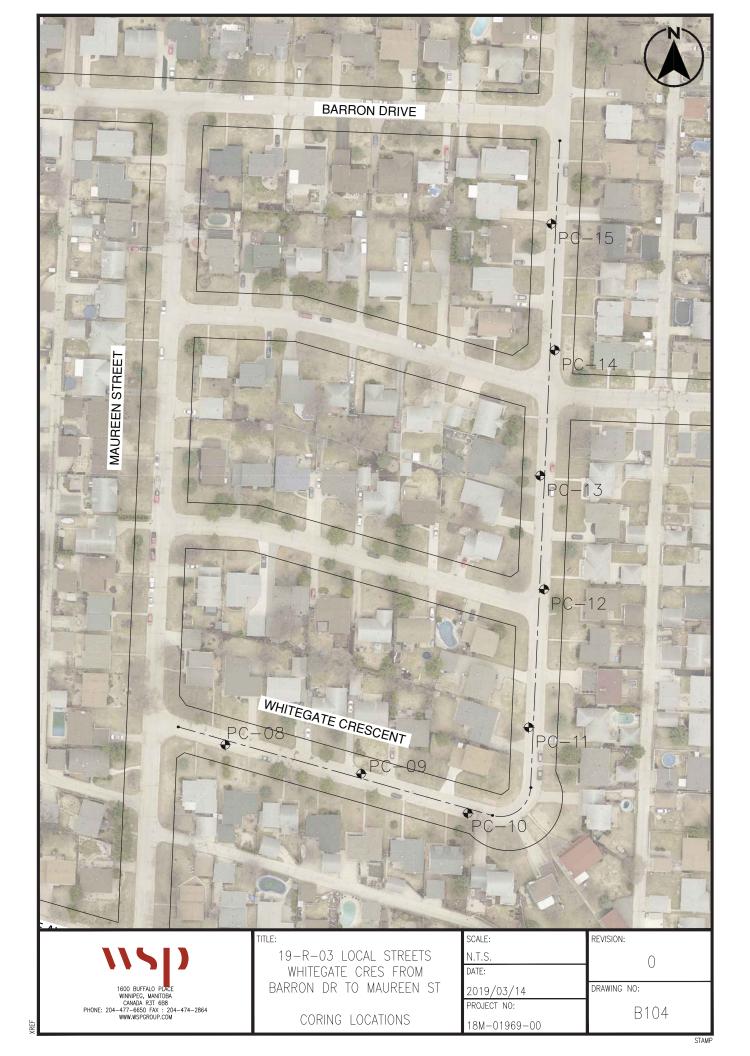




Figure 1 – PC-08 Whitegate Cres

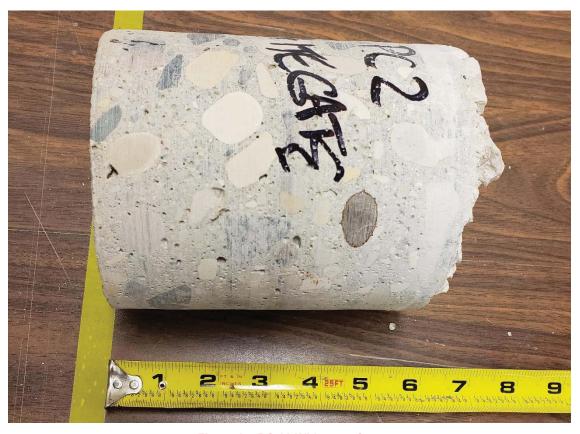


Figure 2 – PC-09 Whitegate Cres

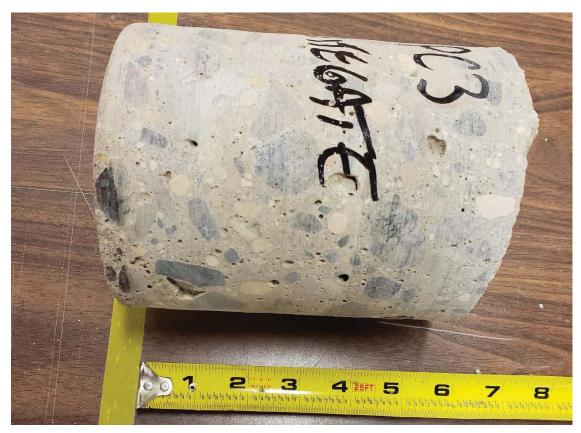


Figure 3 – PC-10 Whitegate Cres



Figure 4 – PC-11 Whitegate Cres



Figure 5 – PC-12 Whitegate Cres



Figure 6 – PC-13 Whitegate Cres



Figure 7 – PC-14 Whitegate Cres



Figure 8 – PC-15 Whitegate Cres

APPENDIX

E NESS AVE





Figure 1 – PC-16 Ness Ave



Figure 2 – PC-17 Ness Ave

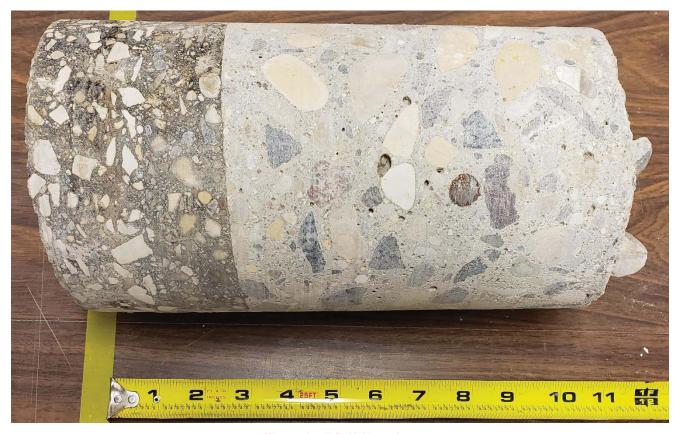


Figure 3 – PC-18 Ness Ave



Figure 4 – PC-19 Ness Ave



Figure 5 – PC-20 Ness Ave