



GENERAL NOTES

1. STRUCTURAL DESIGN BASED ON THE MANITOBA BUILDING CODE 2011 EDITION. A) IMPORTANCE CATEGORY: NORMAL

B) WIND LOAD: q50 = 9.4 PSF

C) GROUND SNOW LOAD: Ss = 39.6 PSF D) ASSOCIATED RAIN LOAD: Sr = 4.2 PSF

SEISMIC SITE CLASSIFICATION: NOT APPLICABLE DO NOT SCALE DRAWINGS

NOT LIMITED TO ALL TEMPORARY SHORING/BRACING.

4. DO NOT BACKFILL UNTIL GROUND FLOOR STRUCTURE IS IN PLACE AND BASEMENT SLABS HAVE

BEEN POURED AND CURED. 5. ALL DIMENSIONS ARE TO BE VERIFIED WITH THE ARCHITECTURAL DRAWINGS, MECHANICAL DRAWINGS

AND EXISTING SITE CONDITIONS PRIOR TO CONSTRUCTION. THESE STRUCTURAL DRAWINGS SHOW THE COMPLETED STRUCTURE AND DO NOT INDICATE ALL COMPONENTS NECESSARY FOR SAFETY DURING CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SAFETY ON AND AROUND THE JOBSITE DURING CONSTRUCTION INCLUDING BUT

CONTRACTOR TO ENSURE FUMES FROM EQUIPMENTS ARE CAPTURED PROPERLY. CONTRACTOR TO COORDINATE DRILLING RIG LOCATIONS TO POSSIBLY UTILIZE THE CURRENT EXHAUST CAPTURE SYSTEM FOR THE BUSES. USE OF ELECTRICAL POWERED SKID STEER IS MANDATORY FOR REMOVAL

FOUNDATIONS

1. FOUNDATION DESIGN BASED ON GEOTECHNICAL RECOMMENDATIONS PROVIDED BY ENG-TECH CONSULTING LIMITED. ACTUAL DESIGN VALUES WILL BE CONFIRMED AT THE TIME OF PILE INSTALLATION AND DOCUMENTED TO THE CITY OF WINNIPEG VIA THE LETTER OF COMMITMENT

NOTWITHSTANDING THE INFORMATION PROVIDED IN THE GEOTECHNICAL REPORT, THE FOUNDATION AND GENERAL CONTRACTORS SHALL SATISFY THEMSELVES AS TO THE PREVAILING CONDITIONS AT

THE SITE AS NO EXTRAS SHALL BE GRANTED SHOULD CONDITIONS DIFFER FROM THOSE INDICATED. ALL FRICTION PILES ARE DESIGNED BASED ON THE FOLLOWING: A) COMPRESSIVE DEPTH FT ULS PSF SLS PSF

BE 6-15M FOR 24 IN., 8-15M FOR 30 IN.

375 8 - 25 ULS SKIN FRICTION VALUES HAVE BEEN MULTIPLIED BY A GEOTECHNICAL RESISTANCE FACTOR

B) EFFECTIVE LENGTH OF FRICTION PILES IS TOTAL LENGTH AS SHOWN ON PLAN MINUS 8'-0"

FOR INTERIOR PILES BELOW GRADE. C) FRICTION PILE REINFORCING TO BE 20'-0" LONG UNLESS NOTED IN PLANS; 10M RINGS AT 48 IN. ON-CENTRE AND 3-10M RINGS AT 6 IN. ON-CENTRE AT TOP. PILE REINFORCING TO

ALL FOUNDATION INSTALLATIONS SHALL BE REVIEWED BY QUALIFIED GEOTECHNICAL PERSONNEL REPORTING TO THE GEOTECHNICAL ENGINEER THAT ISSUED THE SITE—SPECIFIC GEOTECHNICAL REPORT IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE NATIONAL BUILDING CODE

OF CANADA/THE MANITOBA BUILDING CODE. REMOVAL OF UNSUITABLE MATERIALS, SUB-GRADE PREPARATIONS & COMPACTED GRANULAR FILL FOR ALL SLABS SUPPORTED ON GRADE AS PER SITE-SPECIFIC GEOTECHNICAL REPORT.

CAST-IN-PLACE CONCRETE

II CONCRETE

PARTIAL FOUNDATION PLAN

1. ALL CONCRETE IS TO BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF CSA-A23.1-09 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION" AND CSA-A23.2-09 "METHOD OF TEST FOR CONCRETE".

PROVIDE CERTIFICATION THAT MIX PROPORTIONS SELECTED WILL PRODUCE CONCRETE OF QUALITY, YIELD AND STRENGTH AS SPECIFIED IN CONCRETE MIXES, AND WILL COMPLY WITH CSA-A23.1. CERTIFICATION LETTER TO BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE

PROVIDE CERTIFICATION THAT PLANT, EQUIPMENT, AND MATERIALS TO BE USED IN CONCRETE COMPLY WITH REQUIREMENTS OF CSA-A23.1. CERTIFICATION LETTER TO BE SEALED BY A PROFFSSIONAL FNGINFER REGISTERED IN THE PROVINCE OF MANITOBA.

CONCRETE TESTING TO BE PERFORMED IN ACCORDANCE WITH CSA-A23.1-09. MINIMUM ONE SET OF TESTS PER POUR. COST OF TESTING TO BE CARRIED BY THE CONTRACTOR. 5. CONCRETE PROPERTIES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

> 32 MPa MIN. AT 56 DAYS CLASS OF EXPOSURE: S-2 ENTRAINED AIR/CATEGORY: 2 (4% TO 7%) CEMENT TYPE: HS AGGREGATE: MAX. 20 mm CURING TYPE: TYPE 2 - ADDITIONAL SLUMP: MIN. 120 mm

UNLESS INDICATED OTHERWISE THE CONTRACTOR SHALL SPECIFY CONCRETE SLUMP APPROPRIATE WITH PLACEMENT METHODS AND SITE CONDITIONS. THE CONTRACTOR SPECIFIED SLUMP MUST BE SHOWN ON THE CERTIFICATION LETTER AND CONCRETE DELIVERY TICKET.

UNLESS NOTED OTHERWISE CONCRETE CURING TO CONFORM TO THE LATEST EDITION OF CSA-A23.1-09 AS FOLLOWS: A) TYPE 1 - BASIC: 3 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN 40% OF THE SPECIFIED STRENGTH. B) TYPE 2 - ADDITIONAL: 7 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN 70% OF THE SPECIFIED STRENGTH C) TYPE 3 - EXTENDED: 7 DAYS WET CURING ≥ 10°C.

REINFORCING STEEL

1. ALL REINFORCING STEEL TO BE CSA-G30.18M-M92 GRADE 400R DEFORMED BARS EXCEPT COLUMN DUST TIGHT SCREENS TIES AND BEAM STIRRUPS WHICH SHALL BE GRADE 400W STEEL ALL REINFORCING IS TO BE DETAILED IN ACCORDANCE WITH THE LATEST EDITION OF THE REINFORCING STEEL INSTITUTE OF CANADA - MANUAL OF STANDARD PRACTICE, EXCEPT OTHERWISE

NOTED. ALL LAPPED SPLICES TO BE CLASS B SPLICES, UNLESS NOTED. WELDED STEEL WIRE MESH SHALL BE TO ASTM A185/A185M-07, 400 MPg YIELD, FLAT SHEETS

4. REINFORCING STEEL COVER IS TO CONFORM TO CAN/CSA A23.3-09 "DESIGN OF CONCRETE STRUCTURES FOR BUILDINGS" AND AS FOLLOWS:

EXPOSURE CLASS: S-2 3 IN. TO TIES.

5. ALL REINFORCING TO BE HELD IN PLACE, AND TIED BY THE USE OF PROPER ACCESSORIES, SUCH AS HI-CHAIRS, SPACERS, ETC. TO BE SUPPLIED BY THE REINFORCING STEEL FABRICATOR. HI-CHAIRS TO HAVE 4 LEGS AND TO BE STAPLED OR NAILED TO THE FORMWORK. 6. ALL OPENINGS IN CAST-IN-PLACE CONCRETE FLATWORK TO BE TRIMMED WITH 2-15M ALL AROUND

ON BOTH FACES, EXCEPT AS NOTED. 7. FOR ALL STRUCTURAL SLABS A MINIMUM OF 50% OF THE BOTTOM STEEL SHALL BE CONTINUED A MINIMUM DISTANCE OF 6 IN. INTO ALL SUPPORTING WALLS AND BEAMS. IF KEYS ARE USED AT JOINTS BETWEEN SLABS AND WALLS OR BEAMS, BOTTOM DOWELS EQUAL TO BOTTOM REINFORCEMENT OR 10M AT 12 IN. O/C SHALL BE PROVIDED WHICHEVER IS GREATER.

8. ALL MISCELLANEOUS CONCRETE PADS AND CURBS ARE TO BE REINFORCED WITH A MINIMUM OF 10M AT 16 IN. O/C EACH WAY, UNLESS NOTED. 9. WHEN CONCRETE BEAMS ARE CAST INTO A WALL/BEAM CHASE, DOWELS SIZE AND NUMBER SAME

AS BEAM REINFORCING ARE TO BE PROVIDED FROM WALL, UNLESS OTHERWISE SHOWN ON PLAN. 10. FOR TWO-WAY FLAT SLABS PROVIDE MINIMUM 2-10M BOTTOM INTEGRITY BARS THROUGHOUT

STRUCTURES IN ACCORDANCE WITH CSA A23.3-09, CLAUSE 13.10.6. STRUCTURAL STEEL

TEMPORARY GUYING AND BRACING OF THE STEEL FRAMING TO PROVIDE STABILITY FOR THE STRUCTURE AS A WHOLE. THESE SHALL REMAIN IN PLACE UNTIL ALL STEEL DECKING IS ERECTED AND ALL JOIST REINFORCEMENT ARE INSTALLED. 3. STRUCTURAL STEEL TO CONFORM TO CSA-G40.21-04, "STRUCTURAL QUALITY STEELS" AND CSA-G40.20 "GENERAL REQUIREMENTS FOR ROLLED OR WELDED STRUCTURAL QUALITY STEEL". ASTM A572/A572M "STANDARD SPECIFICATION FOR HIGH-STRENGTH LOW-ALLOY COLUMBIUM-VANADIUM STRUCTURAL STEEL" OR ASTM A992/A992M "STANDARD SPECIFICATION FOR

1. THE STRUCTURAL STEEL FABRICATOR'S ENGINEER SHALL BE RESPONSIBLE FOR LOCATING AND

CONSTRUCTION TO MEET MANITOBA WORKPLACE HEALTH AND SAFETY REGULATIONS.

DESIGNING PROVISIONS FOR ALL TEMPORARY FALL PROTECTION SYSTEMS REQUIRED DURING

2. THE STRUCTURAL STEEL ERECTOR SHALL BE RESPONSIBLE FOR SUPPLYING AND ERECTING ALL

STRUCTURAL STEEL SHAPES". 4. ALL ROLLED OR STEEL STRUCTURAL SECTIONS SHALL BE G40.21-350W, ASTM A992 OR ASTM A572 GRADE 50. ALL ANGLES, AND PLATES SHALL BE G40.21-300W. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE PERFORMED IN ACCORDANCE WITH

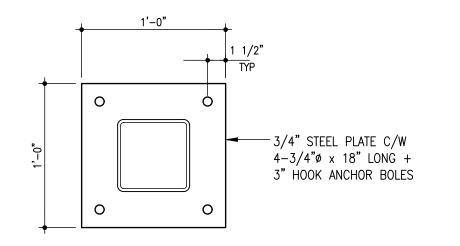
CSA S16-09, "DESIGN OF STEEL STRUCTURES". 6. ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF CSA W59, "WELDED STEEL CONSTRUCTION". FABRICATORS SHALL BE PROPERLY CERTIFIED IN ACCORDANCE WITH CSA W47.1, "CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL STRUCTURES". 7. STRUCTURAL STEEL SUPPLIER IS TO SUBMIT ENGINEERING DRAWINGS BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA COVERING THE DESIGN OF

CONNECTIONS, TO THE PROJECT DESIGN ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONNECTION DESIGN TO INCLUDE FOR ALL ADJUSTABLE CONNECTIONS REQUIRED TO SUIT FABRICATION AND ERECTION PROCEDURES AND TOLERANCES. 8. ALL STRUCTURAL STEEL IS TO RECEIVE ONE COAT OF CISC/CPMA 1-73a QUICK DRYING SHOP PRIMER. STEEL TO BE CLEANED IN CONFORMANCE WITH SSPC-SP2. STEEL RECEIVING FINISH PAINTING TO HAVE ONE COAT OF CISC/CPMA 2-75 QUICK DRYING SHOP PRIMER. STEEL TO BE CLEANED IN CONFORMANCE WITH SSPC-SP7.

9. NO HOLES PERMITTED IN TOP FLANGE OF BEAMS AT COLUMNS WHERE BEAMS ARE CONTINUOUS 10. FABRICATOR TO NOTIFY ENGINEER OF ANY PROPOSED MEMBER SUBSTITUTIONS AND CHANGED CONNECTION DETAILS. 11. THE STRUCTURAL STEEL SUPPLIER SHALL PROVIDE AND BE RESPONSIBLE FOR ALL HOLES IN STEEL SECTIONS REQUIRED BY OTHER TRADES. SECTION SHALL BE STRENGTHENED WHERE

REQUIRED TO GUARANTEE THE ORIGINAL STRENGTH OF THE BEAM. ANY CUTTING OF STEEL AT THE JOB SITE SHALL BE DONE ONLY AS DIRECTED AND APPROVED BY THE ENGINEER. 12. ALL OPENINGS LARGER THAN 18 IN. x 18 IN. THROUGH STEEL DECK TO BE FRAMED WITH L3x3 x 1/4 ANGLES ALL AROUND, EXCEPT AS NOTED. SMALLER OPENINGS THROUGH STEEL DECK TO BE STIFFENED BY STEEL DECK SUPPLIER. WHEN STEEL DECK CHANGES ITS FRAMING DIRECTION, USE L 2 1/2 x 2 1/2 x 1/4 ANGLE TO SUPPORT EDGE.

CONTRACTOR TO ENSURE THE FOLLOWING SILICA DUST MONITORING/CONTROL MEASURES ARE IN PLACE: PROVIDE DUST TIGHT SCREENS TO LOCALIZE DUST GENERATING ACTIVITIES, AND FOR PROTECTION OF WORKERS, FINISHED AREAS OF WORK, AND PUBLIC. MAINTAIN AND RELOCATE PROTECTION UNTIL SUCH WORK IS COMPLETE. DUST CONTROL IS TO CONFORM WITH "MANITOBA REGULATION 217/2006 - WORKPLACE SAFETY



U/S BASE PLATE EL 98'-11" UNLESS NOTED OTHERWISE

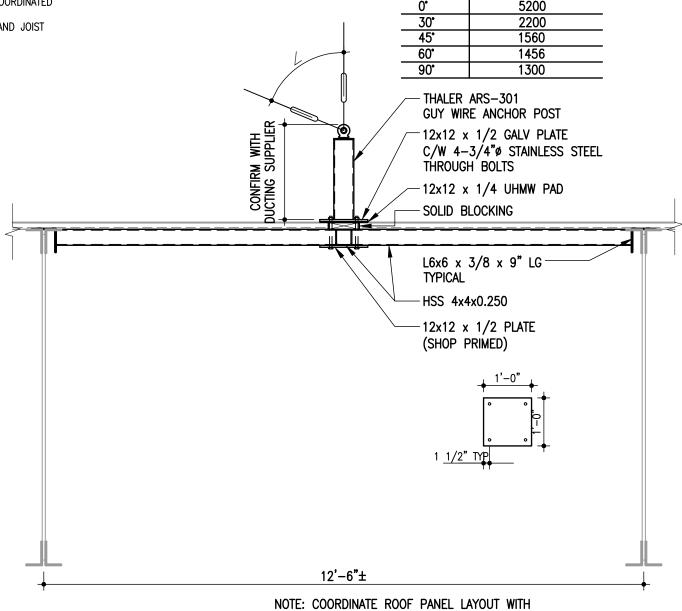
PARTIAL ROOF FRAMING PLAN

DESIGN LOADS:

DEAD LOAD = 18 PSF

SNOW LOAD = 36 PSF + SNOW BUILDUP SEE DRAWING S2.1 FOR JOIST MODIFICATION ELEVATIONS BEAMS AND COLUMNS LAYOUT BASED ON PRELIMINARILY FLOOR PLAN PROVIDED BY

WINNIPEG TRANSIT. EXACT LOCATION OF STRUCTURAL ELEMENTS SHALL BE COORDINATED PRIOR TO CONSTRUCTION. JOIST LOCATIONS SHOWN ARE APPROXIMATE ONLY, SITE CONFIRM LOCATION AND JOIST PROFILE PRIOR TO COMMENCING FABRICATION.



JOIST SPACING FOR GUY WIRE LOCATIONS

MAX ALLOWABLE

<u>′ | Tension_force_(lb)</u>

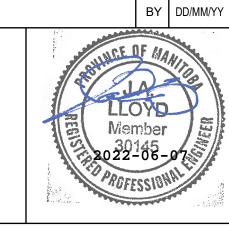
GUY WIRE/ROOF ANCHOR BETWEEN TRUSSES $\sqrt{S1.1} | S1.1 / 1/2" = 1'-0"$

GC TO COORDINATE NUMBER AND LOCATION OF GUY WIRE SUPPORTS WITH DUCTING SUPPLIER. POST COATING TO MATCH ROOFING. NOT DESIGNED AS A FALL ARREST NIT-OFF POINT.

DUCT SUPPORT DESIGN GUIDANCE

DUCT SUPPORT FRAMING IS TO BE DESIGNED AND SUPPLIED BY THE DUCTING CONTRACTOR. SHOP DRAWINGS OF DUCT SUPPORTS ARE TO BE SUBMITTED FOR REVIEW AND SHALL INCLUDE DESIGN LOADING AND MAXIMUM SUPPORT LEG REACTIONS. MAXIMUM LOAD APPLIED TO ROOF FRAMING TO BE 36 PSF, SUPPORT PADS TO BE DESIGNED ACCORDINGLY TO LIMIT LOAD APPLIED TO ROOF. SUPPORT PADS TO BE CONSTRUCTED OF MATERIAL TO AVOID DAMAGE TO ROOFING MEMBRANE, OR SHALL BE PLACED ON SOPREMA SOPRAMAT ROOF PROTECTION PADS.

ENGINEERS GEOSCIENTISTS **MANITOBA** Certificate of Authorization Crosier Kilgour & Partners Ltd. No. 235 Winnipeg, Manitoba R3C 5R6 T 204. 943. 7501 F 204. 943. 7507 Crosier Kilgour & Partners Ltd. 2021-0050 CONSULTING STRUCTURAL ENGINEER ISSUED FOR CONSTRUCTION JAL 07/06/22 NO. Description



AREA OF WORK





roject Title

TRANSIT MAINTENANCE AND REPAIR BUILDING MECHANICAL **UPGRADE - WEST HIGH BAY** WINNIPEG **MANITOBA**

JOIST REINFORCEMENT

PARTIAL ROOF FRAMING PLAN, AND GENERAL NOTES

71110 0211210 12110120		
Orawn By	Checked By	Approved By
CJM/MS	AL	JAL
Scale	Date	Project No.
AS NOTED	JUNE 2022	20-349-01
Revision Number	Drawing Number	Sheet Order
0	S1.1	1 OF 2