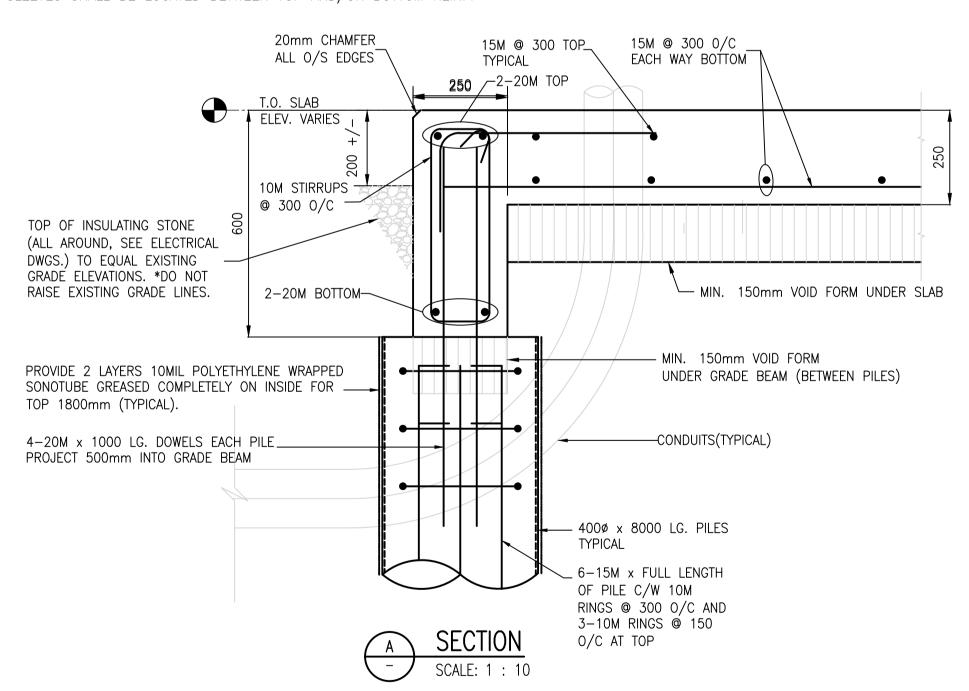
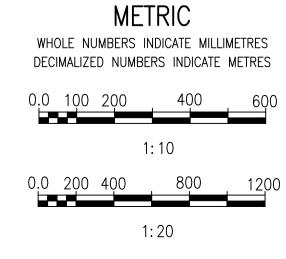


PLAN NOTES -PROVIDE 2 LAYERS 10 MIL POLYETHYLENE WRAPPED SONOTUBE GREASED COMPLETELY ON INSIDE FOR TOP 1800mm OF ALL PILES. -T.O.CONCRETE PAD ELEVATION TO BE SITE DETERMINED AND SHALL NOT BE LOWER THAN THE EXISTING TRANSFORMER PAD ELEVATION. USE 2% MINIMUM SLOPE AWAY FROM THE CONCRETE PAD FOR SURROUNDING GRADE. -SLAB SLEEVES SHALL BE LOCATED BETWEEN TOP AND/OR BOTTOM REINF.



-0142F-E0002-001 SITE PLAN — ELECTRICAL REFERENCE DRAWINGS DRAWING NUMBER



ENGINEERS
GEOSCIENTISTS MANITOBA Certificate of Authorization AtkinsRéalis Canada Inc. No. 4489

NO. REVISIONS

GENERAL NOTES:

1. STRUCTURAL DESIGN BASED ON THE MANITOBA BUILDING CODE 2023 EDITION, WHICH IS BASED ON THE NATIONAL BUILDING CODE (NBCC 2020). a. IMPORTANCE CATEGORY: POST DISASTER

b. WIND LOAD: $q_{1/50} = 0.45$

c. GROUND SNOW LOAD: $S_s=1.9$ kPa

d. ASSOCIATED RAIN LOAD: $S_r=0.2$ kPa e. SEISMIC SITE CLASSIFICATION: E

2. DO NOT SCALE DRAWINGS.

3. ALL DIMENSIONS AND ELEVATIONS ARE TO BE VERIFIED WITH THE EXISTING SITE CONDITIONS PRIOR TO CONSTRUCTION.

4. THE CONTRACTOR SHALL TAKE MEASUREMENTS IN THE FIELD AS REQUIRED TO

SUPPLEMENT DIMENSIONS AND ELEVATIONS SHOWN ON THE DRAWINGS. 5. ALL EXISTING SERVICE LINES SHALL BE LOCATED, MARKED AND PROTECTED

DURING CONSTRUCTION. 6. THESE NOTES ARE TO BE READ IN CONJUNCTION WITH ALL PERTINENT CODES AND CONTRACT DOCUMENTS. IN THE EVENT OF A CONFLICT, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN.

7. COORDINATED EMBEDDED ELECTRICAL CONDUITS WITH ELECTRICAL CONTRACTOR PRIOR TO PLACING CONCRETE & BACKFILL.

FOUNDATION:

1. FOUNDATION DESIGN BASED ON GEOTECHNICAL REPORT BY TREK

GEOTECHNICAL, PROJECT NUMBER 0019 015 00, DATED OCTOBER 17, 2024. NOTWITHSTANDING THE INFORMATION PROVIDED IN THE GEOTECHNICAL REPORT, THE FOUNDATION AND GENERAL CONTRACTORS SHALL SATISFY THEMSELVES AS TO THE PREVAILING CONDITIONS AT THE SITE AS NO EXTRAS SHALL BE GRANTED SHOULD CONDITIONS DIFFER FROM THOSE INDICATED.

3. ALL GRADE SUPPORTED CONCRETE PADS ARE DESIGNED BASED ON CAPACITIES AS FOLLOWS:

• FACTORED ULS BEARING CAPACITY OF 110 kPa. THIS BEARING CAPACITY HAS BEEN MULTIPLIED BY A GEOTECHNICAL RESISTANCE FACTOR OF 0.5.

FACTORED SLS BEARING CAPACITY OF 75 kPa.

• CONCRETE SLAB SHALL BE PLACED ON A 150 MM THICK LAYER OF 25 MM GRANULAR A CRUSHED LIMESTONE BASE COURSE COMPACTED TO 100% OF THE SPMDD. THE UPPER 500 MM CLAY FILL SHALL BE REMOVED AND REPLACED WITH 50 MM GRANULAR A CRUSHED LIMESTONE COMPACTED TO 100% OF THE SPMDD IN LIFTS NO GRATER THAN 150 MM. GRANULAR FILL MATERIALS TO BE CONSISTENT WITH THE CITY OF WINNIPEG SPECIFICATION NO CW 3110-R22.

4. ALL FRICTION PILES ARE DESIGNED BASED ON THE FOLLOWING: DEPTH (M) a. COMPRESSIVE ULS (kPa) SLS (kPa) 0 - 2.42.4 - 9.514 9.5 - 12.510 ULS SKIN FRICTION VALUE HAVE BEEN MULTIPLIED BY A GEOTECHNICAL

RESISTANCE FACTOR OF 0.4. DEPTH (M) ULS (kPa) SLS (kPa) b. UPLIFT 0 - 2.4

2.4 - 9.510.5 10.5 9.5 - 12.57.5 7.5

RESISTANCE FACTOR OF 0.3.

ULS SKIN FRICTION VALUES HAVE BEEN MULTIPLIED BY A GEOTECHNICAL

c. ULS BEARING CAPACITY OF 70 kPa. THIS BEARING CAPACITY HAS BEEN MULTIPLIED BY A GEOTECHNICAL RESISTANCE FACTOR OF 0.4,

d. SLS BEARING CAPACITY OF (0) kPa

e. EFFECTIVE LENGTH OF FRICTION PILES IS TOTAL LENGTH AS SHOWN ON PLAN MINUS 2400 mm.

f. FRICTION PILE REINFORCING TO BE FULL LENGTH UNLESS NOTED IN PLANS; 10M RINGS AT 300 mm ON-CENTRE AND 3-10M RINGS AT 150 mm ON-CENTRE AT TOP. EXTEND VERTICAL PILE REINFORCING 500 mm INTO BEAMS. PILE REINFORCING TO BE 6-15M FOR 400 mm DIAMETER PILES, 7-15M FOR 450 mm, 9-15M FOR 500 mm.

5. ALL FOUNDATION INSTALLATIONS SHALL BE MONITORED AND REVIEWED BY QUALIFIED GEOTECHNICAL PERSONNEL REPORTING TO THE GEOTECHNICAL ENGINEER THAT ISSUED THE SITE-SPECIFIC GEOTECHNICAL REPORT IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE NATIONAL BUILDING CODE OF CANADA.

6. REMOVAL OF UNSUITABLE MATERIALS, SUBGRADE PREPARATIONS & COMPACTED GRANULAR FILL FOR ALL SLABS SUPPORTED ON GRADE AS PER SITE-SPECIFIC GEOTECHNICAL REPORT.

7. PROVIDE 2 LAYERS 10 MIL POLYETHYLENE WRAPPED SONOTUBE, GREASED COMPLETELY ON INSIDE FOR TOP 1800 mm OF PILES INDICATED ON PLAN.

CAST-IN-PLACE CONCRETE:

- 1. THE CONTRACTOR SHALL PROVIDE HISTORY OF USE AND CERTIFICATION UNDER SEAL THAT THE MIX PROPORTIONS SELECTED WILL PRODUCE CONCRETE OF QUALITY AND STRENGTH AS SPECIFIED AND WILL COMPLY WITH CSA A23.1 AND THE CONTRACT DOCUMENTS.
- 2. THE CONTRACTOR SHALL PROVIDE CERTIFICATION UNDER SEAL THAT THE PLANT, EQUIPMENT, AND MATERIALS TO BE USED IN PRODUCING THE CONCRETE FOR THE WORK ARE IN ACCORDANCE WITH THE REQUIREMENTS OF CSA A23.1.
- 3. ALL MATERIALS, TESTING, AND WORKMANSHIP (INCLUDING REQUIREMENTS FOR HEATING, COOLING, HOARDING, COLD WEATHER AND WARM WEATHER CONCRETING) SHALL CONFORM TO THE LATEST REVISION OF THE PROJECT SPECIFICATIONS AND CSA A23.1 & CSA A23.2.
- 4. UNLESS NOTED OTHERWISE, THE CONTRACTOR SHALL SPECIFY CONCRETE SLUMP APPROPRIATE FOR PLACEMENT METHODS AND SITE CONDITIONS. THE SLUMP SPECIFIED BY THE CONTRACTOR MUST BE INDICATED ON THE CERTIFICATION LETTER AND THE DELIVERY TICKET.

CONCRETE PROPERTIES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE; PILES, BEAMS & CONCRETE PADS: 35MPa MIN. AT 56 DAYS CLASS EXPOSURE: S-1

> CEMENT TYPE: HS ENTRAINED AIR/CATEGORY: 1 (5% TO 8%) AGGREGATE: 20mm MAX.

CURING TYPE: TYPE 2-ADDITIONAL 6. UNLESS NOTED OTHERWISE CONCRETE CURING TO CONFORM TO CSA-A23.1 AS

FOLLOWS: TYPE 1 — BASIC: 3 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN

• TYPE 2 - ADDITIONAL: 7 DAYS ≥ 10°C AND FOR A TIME NECESSARY TO ATTAIN 70% OF THE SPECIFIED STRENGTH.

• TYPE 3 - EXTENDED: 7 DAYS WET CURING ≥ 10°C.

9. PROVIDE A 20 x20 CHAMFER ON ALL EXPOSED CONCRETE EDGES.

CONCRETE SHALL BE TESTED IN ACCORDANCE WITH CSA A23.1 & CSA A23.2. PROVIDE BROOM FINISH TO CSA A23.1 FOR THE TOP OF ALL CONCRETE PADS.

10. SLOPE TOP SURFACES OF CONCRETE USING 2% SLOPE.

40% OF THE SPECIFIED STRENGTH.

CONCRETE REINFORCEMENT:

1. ALL REINFORCING STEEL SHALL CONFORM TO CSA G30.18M, GRADE 400R.

- 2. ALL REINFORCING STEEL TO BE DETAILED IN ACCORDANCE WITH THE LATEST EDITION OF THE REINFORCING STEEL INSTITUTE OF CANADA — MANUAL OF STANDARD PRACTICE, EXCEPT OTHERWISE NOTED. ALL LAP SPLICES TO BE CLASS B SPLICES UNLESS NOTED.
- REINFORCING STEEL COVER IS TO CONFORM TO CAN/CSA A23.3-14 "DESIGN OF CONCRETE STRUCTURES FOR BUILDINGS" AND AS FOLLOWS:

EXPOSURE CLASS: S-1 76 mm TO TIES

GRADE BEAMS: EXPOSURE CLASS: S-1 60 mm TO TIES TOP/BOTTOM/SIDE

STRUCTURAL SLABS: EXPOSURE CLASS: S-1 60 mm TO TIES TOP/BOTTOM

4. SUBMIT TO THE ENGINEER SHOP DRAWINGS UNDER SEAL WHICH CLEARLY INDICATE BAR SIZES, GRADE, SPACING, HOOKS, BENDS, AND SUPPORTING / SPACING DEVICES, ETC., FOR REVIEW PRIOR TO THE FABRICATION OF REINFORCING STEEL.

5. DO NOT PLACE REINFORCEMENT WITHOUT FIRST OBTAINING A COPY OF THE REVIEWED SHOP DRAWINGS FROM THE ENGINEER.

6. ALL REINFORCING SHALL BE HELD IN PLACE AND TIED BY THE USE OF

PROPER ACCESSORIES SUPPLIED BY THE REINFORCING STEEL FABRICATOR.

7. ALL REINFORCING STEEL SHALL BE CLEANED OF ALL DIRT, GREASE AND OTHER DELETERIOUS MATERIALS PRIOR TO PLACING.

EXCAVATION AND BACK FILL

1. EXCAVATIONS MUST BE CARRIED OUT IN COMPLIANCE WITH THE MANITOBA

WORKPLACE SAFETY AND HEALTH ACT. EXCAVATION STABILITY IS THE RESPONIBILITY OF THE CONTRACTOR FOR THE

DURATION OF CONSTRUCTION.

3. GRANULAR FILL AND COMPACTION METHODS TO BE REVIEWED BY THE GEOTECHNICAL ENGINEER.

AtkinsRéalis ENGINEER'S SEAL 148 Nature Park Way, Winnipeg, Manitoba Canada R3P 0X7 204-786-8080 DESIGNED BY: S. BERVANAKIS K. KOTYK RAWN BY: APPROVED BY: FM. MANZANO D. BECKER ISSUED FOR CONSTRUCTION SCALE: AS SHOWN BY: B. SOLOWAY **2024/12/20** 2024/11/20 DATE: 2024/12/20 00 ISSUED FOR TENDER AND CONSTRUCTION | 2024/12/20 | S.B. | K.K. CONSULTANT NO.: DATE DESIGN CHECK

THE CITY OF WINNIPEG WATER AND WASTE DEPARTMENT

GALT FLOOD PUMPING STATION 2025 TRANSFORMER REMEDIATION UPGRADES STRUCTURAL FOUNDATION

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