

APPENDIX 'A' - GEOTECHNICAL REPORT

GEOTECHNICAL REPORTS FOR:

Aikins Street from Sutherland Avenue to Jarvis Avenue – Asphalt Pavement Reconstruction
Robinson Street from Sutherland Avenue to Jarvis Avenue – Asphalt Pavement Reconstruction

PAVEMENT CORES FOR:

Battery Street from Aberdeen Avenue to Redwood Avenue – Concrete Pavement Rehabilitation
Mountain Avenue from Fife Street to McPhillips Street – Concrete Pavement Rehabilitation
Radford Street from College Avenue to Mountain Avenue – Concrete Pavement Rehabilitation

The geotechnical report is provided to aid in the Contractor's evaluation of the existing pavement structure and/or soil conditions. The information presented is considered accurate at the locations shown on the Drawings and at the time of drilling. However, variations in pavement structure and/or soil conditions may exist between test holes and fluctuations in groundwater levels can be expected seasonally and may occur as a result of construction activities. The nature and extent of variations may not become evident until construction commences.



Stantec Consulting Ltd.
199 Henlow Bay
Winnipeg MB R3Y 1G4

February 21, 2025

Project/File: 123317463-2

Geoff Kerr
City of Winnipeg
1155 Pacific Avenue
Winnipeg, Manitoba R3E 3P1

Good day Geoff,

Reference: 2025 Local Street Renewal Program (Contract 2) - Geotechnical Investigation

Stantec Consulting Ltd. (Stantec) was retained to undertake a factual geotechnical investigation for the 2025 Local Street Renewal Program (Contract 2) in Winnipeg, Manitoba. Use of this report is subject to the Statement of General Conditions provided in Appendix A.

The coring and drilling program was conducted from January 6 to January 24, 2025. A total of 19 locations were investigated with pavement coring and/or subsurface geotechnical drilling. Pavement coring was performed by Stantec's geotechnical field technologist, and drilling services were provided by Maple Leaf Drilling Ltd. under the supervision of Stantec's technologist. A Borehole Location Plan is provided in Appendix B.

1. Pavement Coring

A total of 19 pavement core samples were recovered to determine the in-place pavement thickness. In addition, 9 concrete core samples were tested to assess the in-place compressive strength of the concrete. One (1) concrete compressive strength test was cancelled due to the core sample being inadequate for testing (crumbly/fractured condition). The existing pavement thicknesses are summarized in Table 1 below, and the core photographs are provided in Appendix C.

2. Geotechnical Drilling

A total of 9 boreholes were investigated by geotechnical drilling. The boreholes were terminated at a depth of 2.0 m below the pavement, which resulted in borehole depths ranging from 2.6 to 2.75 m. Soil samples were obtained directly from the auger flights at depths of 0.6 m, 0.9 m, 1.2 m, 1.6 m, and 2.0 m from the bottom of the existing pavement. The testholes were examined for evidence of sloughing and groundwater seepage upon completion of drilling.

Reference: 2025 Local Street Renewal Program (Contract 2) - Geotechnical Investigation

The borehole records are provided in Appendix D. The soil classification used in the borehole records is as per ASTM D2487 – *Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System)*.

3. Existing Pavement Thicknesses

The existing pavement thicknesses are provided in the following table:

Table 1 – Existing Pavement Thicknesses

Borehole No.	Street	Asphalt Thickness (mm)	Concrete Thickness (mm)	Total Pavement Thickness (mm)
192	Aikins St	35	215	250
193	Aikins St	80	170	250
194	Aikins St	110	140	250
195	Robinson St	130	120	250
196	Robinson St	80	0	80
197	Robinson St	80	0	80
198	Charles St	80	170	250
199	Charles St	40	0	40
200	Charles St	150	0	150
201	Radford St	0	170	170
202	Radford St	0	190	190
203	Battery St	0	195	195
204	Battery St	0	150	150
205	Mountain Ave	55	235	290
206	Mountain Ave	70	210	280
207	Mountain Ave	60	240	300
208	Mountain Ave	40	230	270
209	Mountain Ave	100	225	325
210	Mountain Ave	70	180	250

4. Laboratory Testing

Laboratory determination of moisture content (ASTM D2216) was conducted on all soil samples. The results are provided on the attached borehole records.

Reference: 2025 Local Street Renewal Program (Contract 2) - Geotechnical Investigation

In addition, the following laboratory tests were conducted on select samples:

- ASTM D4318 - *Liquid Limit, Plastic Limit, and Plasticity Index of Soils*
- ASTM D7928 - *Particle-Size Distribution of Fine-Grained Soils Using The Sedimentation Analysis*
- ASTM D698 - *Laboratory Compaction Characteristics of Soil Using Standard Effort*
- ASTM D1883 - *California Bearing Ratio (CBR) of Laboratory-Compacted Soils*
- CSA A23.2-14C – *Obtaining and testing drilled cores for compressive strength testing*

The CBR tests were performed on test specimens compacted to 95% of the maximum dry density under soaked conditions.

Prior to compressive strength testing, the concrete core samples were conditioned in water at room temperature for 48 hours.

The laboratory test reports are provided in Appendix E.

5. Closure

Please contact the undersigned if you have any questions regarding this report.

Regards,

Stantec Consulting Ltd.



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Attachment: Appendix A – Statement of General Conditions
Appendix B – Borehole Location Plan
Appendix C – Core Photographs
Appendix D – Borehole Records
Appendix E – Laboratory Test Reports

- Atterberg Limits Test Reports
- Particle-Size Analysis Reports
- Standard Proctor Test Reports
- CBR Test Reports
- Concrete Compressive Strength Test Results

Appendix A

Statement of General Conditions

STATEMENT OF GENERAL CONDITIONS

USE OF THIS REPORT: This report has been prepared for the sole benefit of the Client or its agent and may not be used by any third party without the express written consent of Stantec and the Client. Any use which a third party makes of this report is the responsibility of such third party.

BASIS OF THE REPORT: The information, opinions, and/or recommendations made in this report are in accordance with Stantec's present understanding of the site-specific project as described by the Client. The applicability of these is restricted to the site conditions encountered at the time of the investigation or study. If the proposed site-specific project differs or is modified from what is described in this report or if the site conditions are altered, this report is no longer valid unless Stantec is requested by the Client to review and revise the report to reflect the differing or modified project specifics and/or the altered site conditions.

STANDARD OF CARE: Preparation of this report, and all associated work, was carried out in accordance with the normally accepted standard of care in the state or province of execution for the specific professional service provided to the Client. No other warranty is made.

INTERPRETATION OF SITE CONDITIONS: Soil, rock, or other material descriptions, and statements regarding their condition, made in this report are based on site conditions encountered by Stantec at the time of the work and at the specific testing and/or sampling locations. Classifications and statements of condition have been made in accordance with normally accepted practices which are judgmental in nature; no specific description should be considered exact, but rather reflective of the anticipated material behavior. Extrapolation of in situ conditions can only be made to some limited extent beyond the sampling or test points. The extent depends on variability of the soil, rock, and groundwater conditions as influenced by geological processes, construction activity, and site use.

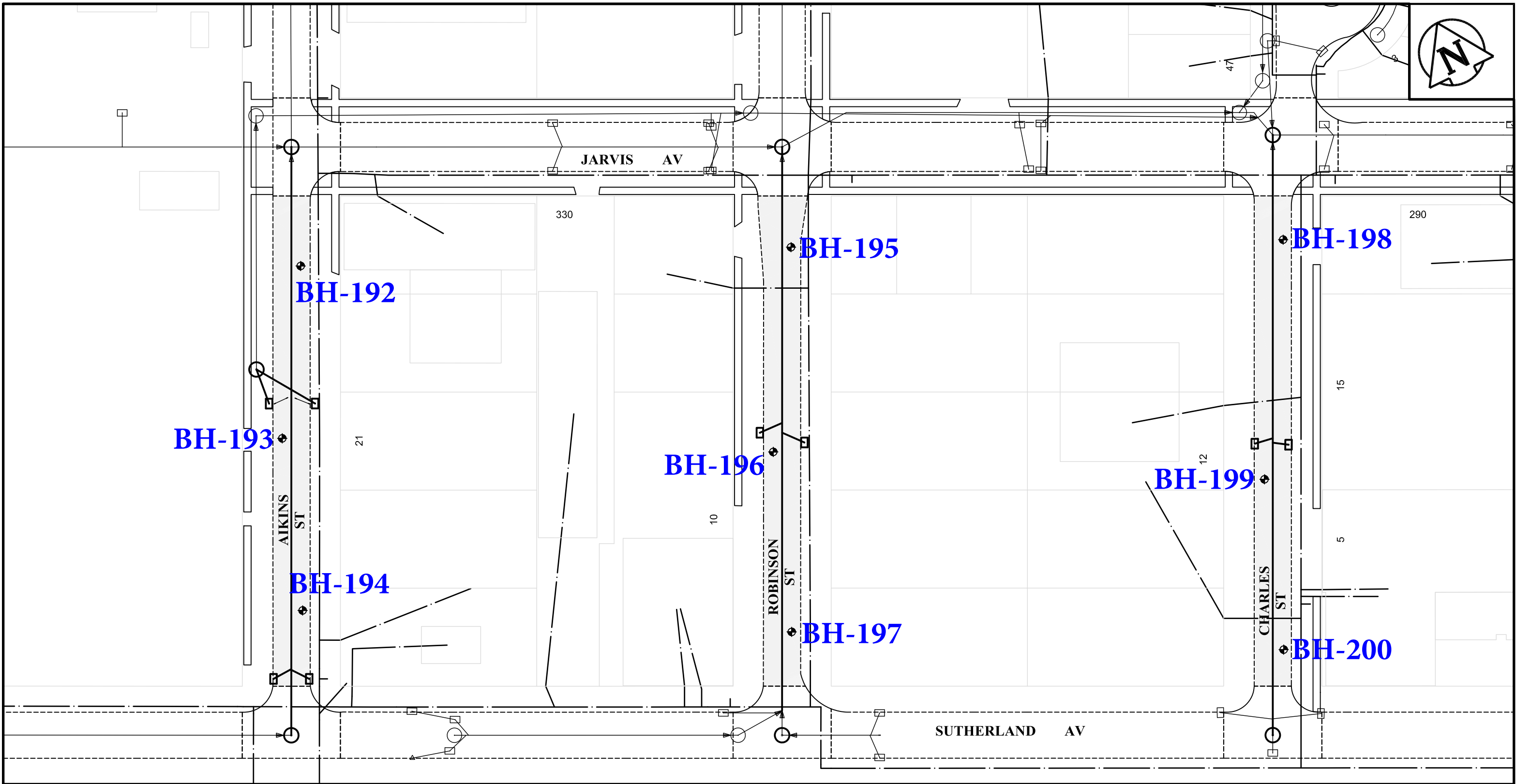
VARYING OR UNEXPECTED CONDITIONS: Should any site or subsurface conditions be encountered that are different from those described in this report or encountered at the test locations, Stantec must be notified immediately to assess if the varying or unexpected conditions are substantial and if reassessments of the report conclusions or recommendations are required. Stantec will not be responsible to any party for damages incurred as a result of failing to notify Stantec that differing site or sub-surface conditions are present upon becoming aware of such conditions.

PLANNING, DESIGN, OR CONSTRUCTION: Development or design plans and specifications should be reviewed by Stantec, sufficiently ahead of initiating the next project stage (property acquisition, tender, construction, etc.), to confirm that this report completely addresses the elaborated project specifics and that the contents of this report have been properly interpreted. Specialty quality assurance services (field observations and testing) during construction are a necessary part of the evaluation of sub-subsurface conditions and site preparation works. Site work relating to the recommendations included in this report should only be carried out in the presence of a qualified geotechnical engineer; Stantec cannot be responsible for site work carried out without being present.



Appendix B

Borehole Location Plan



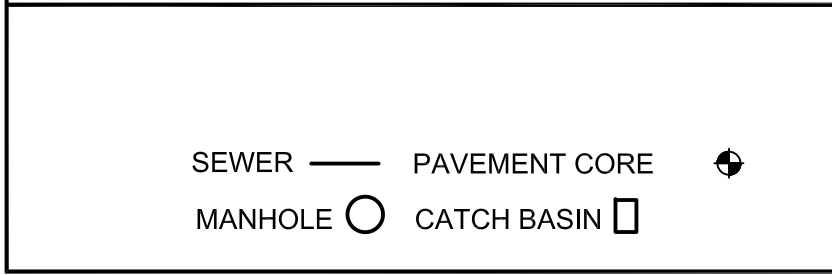
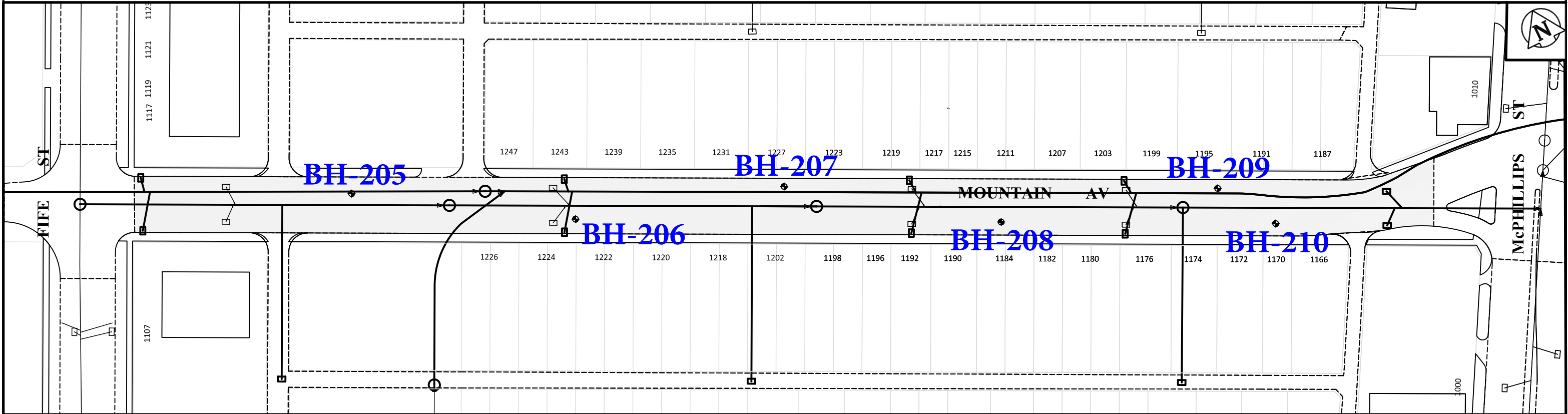
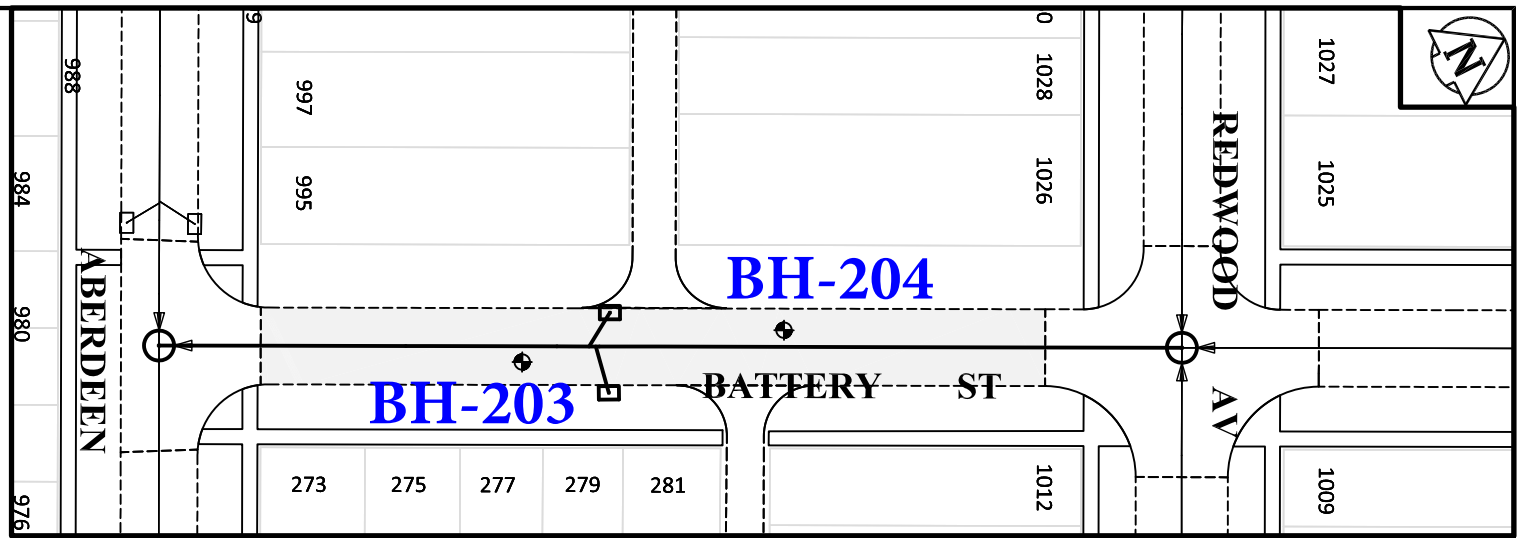
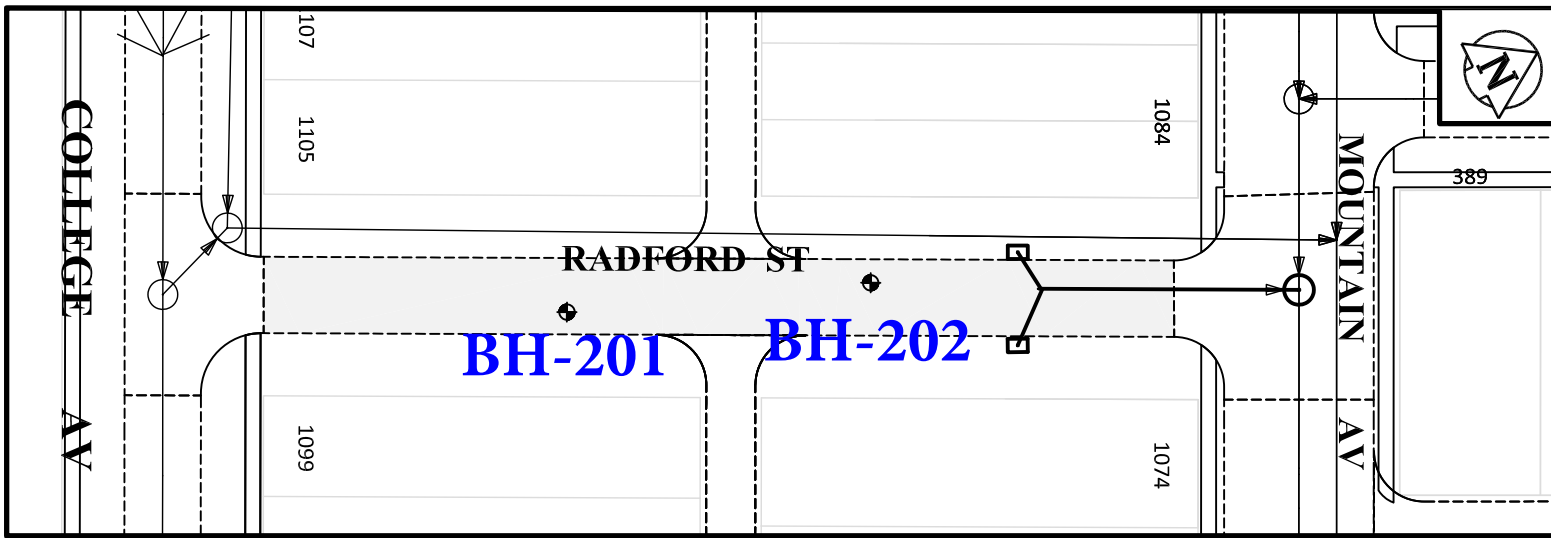
2025 LOCAL STREET RENEWAL PROGRAM CORING DRAWING - **CONTRACT 2**

AIKINS ST FROM JARVIS AV TO SUTHERLAND AV - RECONSTRUCTION
ROBINSON ST FROM JARVIS AV TO SUTHERLAND AV - RECONSTRUCTION
CHARLES ST FROM JARVIS AV TO SUTHERLAND AV - RECONSTRUCTION

SEWER — BORE HOLE ◆ WATER MAIN/ WATERLINE - - -
 MANHOLE ○ CATCH BASIN □

NOTE: DRILL 2.5m BOREHOLE AS PER SITE INVESTIGATION SPECIFICATION F3.4

DATE: 11/14/2024	DRAWING NO.: 1 of 2
DRAWN BY: J.W.	SCALE: 1:750



NOTE: DRILL PAVEMENT CORES AS PER SITE INVESTIGATION SPECIFICATION F3.5

DATE: 11/12/2024
 DRAWING NO.: 2 of 2
 DRAWN BY: J.W.
 SCALE: VARIES

2025 LOCAL STREET RENEWAL PROGRAM CORING DRAWING - CONTRACT 2

RADFORD ST FROM MOUNTAIN AV TO COLLEGE AV - MAJOR REHABILITATION
BATTERY ST FROM REDWOOD AV TO ABERDEEN AV - MINOR REHABILITATION
MOUNTAIN AV FROM McPHILLIPS ST TO FIFE ST - MAJOR REHABILITATION

Appendix C

Core Photographs



Figure 1 – Core Sample No. 192 - Aikins St



Figure 2 – Core Sample No. 193 – Aikins St



Figure 3 – Core Sample No. 194 - Aikins St



Figure 4 – Core Sample No. 195 – Robinson St



Figure 5 – Core Sample No. 196 – Robinson St



Figure 6 – Core Sample No. 197 – Robinson St



Figure 7 – Core Sample No. 198 – Charles St

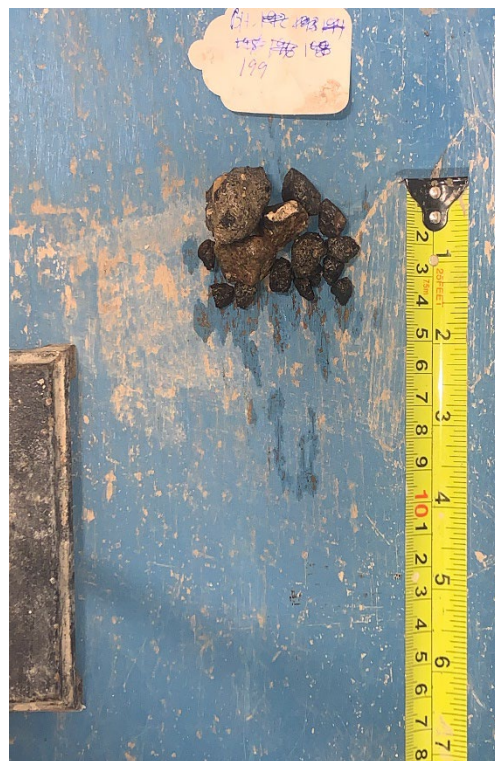


Figure 8 – Core Sample No. 199 – Charles St



Figure 9 – Core Sample No. 200 – Charles St



Figure 10 – Core Sample No. 201 – Radford St



Figure 11 – Core Sample No. 202 – Radford St



Figure 12 – Core Sample No. 203 – Battery St



Figure 13 – Core Sample No. 204 – Battery St



Figure 14 – Core Sample No. 205 – Mountain Ave



Figure 15 – Core Sample No. 206 – Mountain Ave



Figure 16 – Core Sample No. 207 – Mountain Ave



Figure 17 – Core Sample No. 208 – Mountain Ave



Figure 18 – Core Sample No. 209 – Mountain Ave



Figure 19 – Core Sample No. 210 – Mountain Ave

Appendix D

Borehole Records

SYMBOLS AND TERMS USED ON BOREHOLE AND TEST PIT RECORDS

SOIL DESCRIPTION

Terminology describing common soil genesis

<i>Rootmat</i>	vegetation, roots and moss with organic matter and topsoil typically forming a mattress at the ground surface
<i>Topsoil</i>	mixture of soil and humus capable of supporting vegetative growth
<i>Peat</i>	mixture of visible and invisible fragments of decayed organic matter
<i>Till</i>	unstratified glacial deposit which may range from clay to boulders
<i>Fill</i>	material below the surface identified as placed by humans (excluding buried services)

Terminology describing soil structure

<i>Desiccated</i>	having visible signs of weathering by oxidization of clay minerals, shrinkage cracks, etc.
<i>Fissured</i>	having cracks, and hence a blocky structure
<i>Varved</i>	composed of regular alternating layers of silt and clay
<i>Stratified</i>	composed of alternating successions of different soil types, e.g. silt and sand
<i>Layer</i>	> 75 mm in thickness
<i>Seam</i>	2 mm to 75 mm in thickness
<i>Parting</i>	< 2 mm in thickness

Terminology describing soil types

The classification of soil types are made on the basis of grain size and plasticity in accordance with the Unified Soil Classification System (USCS) (ASTM D 2487 or D 2488) which excludes particles larger than 75 mm. For particles larger than 75 mm, and for defining percent clay fraction in hydrometer results, definitions proposed by Canadian Foundation Engineering Manual, 4th Edition are used. The USCS provides a group symbol (e.g. SM) and group name (e.g. silty sand) for identification.

Terminology describing cobbles, boulders, and non-matrix materials (organic matter or debris)

Terminology describing materials outside the USCS, (e.g. particles larger than 75 mm, visible organic matter, and construction debris) is based upon the proportion of these materials present:

<i>Trace, or occasional</i>	Less than 10%
<i>Some</i>	10-20%
<i>Frequent</i>	> 20%

Terminology describing compactness of cohesionless soils

The standard terminology to describe cohesionless soils includes compactness (formerly "relative density"), as determined by the Standard Penetration Test (SPT) N-Value - also known as N-Index. The SPT N-Value is described further on Page 2. A relationship between compactness condition and N-Value is shown in the following table.

Compactness Condition	SPT N-Value
<i>Very Loose</i>	<4
<i>Loose</i>	4-10
<i>Compact</i>	10-30
<i>Dense</i>	30-50
<i>Very Dense</i>	>50

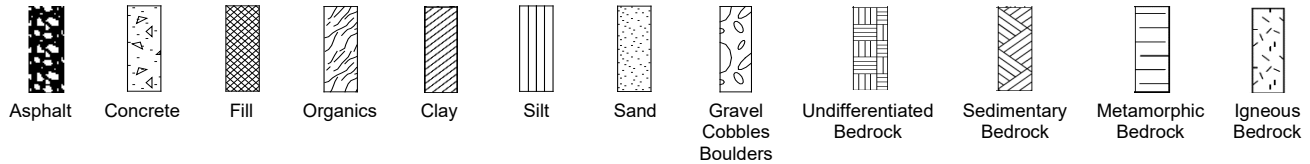
Terminology describing consistency of cohesive soils

The standard terminology to describe cohesive soils includes the consistency, which is based on undrained shear strength as measured by *in situ* vane tests, penetrometer tests, or unconfined compression tests. Consistency may be crudely estimated from SPT N-Value based on the correlation shown in the following table (Terzaghi and Peck, 1967). The correlation to SPT N-Value is used with caution as it is only very approximate.

Consistency	Undrained Shear Strength		Approximate SPT N-Value
	kips/sq.ft	kPa	
<i>Very Soft</i>	<0.25	<12.5	<2
<i>Soft</i>	0.25 - 0.5	12.5 - 25	2-4
<i>Firm</i>	0.5 - 1.0	25 - 50	4-8
<i>Stiff</i>	1.0 - 2.0	50 - 100	8-15
<i>Very Stiff</i>	2.0 - 4.0	100 - 200	15-30
<i>Hard</i>	>4.0	>200	>30

STRATA PLOT

Strata plots symbolize the soil or bedrock description. They are combinations of the following basic symbols. The dimensions within the strata symbols are not indicative of the particle size, layer thickness, etc. Not all bedrock strata plots are shown.



SAMPLE TYPE

AS, BS, GS		Auger sample; bulk sample; grab sample
DP		Direct-Push sample (small diameter tube sampler hydraulically advanced)
PS		Piston sample
SO		Sonic tube
SS		Split spoon sample (obtained by performing the Standard Penetration Test)
ST		Shelby Tube or thin wall tube
SV		Shear vane
RC HQ, NQ, BQ, etc.		Rock Core; samples obtained with the use of standard size diamond coring bits.

WATER LEVEL



Measured:
in standpipe, piezometer, or well



Inferred:
seepage noted or water level measured during or at completion of drilling

RECOVERY FOR SOIL SAMPLES

The recovery is recorded as the length of the soil sample recovered in the direct push, split spoon sampler, Shelby Tube, or sonic tube.

N-VALUE

Numbers in this column are the field results of the Standard Penetration Test (SPT): the number of blows of a 140-pound (63.5 kg) hammer falling 30 inches (760 mm), required to drive a 2 inch (50.8 mm) O.D. split spoon sampler one foot (300 mm) into the soil. In accordance with ASTM D1586, the N-Value equals the sum of the number of blows (N) required to drive the sampler over the interval of 6 to 18 in. (150 to 450 mm). However, when a 24 in. (610 mm) sampler is used, the number of blows (N) required to drive the sampler over the interval of 12 to 24 in. (300 to 610 mm) may be reported if this value is lower. For split spoon samples where insufficient penetration was achieved and N-Values cannot be presented, the number of blows are reported over sampler penetration in millimetres (e.g. 50 for 75 mm or 50/75 mm). Some design methods make use of N-values corrected for various factors such as overburden pressure, energy ratio, borehole diameter, etc. No corrections have been applied to the N-values presented on the log.

DYNAMIC CONE PENETRATION TEST (DCPT)

Dynamic cone penetration tests are performed using a standard 60-degree apex cone connected to 'A' size drill rods with the same standard fall height and weight as the Standard Penetration Test. The DCPT value is the number of blows of the hammer required to drive the cone one foot (300 mm) into the soil. The DCPT is used as a probe to assess soil variability.

OTHER TESTS

S	Sieve analysis
H	Hydrometer analysis
k	Laboratory permeability
γ	Unit weight
G_s	Specific gravity of soil particles
CD	Consolidated drained triaxial
CU	Consolidated undrained triaxial with pore pressure measurements
UU	Unconsolidated undrained triaxial
DS	Direct Shear
C	Consolidation
Q_u	Unconfined compression
I_p	Point Load Index (I_p on Borehole Record equals $I_p(50)$ in which the index is corrected to a reference diameter of 50 mm)

	Single packer permeability test; test interval from depth shown to bottom of borehole
	Double packer permeability test; test interval as indicated
	Falling head permeability test using casing
	Falling head permeability test using well point or piezometer

ROCK DESCRIPTION

Except where specified below, terminology for describing rock is as defined by the International Society for Rock Mechanics (ISRM) 2007 publication "The Complete ISRM Suggested Methods for Rock Characterization, Testing and Monitoring: 1974-2006"

Total Core Recovery (TCR) denotes the sum of all measurable rock core recovered in one drill run. The value is noted as a percentage of recovered rock core based on the total length of the drill run.

Solid Core Recovery (SCR) is defined as total length of solid core divided by the total drilled length, presented as a percentage. Solid core is defined as core with one full diameter.

Rock Quality Designation (RQD) is a modified core recovery that incorporates only pieces of solid core that are equal to or greater than 10 cm (4") along the core axis. It is calculated as the total cumulative length of solid core (> 10 cm) as measured along the centerline of the core divided by the total length of borehole drilled for each drill run or geotechnical interval, presented as a percentage. RQD is determined in accordance with ASTM D6032.

Fracture Index (FI) is defined as the number of naturally occurring fractures within a given length of core. The Fracture Index is reported as a simple count of natural occurring fractures.

Terminology describing rock quality

Rock Mass Quality	Rock Quality Designation Number (RQD)	Alternate (Colloquial) Rock Mass Quality	
<i>Very Poor Quality</i>	0-25	<i>Very Severely Fractured</i>	<i>Crushed</i>
<i>Poor Quality</i>	25-50	<i>Severely Fractured</i>	<i>Shattered or Very Blocky</i>
<i>Fair Quality</i>	50-75	<i>Fractured</i>	<i>Blocky</i>
<i>Good Quality</i>	75-90	<i>Moderately Jointed</i>	<i>Sound</i>
<i>Excellent Quality</i>	90-100	<i>Intact</i>	<i>Very Sound</i>

Terminology describing rock strength

Strength Classification	Grade	Field Estimates of Uniaxial Compressive Strength	Unconfined Compressive Strength (MPa)
<i>Extremely Weak</i>	R0	Indented by thumbnail	<1
<i>Very Weak</i>	R1	Crumbles under firm blows of geological hammer, can be peeled with a pocketknife	1 – 5
<i>Weak</i>	R2	Peeled by pocketknife with difficulty, shallow indentations made by firm blow with point of geological hammer	5 – 25
<i>Medium Strong</i>	R3	Cannot be scraped or peeled with a pocketknife, can be fractured with single firm blow of geological hammer	25 – 50
<i>Strong</i>	R4	More than one blow with geological hammer to fracture	50 – 100
<i>Very Strong</i>	R5	Many blows with geological hammer to fracture	100 – 250
<i>Extremely Strong</i>	R6	Can only be chipped with geological hammer	>250

Terminology describing rock weathering

Term	Symbol	Description
<i>Fresh</i>	W1	No visible signs of rock weathering. Slight discoloration along major discontinuities
<i>Slightly</i>	W2	Discoloration indicates weathering of rock on discontinuity surfaces. All the rock material may be discolored.
<i>Moderately</i>	W3	Less than half the rock is decomposed and/or disintegrated into soil.
<i>Highly</i>	W4	More than half the rock is decomposed and/or disintegrated into soil.
<i>Completely</i>	W5	All the rock material is decomposed and/or disintegrated into soil. The original mass structure is still largely intact.
<i>Residual Soil</i>	W6	All the rock converted to soil. Structure and fabric destroyed.

Terminology describing rock with respect to discontinuity and bedding spacing

Spacing (mm)	Discontinuities Spacing	Bedding
>6000	<i>Extremely Wide</i>	-
2000-6000	<i>Very Wide</i>	<i>Very Thick</i>
600-2000	<i>Wide</i>	<i>Thick</i>
200-600	<i>Moderate</i>	<i>Medium</i>
60-200	<i>Close</i>	<i>Thin</i>
20-60	<i>Very Close</i>	<i>Very Thin</i>
<20	<i>Extremely Close</i>	<i>Laminated</i>
<6	-	<i>Thinly Laminated</i>

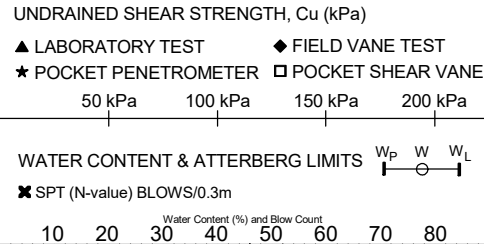


CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Aikins St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A

DEPTH (m)	ELEVATION (m)	SOIL DESCRIPTION (USCS)	STRATA PLOT	SAMPLES				OTHER TESTS / REMARKS	UNDRAINED SHEAR STRENGTH, Cu (kPa)				BACKFILL	ELEVATION (m)
				TYPE	NUMBER	RECOVERY (mm) or TCR %	N-VALUE or RQD %		50 kPa	100 kPa	150 kPa	200 kPa		
0		ASPHALT CONCRETE												
		Firm brown FAT CLAY (CH)												
1		Very soft tan SILT (ML) - some sand, trace clay		AS										
		Firm brown FAT CLAY (CH) - some silt		AS										
2				AS										
				AS										
				AS										
3		End of Borehole • Borehole terminated at a depth of 2.75 m. • No groundwater seepage or soil sloughing was observed during or upon completion of drilling. • Borehole backfilled in accordance with the City of Winnipeg Street Cuts Manual.												

Sieve/Hydro at 1.2 m
 G 0% S 1% M 42% C 58%

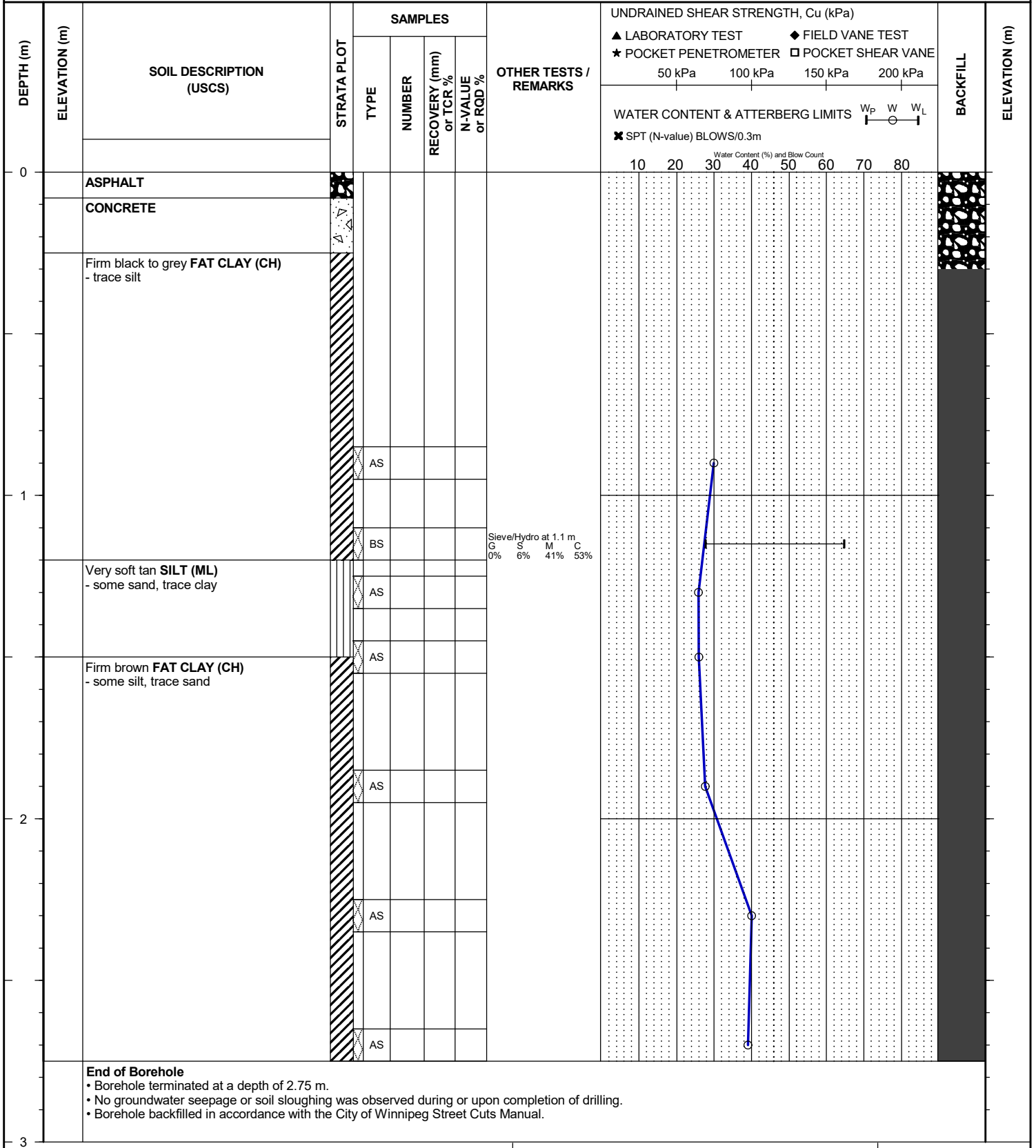


BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.75 m Page 1 of 1

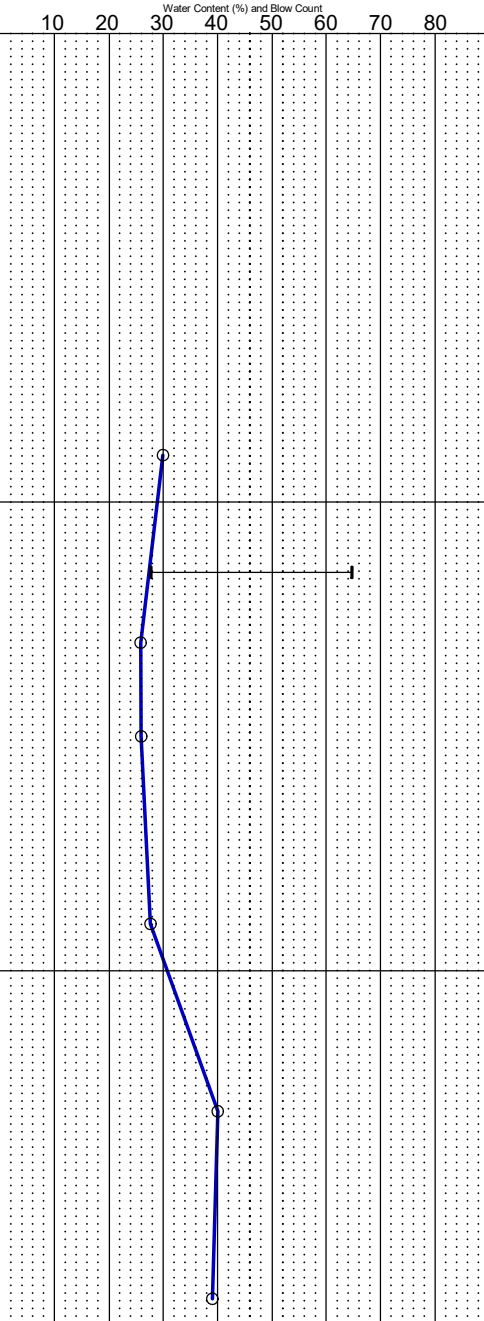
CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Aikins St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A



Sieve/Hydro at 1.1 m
 G S M C
 0% 6% 41% 53%

UNDRAINED SHEAR STRENGTH, Cu (kPa)
 ▲ LABORATORY TEST ◆ FIELD VANE TEST
 ★ POCKET PENETROMETER □ POCKET SHEAR VANE
 50 kPa 100 kPa 150 kPa 200 kPa
 WATER CONTENT & ATTERBERG LIMITS W_P W W_L
 ✕ SPT (N-value) BLOWS/0.3m



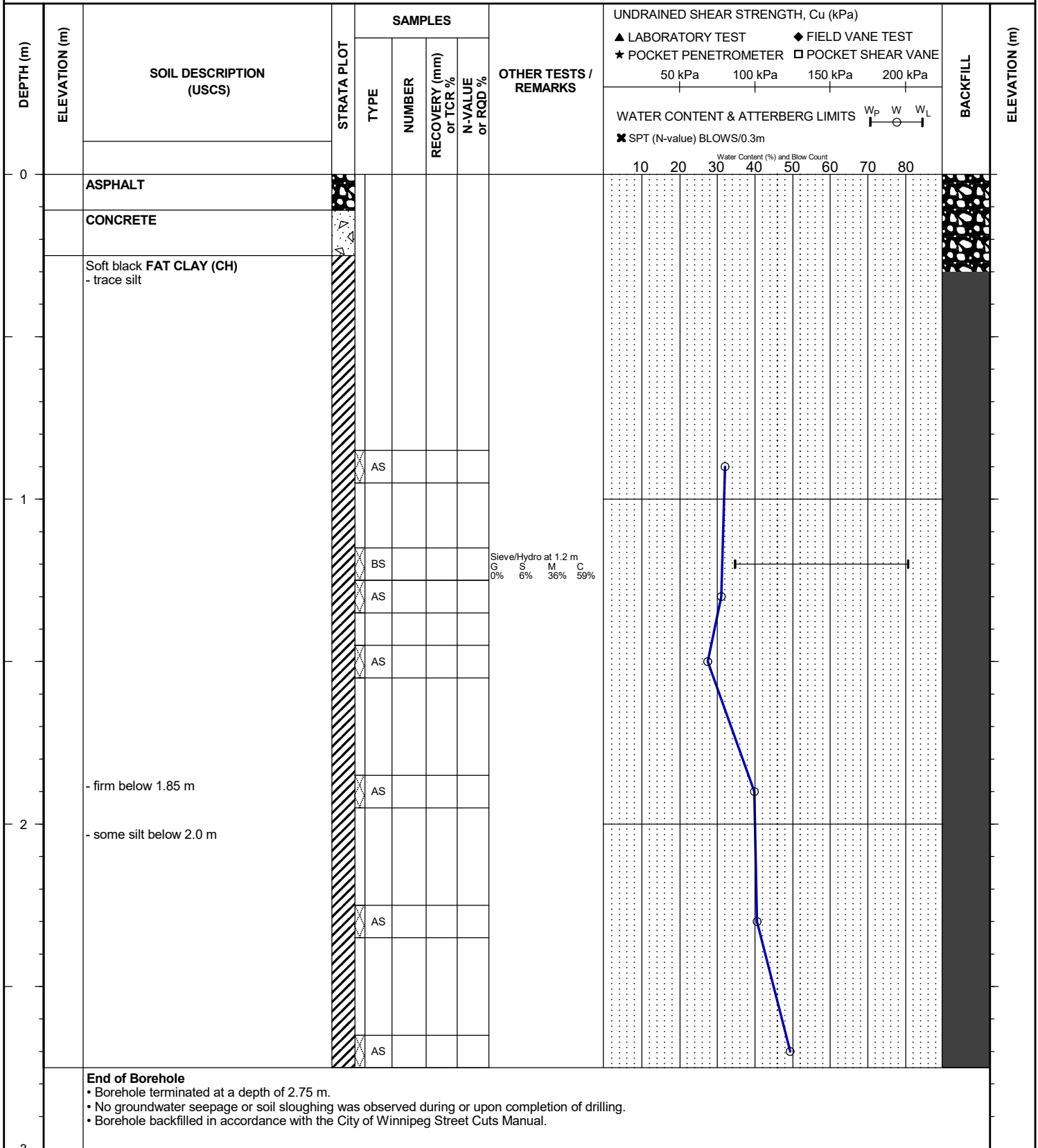
Printed Feb 21 2025 11:02:21 SOIL_123317463-2.GPJ 2/21/25

BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.75 m Page 1 of 1

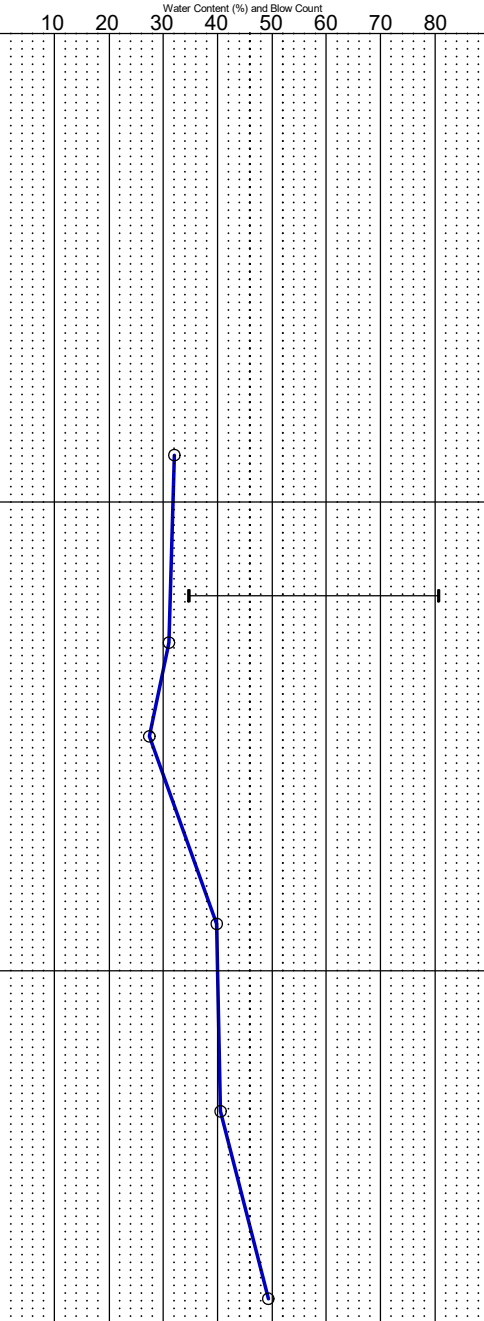
CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Aikins St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A



Sieve/Hydro at 1.2 m
 C 0% S 6% M 36% C 59%

UNDRAINED SHEAR STRENGTH, Cu (kPa)
 ▲ LABORATORY TEST ◆ FIELD VANE TEST
 ★ POCKET PENETROMETER □ POCKET SHEAR VANE
 WATER CONTENT & ATTERBERG LIMITS W_P W W_L
 ✕ SPT (N-value) BLOWS/0.3m



Printed Feb 21 2025 11:02:22 SOIL_123317463-2.GPJ_2/21/25

BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.75 m Page 1 of 1

CLIENT: City of Winnipeg

PROJECT NO.: 123317463-2

PROJECT: 2025 Local Street Renewal Program (Contract 2)

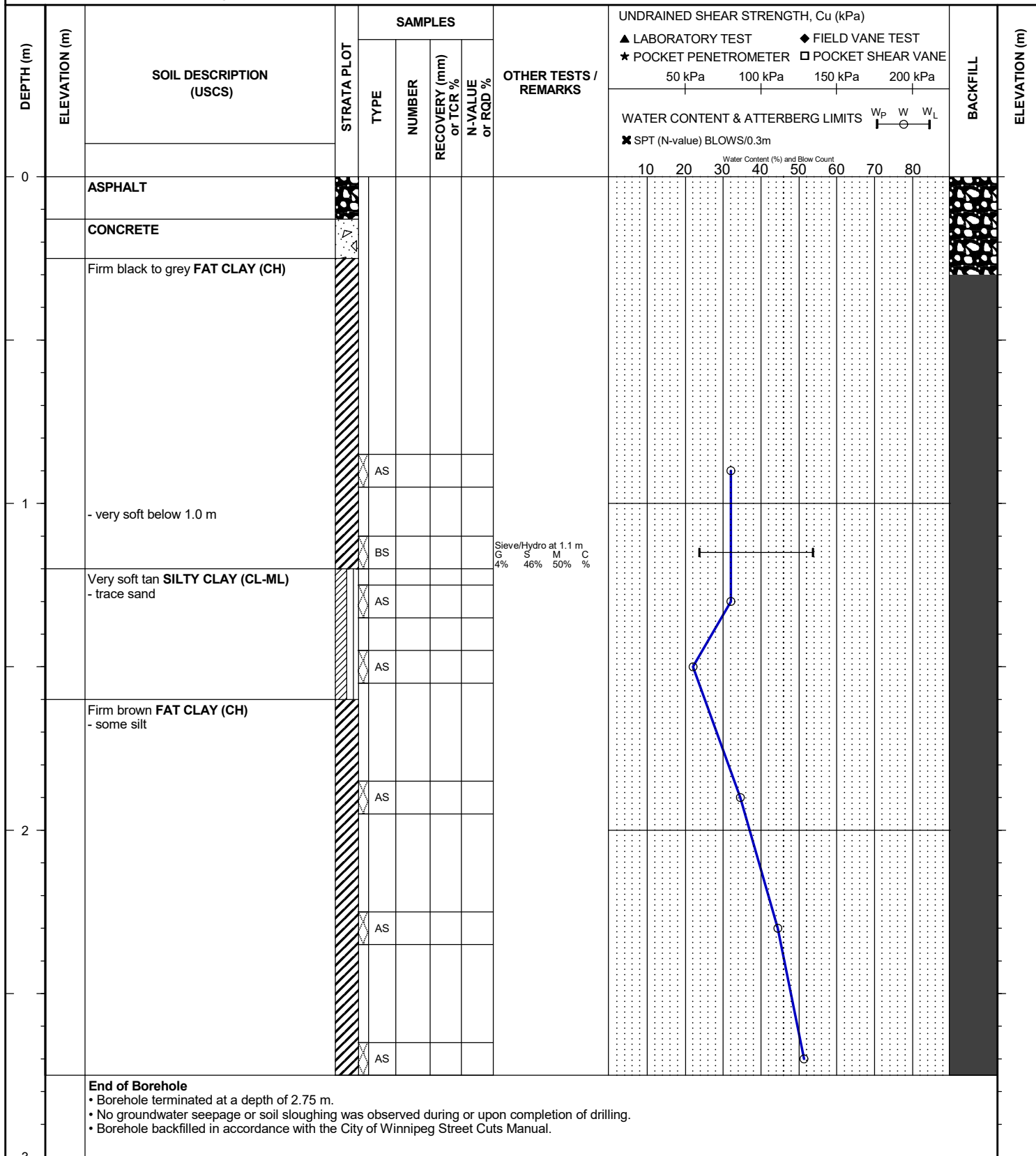
BH ELEVATION: N/A

LOCATION: Robinson St

DATUM: N/A

DATE BORED: January 07 2025

WATER LEVEL: N/A



Printed Feb 21 2025 11:02:23 SOIL_123317463-2.GPJ 2/21/25

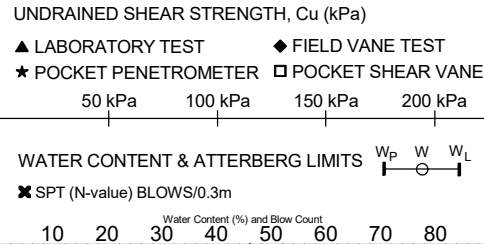
BACKFILL SYMBOL ASPHALT BENTONITE DRILL CUTTINGS	GROUT SAND	CONCRETE SLOUGH	Drilling Contractor: Maple Leaf Drilling Ltd. Drilling Method: 125 mm SSA Completion Depth: 2.75 m	Logged By: RB Reviewed By: GB Page 1 of 1
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CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Robinson St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A

DEPTH (m)	ELEVATION (m)	SOIL DESCRIPTION (USCS)	STRATA PLOT	SAMPLES				OTHER TESTS / REMARKS	UNDRAINED SHEAR STRENGTH, Cu (kPa)				BACKFILL	ELEVATION (m)
				TYPE	NUMBER	RECOVERY (mm) or TCR %	N-VALUE or RQD %		50 kPa	100 kPa	150 kPa	200 kPa		
0		ASPHALT												
		Firm black to grey FAT CLAY (CH) - trace silt												
1		Very soft tan SILTY CLAY (CL-ML) - trace sand		AS										
				BS										
				AS										
				AS										
				AS										
2		Firm brown FAT CLAY (CH) - some silt, trace sand		AS										
				AS										
				AS										
3		End of Borehole • Borehole terminated at a depth of 2.6 m. • No groundwater seepage or soil sloughing was observed during or upon completion of drilling. • Borehole backfilled in accordance with the City of Winnipeg Street Cuts Manual.												

Sieve/Hydro at 1.0 m
 G S M C
 1% 3% 84% 13%



BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

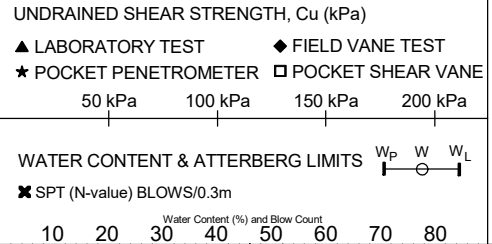
Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.6 m Page 1 of 1

CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Robinson St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A

DEPTH (m)	ELEVATION (m)	SOIL DESCRIPTION (USCS)	STRATA PLOT	SAMPLES				OTHER TESTS / REMARKS	UNDRAINED SHEAR STRENGTH, Cu (kPa)				BACKFILL	ELEVATION (m)
				TYPE	NUMBER	RECOVERY (mm) or TCR %	N-VALUE or RQD %		50 kPa	100 kPa	150 kPa	200 kPa		
0		ASPHALT												
		Firm black to grey FAT CLAY (CH) - trace silt												
		Soft tan SILT (ML) - trace sand, trace clay		AS										
1		Firm brown FAT CLAY (CH) - some silt		BS										
				AS										
				AS										
2				AS										
				AS										
				AS										
3		End of Borehole • Borehole terminated at a depth of 2.6 m. • No groundwater seepage or soil sloughing was observed during or upon completion of drilling. • Borehole backfilled in accordance with the City of Winnipeg Street Cuts Manual.												

Sieve/Hydro at 1.0 m
 G S M C
 0% 6% 86% 8%



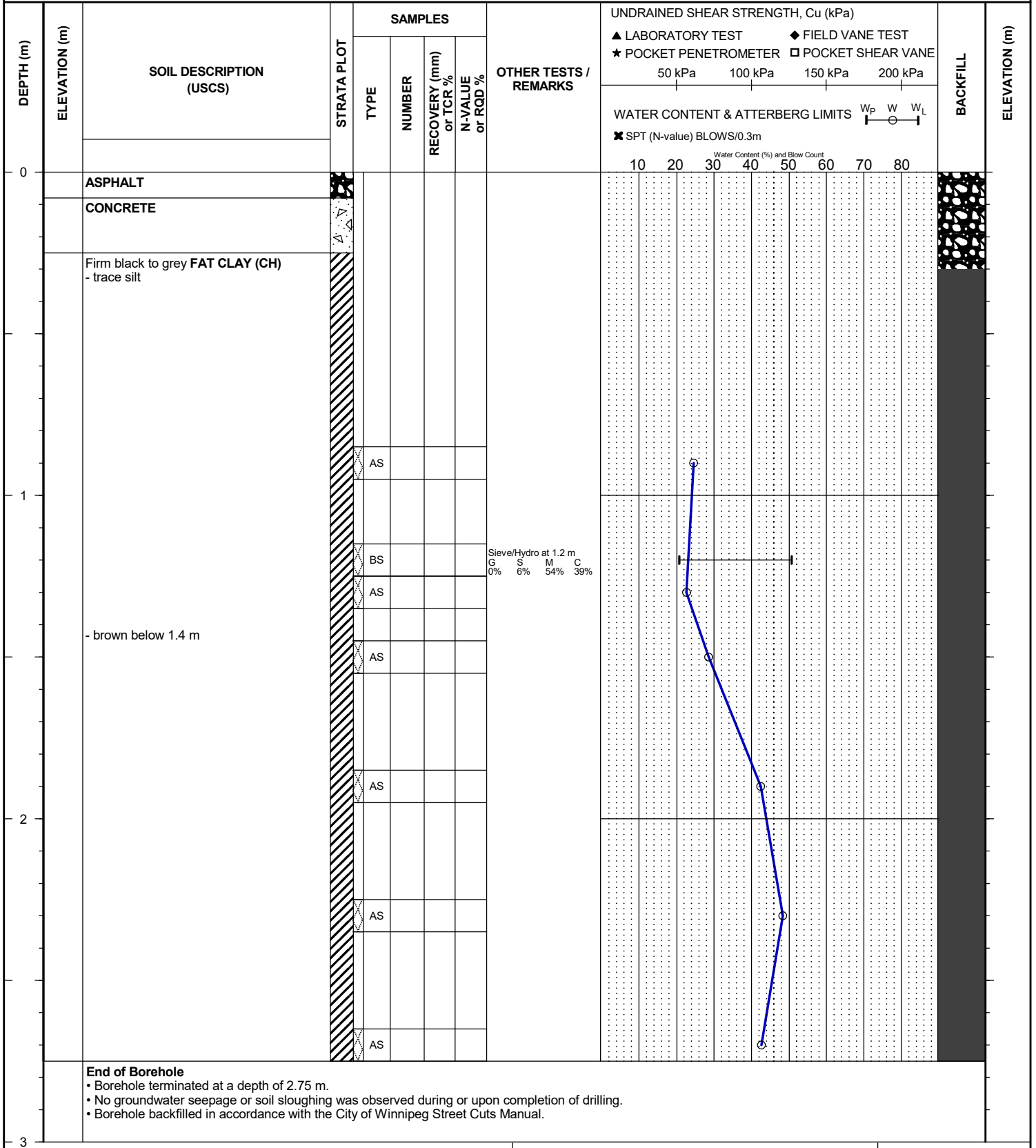
Printed Feb 21 2025 11:02:26 SOIL_123317463-2.GPJ 2/21/25

- ASPHALT
- GROUT
- CONCRETE
- BENTONITE
- DRILL CUTTINGS
- SAND
- SLOUGH

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.6 m Page 1 of 1

CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Charles St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A



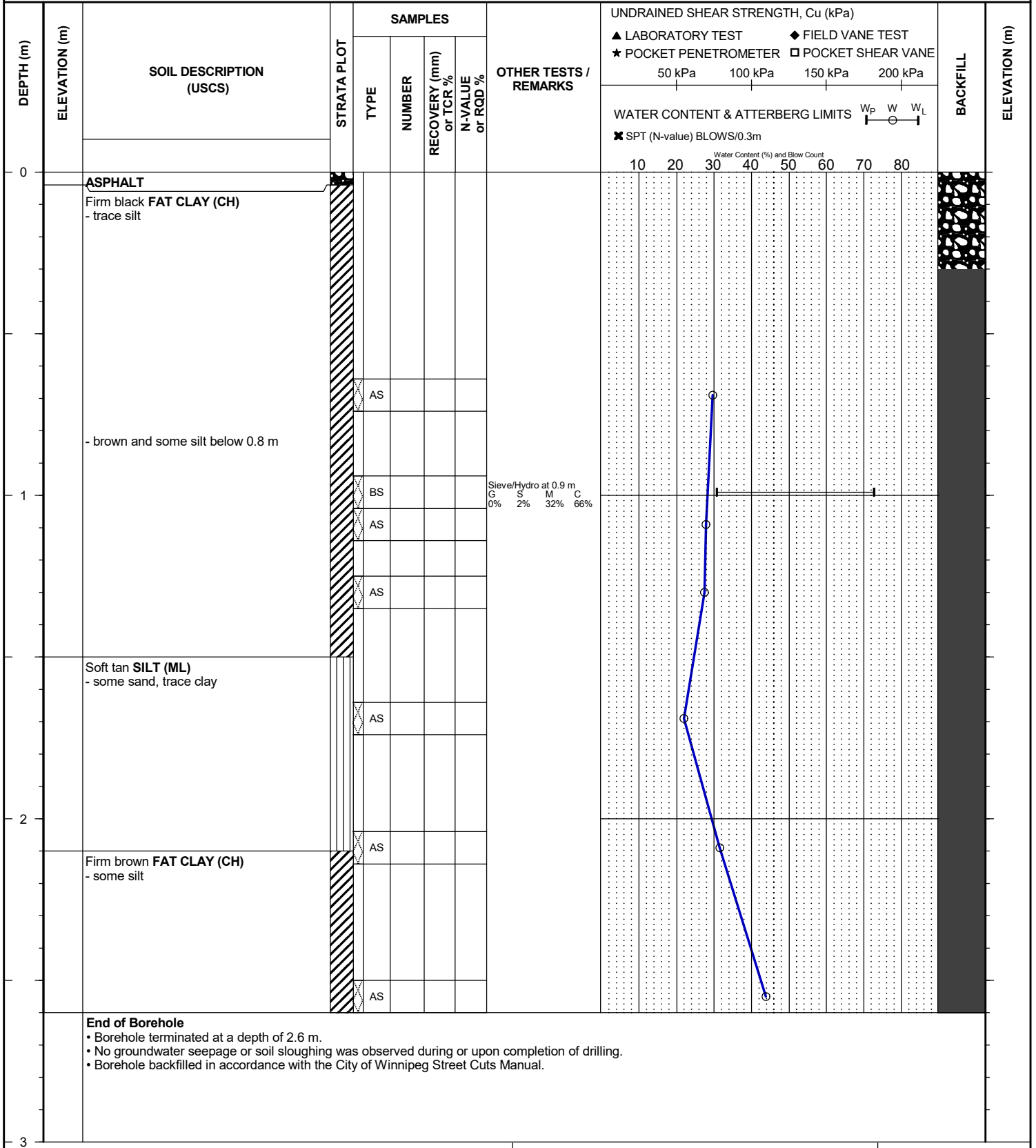
End of Borehole
 • Borehole terminated at a depth of 2.75 m.
 • No groundwater seepage or soil sloughing was observed during or upon completion of drilling.
 • Borehole backfilled in accordance with the City of Winnipeg Street Cuts Manual.

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.75 m Page 1 of 1

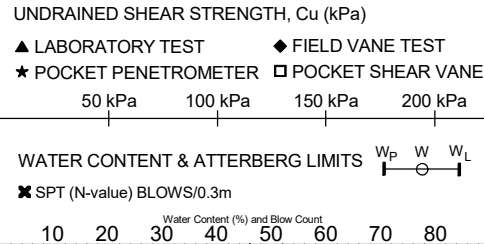
BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Charles St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A



Sieve/Hydro at 0.9 m
 G 0% S 2% M 32% C 66%



Printed Feb 21 2025 11:02:28 SOIL_123317463-2.GPJ 2/21/25

BACKFILL SYMBOL: ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.6 m Page 1 of 1

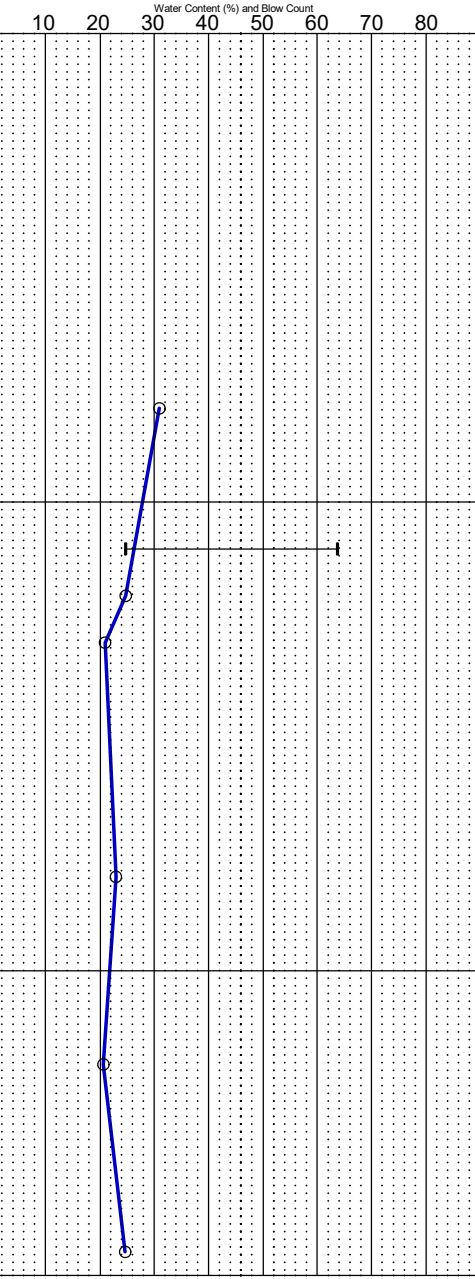
CLIENT: City of Winnipeg
 PROJECT: 2025 Local Street Renewal Program (Contract 2)
 LOCATION: Charles St
 DATE BORED: January 07 2025

PROJECT NO.: 123317463-2
 BH ELEVATION: N/A
 DATUM: N/A
 WATER LEVEL: N/A

DEPTH (m)	ELEVATION (m)	SOIL DESCRIPTION (USCS)	STRATA PLOT	SAMPLES				OTHER TESTS / REMARKS	UNDRAINED SHEAR STRENGTH, Cu (kPa)				BACKFILL	ELEVATION (m)
				TYPE	NUMBER	RECOVERY (mm) or TCR %	N-VALUE or RQD %		50 kPa	100 kPa	150 kPa	200 kPa		
0		ASPHALT												
		FILL: granular base, 12.5 mm Firm black to grey FAT CLAY (CH) - trace silt												
1		Soft tan SILT (ML) - some sand, trace clay		AS										
				BS										
				AS										
				AS										
2				AS										
				AS										
				AS										
3		End of Borehole												

Sieve/Hydro at 1.1 m
 C 0% S 1% M 38% C 61%

UNDRAINED SHEAR STRENGTH, Cu (kPa)
 ▲ LABORATORY TEST ◆ FIELD VANE TEST
 ★ POCKET PENETROMETER □ POCKET SHEAR VANE
 50 kPa 100 kPa 150 kPa 200 kPa
 WATER CONTENT & ATTERBERG LIMITS W_p W W_L
 ✕ SPT (N-value) BLOWS/0.3m



- Borehole terminated at a depth of 2.65 m.
- No groundwater seepage or soil sloughing was observed during or upon completion of drilling.
- Borehole backfilled in accordance with the City of Winnipeg Street Cuts Manual.

Drilling Contractor: Maple Leaf Drilling Ltd. Logged By: RB
 Drilling Method: 125 mm SSA Reviewed By: GB
 Completion Depth: 2.65 m Page 1 of 1

BACKFILL SYMBOL ASPHALT GROUT CONCRETE
 BENTONITE DRILL CUTTINGS SAND SLOUGH

Appendix E

Laboratory Testing Reports

- Atterberg Limits
- Particle-Size Analysis
- Standard Proctor
- California Bearing Ratio
- Concrete Compressive Strength

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 1

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.30

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

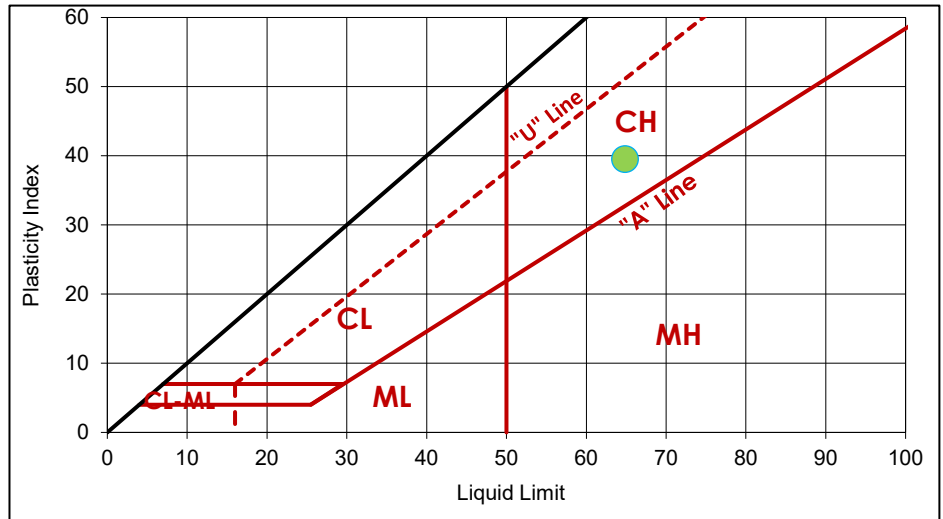
CLIENT FIELD ID BH-192, 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5611

TRIAL	LIQUID LIMIT	
	1	2
BLOWS	28	29
MC (%)	64	64

TRIAL	PLASTIC LIMIT	
	1	2
MC (%)	25	25

LIQUID LIMIT, LL	65
PLASTIC LIMIT, PL	25
PLASTICITY INDEX, PI	39
AS REC'D MC (%)	27.1



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 2

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.30

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

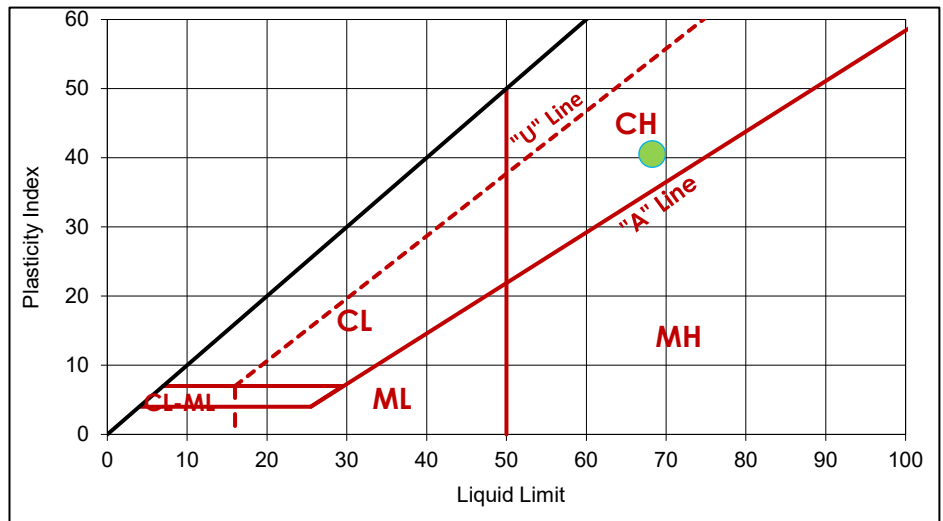
CLIENT FIELD ID BH-193, 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5612

	LIQUID LIMIT	
TRIAL	1	2
BLOWS	23	25
MC (%)	69	69

	PLASTIC LIMIT	
TRIAL	1	2
MC (%)	28	28

LIQUID LIMIT, LL	68
PLASTIC LIMIT, PL	28
PLASTICITY INDEX, PI	41
AS REC'D MC (%)	26.4



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 3

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.30

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

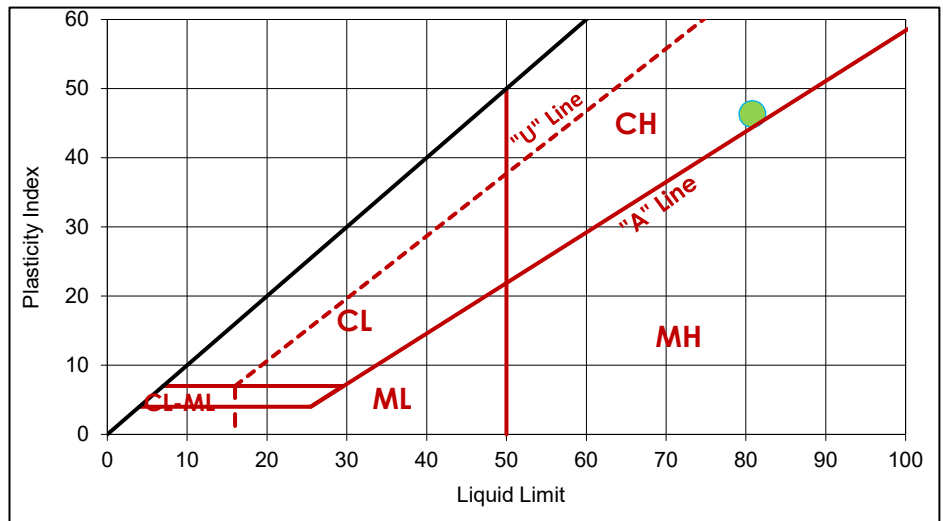
CLIENT FIELD ID BH-194, 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5613

TRIAL	LIQUID LIMIT	
	1	2
BLOWS	26	27
MC (%)	80	80

TRIAL	PLASTIC LIMIT	
	1	2
MC (%)	35	34

LIQUID LIMIT, LL	81
PLASTIC LIMIT, PL	35
PLASTICITY INDEX, PI	46
AS REC'D MC (%)	31.6



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 4

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.29

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

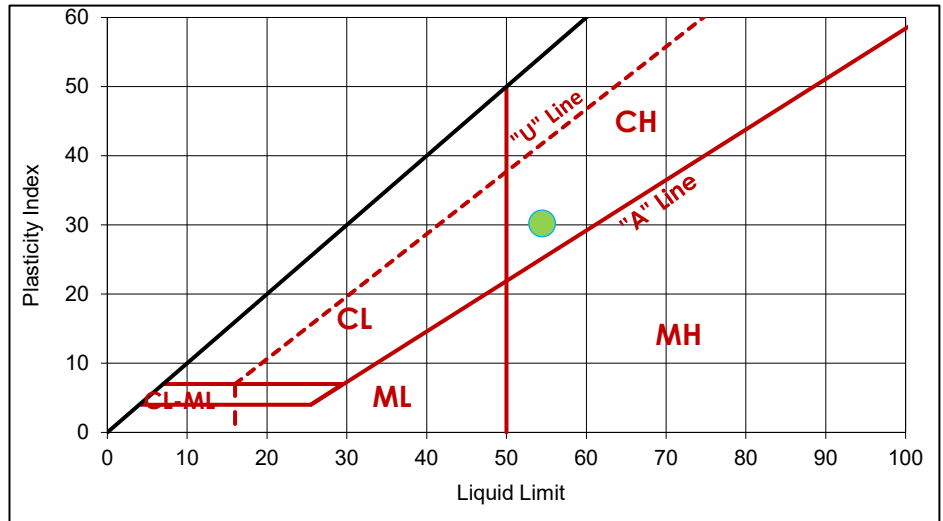
CLIENT FIELD ID BH-195, 1.2 m, Robinson St.

STANTEC SAMPLE NO. 5614

TRIAL	LIQUID LIMIT	
	1	2
BLOWS	25	24
MC (%)	55	54

TRIAL	PLASTIC LIMIT	
	1	2
MC (%)	24	24

LIQUID LIMIT, LL	54
PLASTIC LIMIT, PL	24
PLASTICITY INDEX, PI	30
AS REC'D MC (%)	32.6



COMMENTS

No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 5

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.29

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

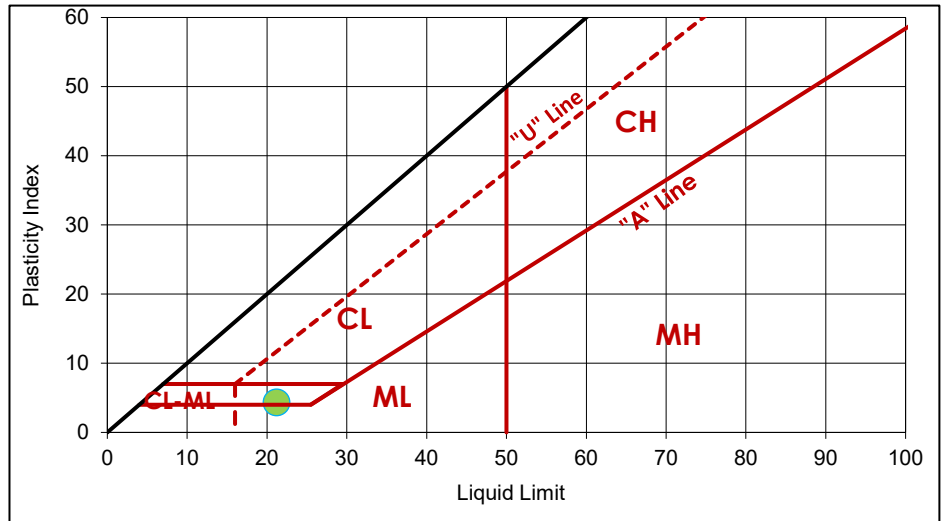
CLIENT FIELD ID BH-196, 1.0 m, Robinson St.

STANTEC SAMPLE NO. 5615

	LIQUID LIMIT	
TRIAL	1	2
BLOWS	26	26
MC (%)	21	21

	PLASTIC LIMIT	
TRIAL	1	2
MC (%)	17	17

LIQUID LIMIT, LL	21
PLASTIC LIMIT, PL	17
PLASTICITY INDEX, PI	4
AS REC'D MC (%)	20.7



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 6

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.29

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

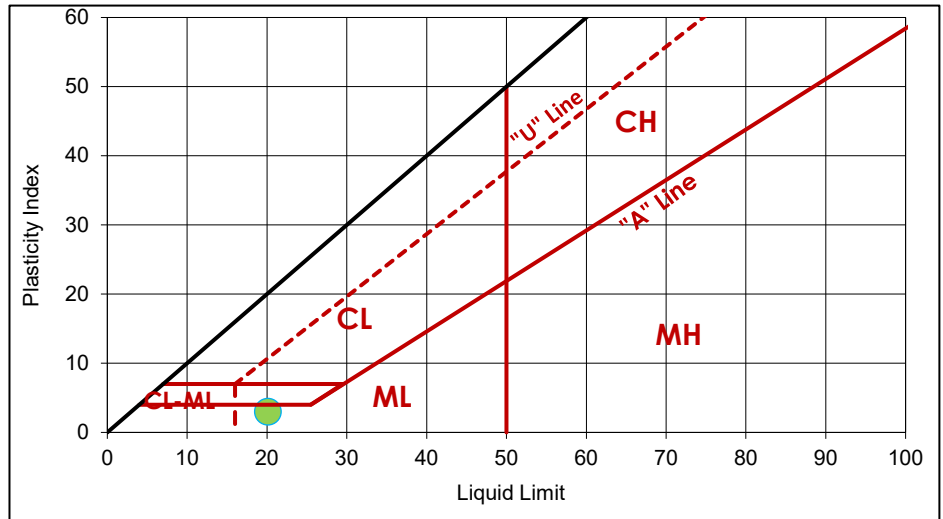
CLIENT FIELD ID BH-197, 1.0 m, Robinson St.

STANTEC SAMPLE NO. 5616

	LIQUID LIMIT	
TRIAL	1	2
BLOWS	26	26
MC (%)	20	20

	PLASTIC LIMIT	
TRIAL	1	2
MC (%)	17	17

LIQUID LIMIT, LL	20
PLASTIC LIMIT, PL	17
PLASTICITY INDEX, PI	3
AS REC'D MC (%)	17.9



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services



ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 7

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.30

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

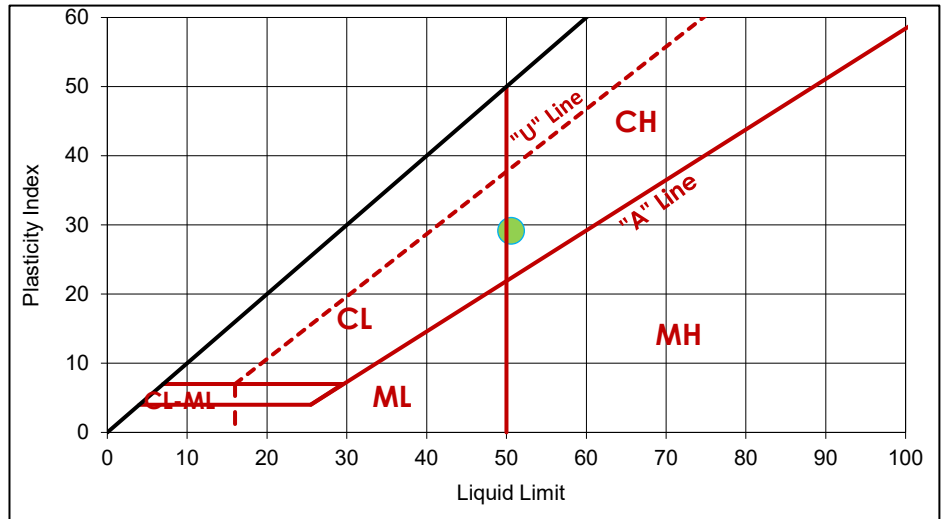
CLIENT FIELD ID BH-198, 1.2 m, Charles St.

STANTEC SAMPLE NO. 5617

	LIQUID LIMIT	
TRIAL	1	2
BLOWS	23	24
MC (%)	51	51

	PLASTIC LIMIT	
TRIAL	1	2
MC (%)	22	21

LIQUID LIMIT, LL	51
PLASTIC LIMIT, PL	21
PLASTICITY INDEX, PI	29
AS REC'D MC (%)	23.2



COMMENTS
 No comments.

REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 8

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.30

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

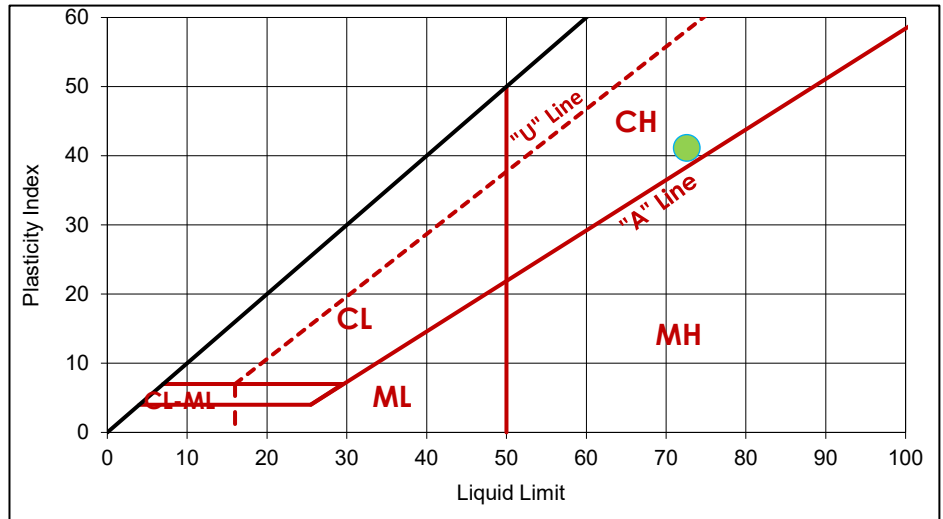
CLIENT FIELD ID BH-199, 0.9 m, Charles St.

STANTEC SAMPLE NO. 5618

TRIAL	LIQUID LIMIT	
	1	2
BLOWS	25	24
MC (%)	73	73

TRIAL	PLASTIC LIMIT	
	1	2
MC (%)	31	32

LIQUID LIMIT, LL	73
PLASTIC LIMIT, PL	31
PLASTICITY INDEX, PI	41
AS REC'D MC (%)	28.4



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D4318 - LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX OF SOILS (LL METHOD B - ONE-POINT)

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 9

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.31

SAMPLED BY: Stantec Consulting Ltd.

SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

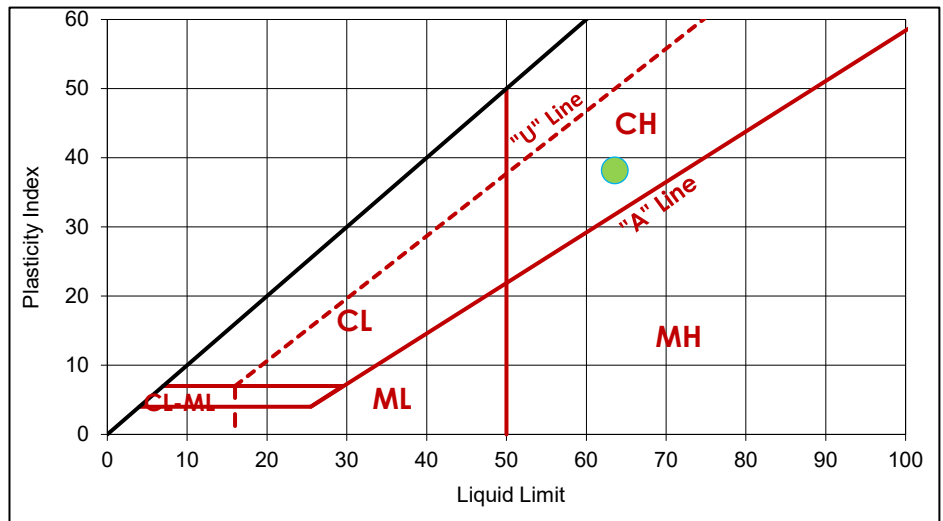
CLIENT FIELD ID BH-200, 1.1 m, Charles St.

STANTEC SAMPLE NO. 5619

	LIQUID LIMIT	
TRIAL	1	2
BLOWS	21	23
MC (%)	65	64

	PLASTIC LIMIT	
TRIAL	1	2
MC (%)	26	25

LIQUID LIMIT, LL	64
PLASTIC LIMIT, PL	25
PLASTICITY INDEX, PI	38
AS REC'D MC (%)	25.3



COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 1

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.24

SAMPLED BY: Stantec Consulting Ltd.

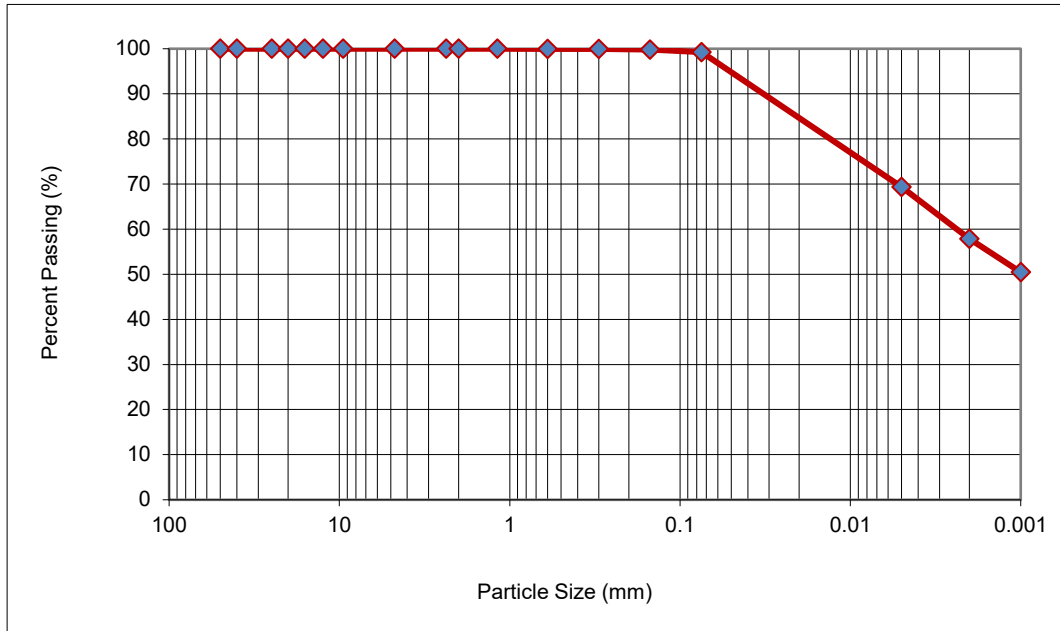
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-192. 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5611



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	100.0
1.18	100.0
0.600	100.0
0.300	99.9
0.150	99.8
0.075	99.3
0.005	69.4
0.002	57.8
0.001	50.5

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.0	0.0	0.7	41.5	57.8	50.5

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 2

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

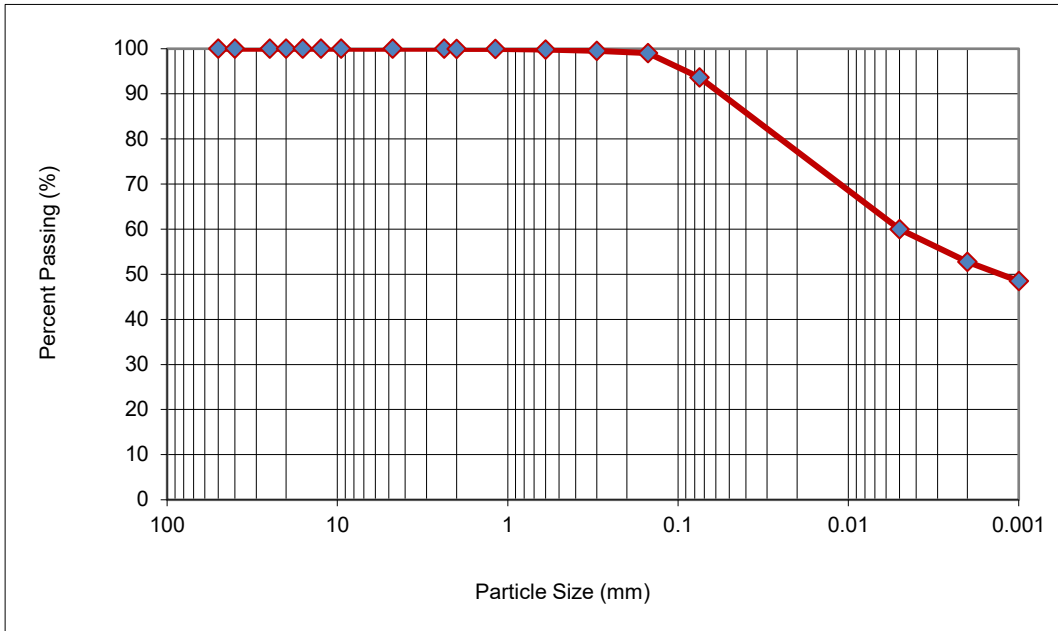
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-193. 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5612



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	100.0
1.18	100.0
0.600	99.8
0.300	99.5
0.150	99.1
0.075	93.7
0.005	60.0
0.002	52.7
0.001	48.5

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.0	0.4	5.9	41.0	52.7	48.5

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 3

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

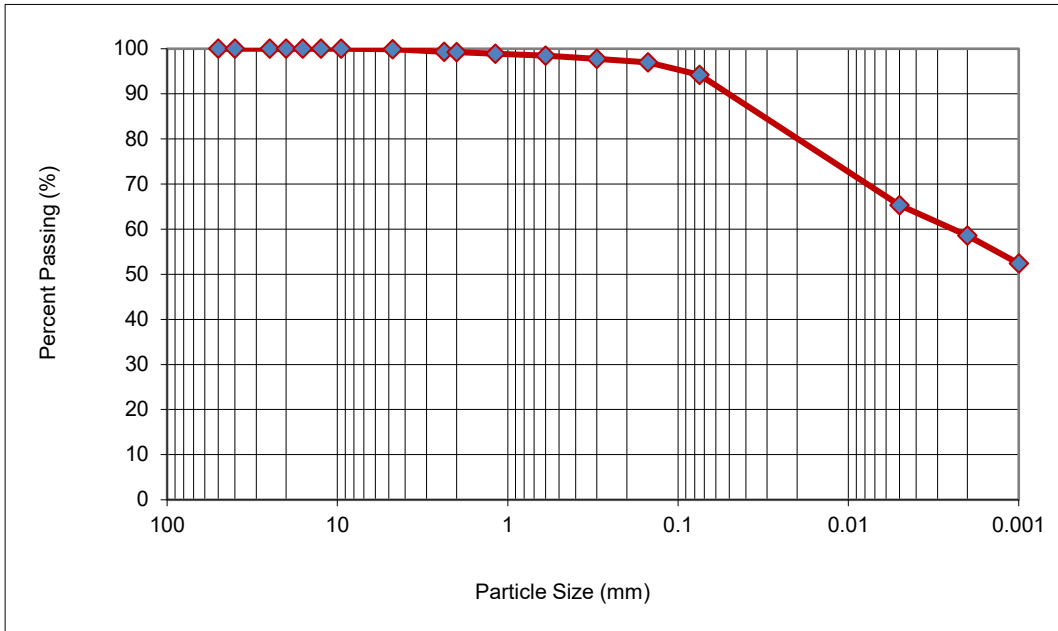
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-194. 1.2 m, Aikins St.

STANTEC SAMPLE NO. 5613



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	99.9
2.36	99.3
2.00	99.3
1.18	98.9
0.600	98.5
0.300	97.7
0.150	97.0
0.075	94.2
0.005	65.3
0.002	58.6
0.001	52.4

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.1	0.6	1.3	3.8	35.6	58.6	52.4

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 4

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

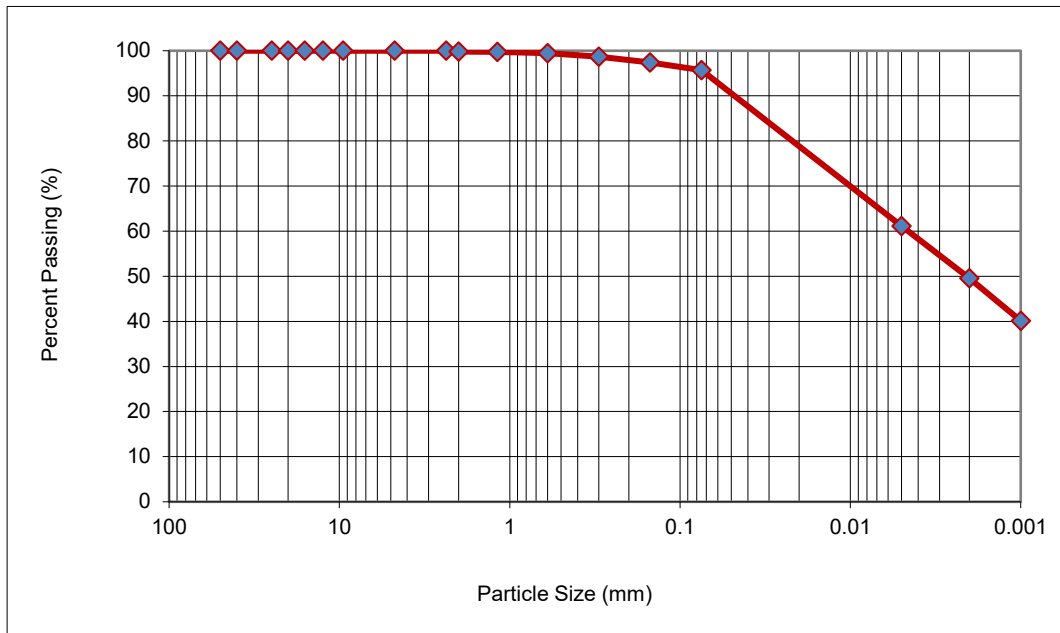
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-195. 1.2 m, Robinson St.

STANTEC SAMPLE NO. 5614



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	99.8
1.18	99.8
0.600	99.5
0.300	98.7
0.150	97.4
0.075	95.7
0.005	61.1
0.002	49.5
0.001	40.1

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.2	0.8	3.3	46.2	49.5	40.1

COMMENTS

No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 5

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

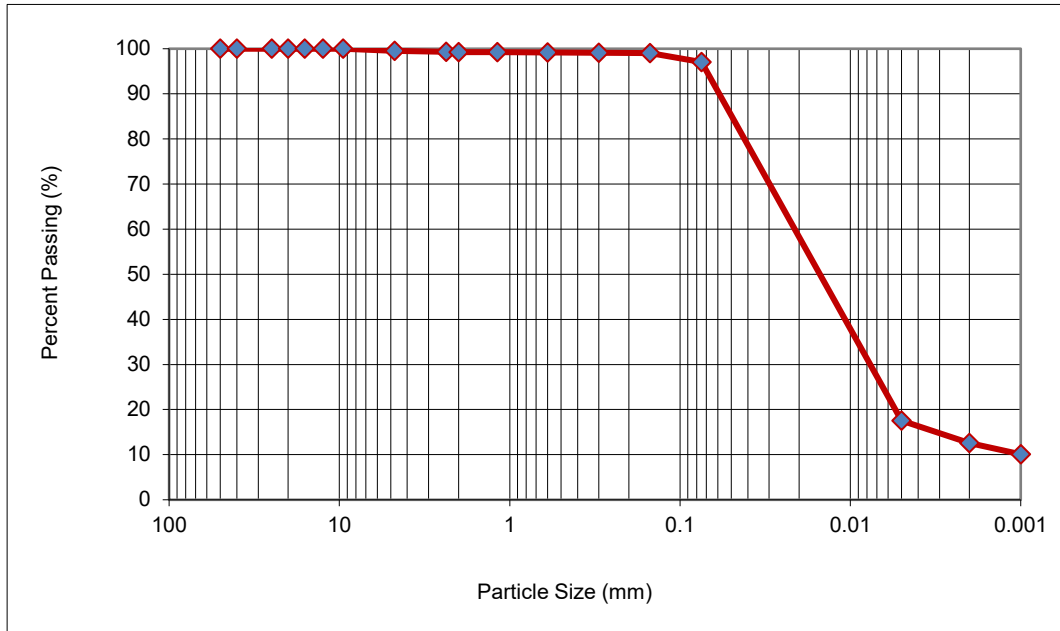
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-196. 1.0 m, Robinson St.

STANTEC SAMPLE NO. 5615



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	99.5
2.36	99.3
2.00	99.2
1.18	99.2
0.600	99.2
0.300	99.1
0.150	99.1
0.075	97.0
0.005	17.6
0.002	12.6
0.001	10.1

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.5	0.3	0.0	2.2	84.4	12.6	10.1

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 6

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

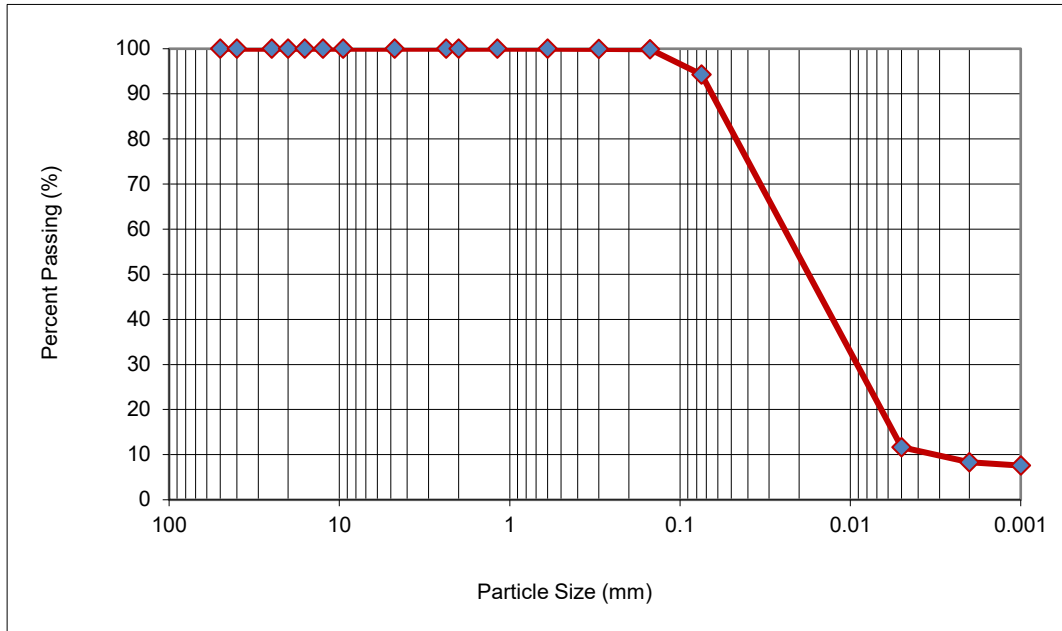
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-197. 1.0 m, Robinson St.

STANTEC SAMPLE NO. 5616



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	100.0
1.18	100.0
0.600	100.0
0.300	99.9
0.150	99.9
0.075	94.3
0.005	11.6
0.002	8.3
0.001	7.6

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.0	0.0	5.7	86.0	8.3	7.6

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 7

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.24

SAMPLED BY: Stantec Consulting Ltd.

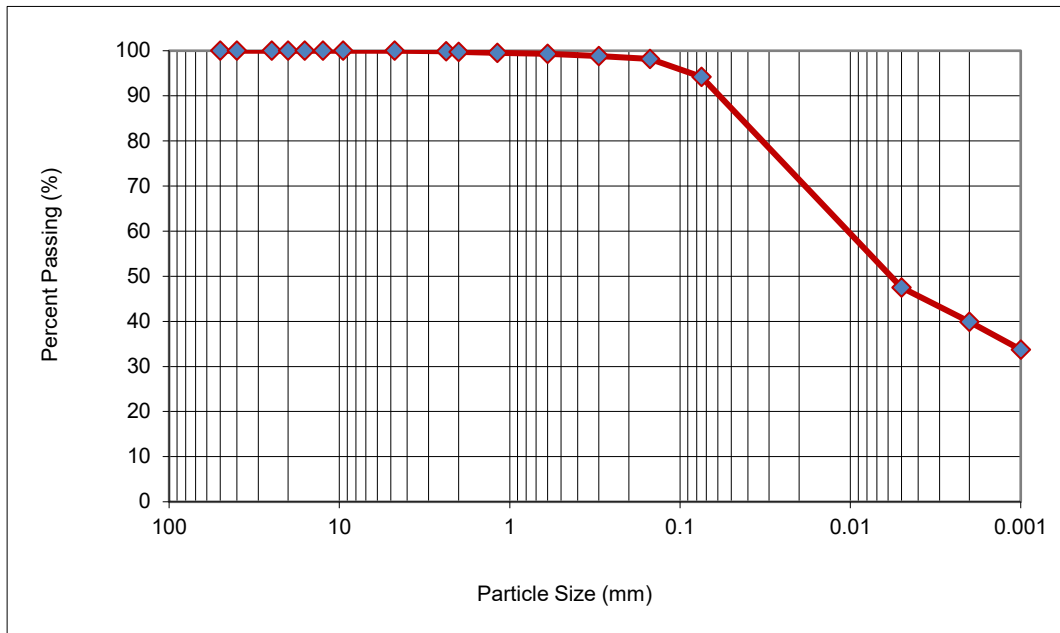
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-198. 1.2 m, Charles St.

STANTEC SAMPLE NO. 5617



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	99.9
2.00	99.7
1.18	99.6
0.600	99.3
0.300	98.8
0.150	98.2
0.075	94.2
0.005	47.5
0.002	39.9
0.001	33.7

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.3	0.7	4.8	54.3	39.9	33.7

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 8

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.24

SAMPLED BY: Stantec Consulting Ltd.

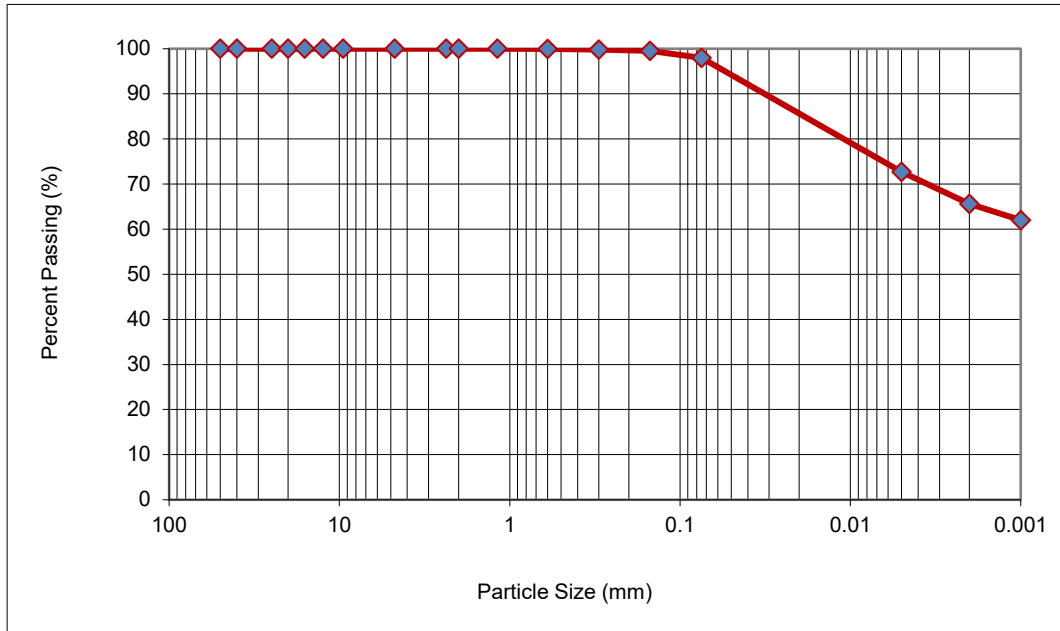
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-199. 0.9 m, Charles St.

STANTEC SAMPLE NO. 5618



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	100.0
1.18	100.0
0.600	100.0
0.300	99.8
0.150	99.5
0.075	98.0
0.005	72.7
0.002	65.6
0.001	62.0

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.0	0.1	1.9	32.4	65.6	62.0

COMMENTS

No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D7928 - PARTICLE-SIZE DISTRIBUTION OF FINE-GRAINED SOILS USING THE SEDIMENTATION ANALYSIS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Avenue
 Winnipeg, Manitoba
 R3E 2P1

PROJECT 2025 Local Street Renewal Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 9

DATE SAMPLED: 2025.Jan.07

DATE RECEIVED: 2025.Jan.07

DATE TESTED: 2025.Jan.27

SAMPLED BY: Stantec Consulting Ltd.

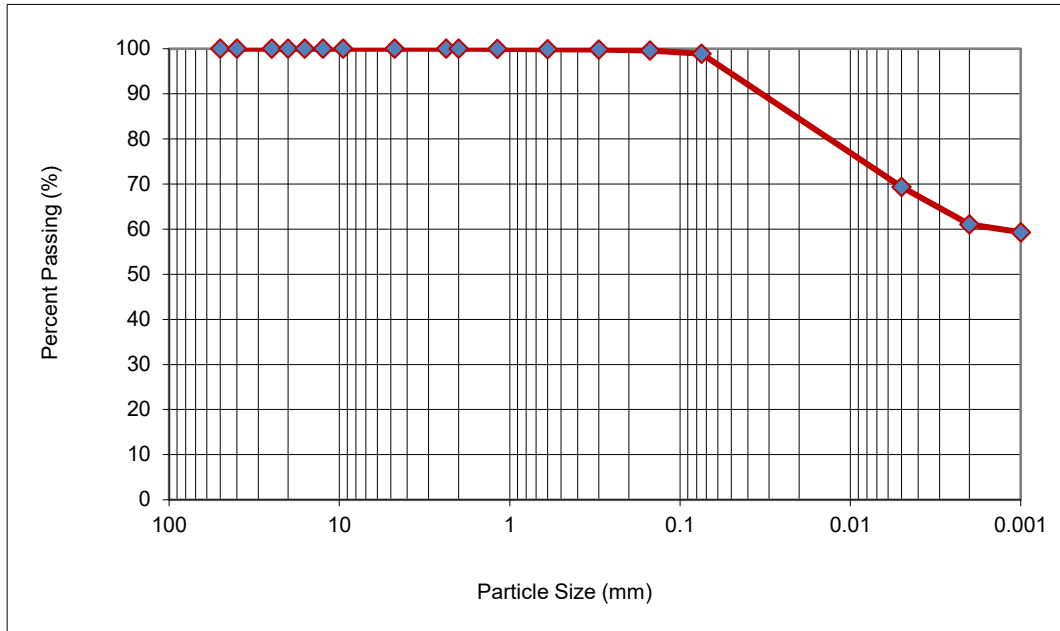
SUBMITTED BY: Stantec Consulting Ltd.

TESTED BY: Rimanshi Gorasiya

MATERIAL IDENTIFICATION

CLIENT FIELD ID BH-200. 1.1 m, Charles St.

STANTEC SAMPLE NO. 5619



Sieve Size (mm)	% Passing
50.0	100.0
40.0	100.0
25.0	100.0
20.0	100.0
16.0	100.0
12.5	100.0
9.5	100.0
4.75	100.0
2.36	100.0
2.00	100.0
1.18	100.0
0.600	99.9
0.300	99.8
0.150	99.6
0.075	98.9
0.005	69.3
0.002	61.1
0.001	59.3

Gravel	Sand			Silt	Clay	Colloids
	Coarse	Medium	Fine			
0.0	0.0	0.1	1.0	37.8	61.1	59.3

COMMENTS
 No comments.



REPORT DATE 2025.Feb.06

REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

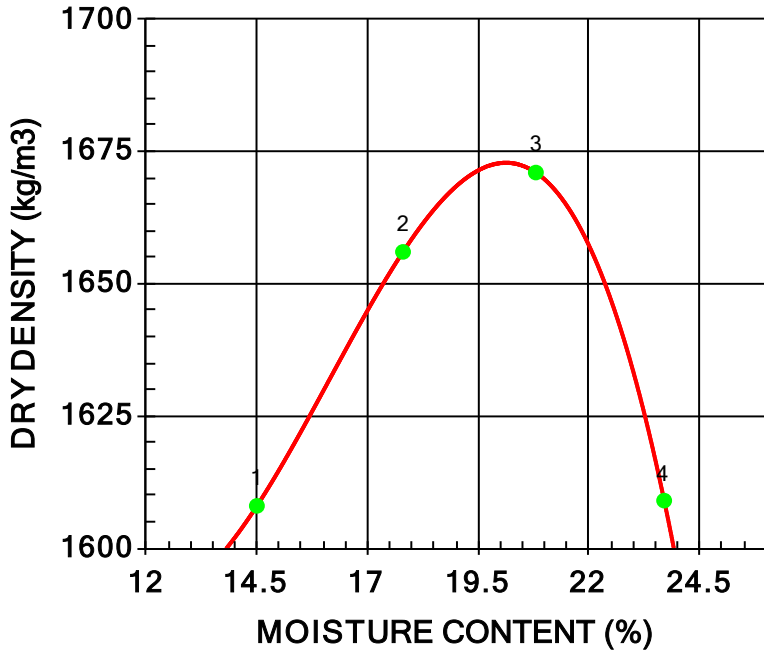
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 1 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.22

INSITU MOISTURE	26.7 %	COMPACTION STANDARD	Standard Proctor, ASTM D698
TESTED BY	Donald Eliazar	COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MATERIAL IDENTIFICATION		RAMMER TYPE	Manual
MAJOR COMPONENT	Subgrade	PREPARATION	Moist
SIZE	Fat Clay (CH)	OVERSIZE CORRECTION METHOD	None
DESCRIPTION		RETAINED 4.75mm SCREEN	N/A %
SUPPLIER	Existing Materials		
SOURCE	BH-192, 1.2 m (Aikins St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1841	1608	14.5
2	1951	1656	17.8
3	2018	1671	20.8
4	1990	1609	23.7

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1670	20.0
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5611.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

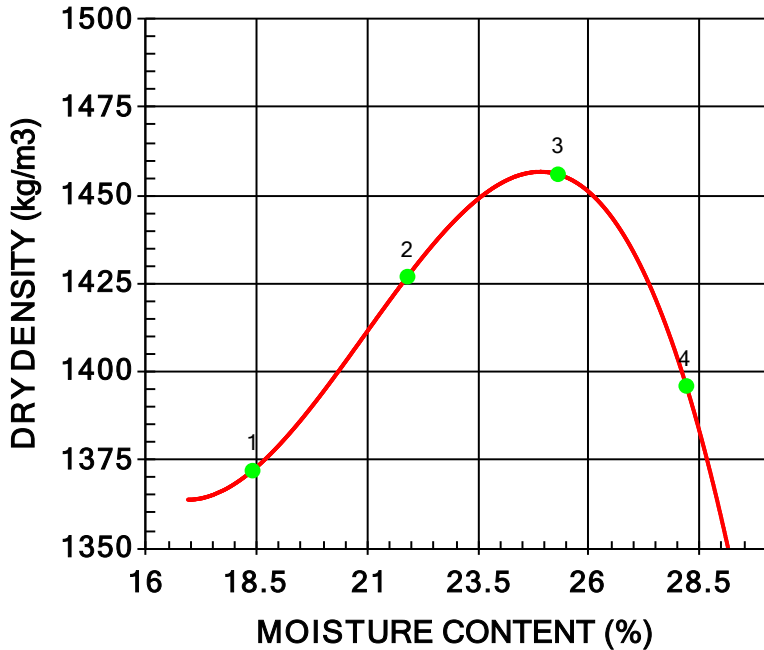
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 2 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.22

INSITU MOISTURE	40.9 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-193, 1.2 m (Aikins St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1624	1372	18.4
2	1740	1427	21.9
3	1824	1456	25.3
4	1790	1396	28.2

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1460	25.0
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5612.



Jason Thompson, C.E.T.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

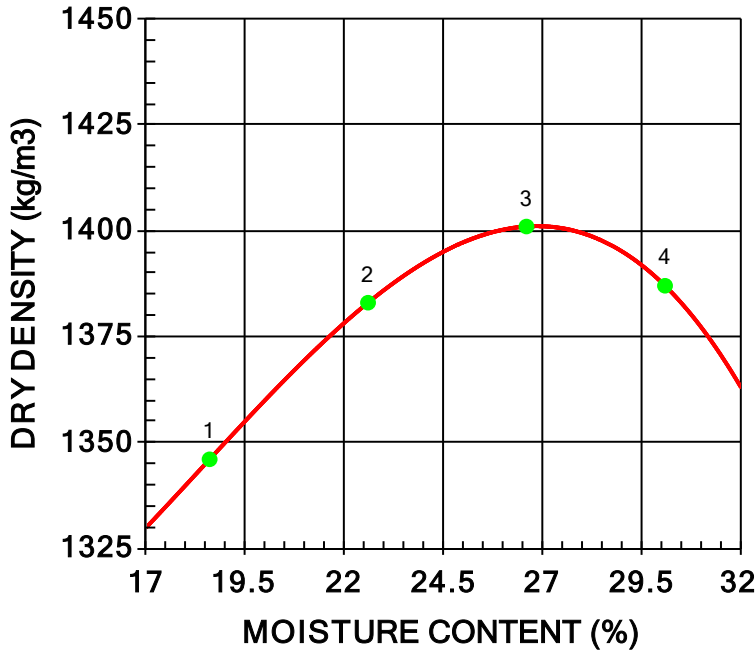
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 3 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.22

INSITU MOISTURE	46.3 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-194, 1.2 m (Aikins St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1596	1346	18.6
2	1695	1383	22.6
3	1774	1401	26.6
4	1804	1387	30.1

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1400	27.0
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5613.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

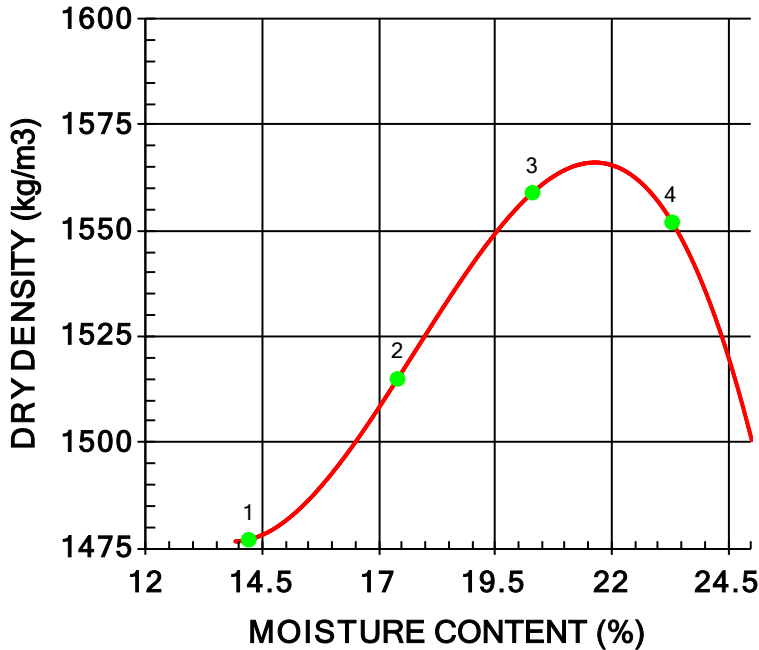
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 4 DATE SAMPLED 2025.Jan.09 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.23

INSITU MOISTURE	27.8 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-195, 1.2 m (Robinson St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1687	1477	14.2
2	1779	1515	17.4
3	1876	1559	20.3
4	1913	1552	23.3

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1570	21.5
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5614.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

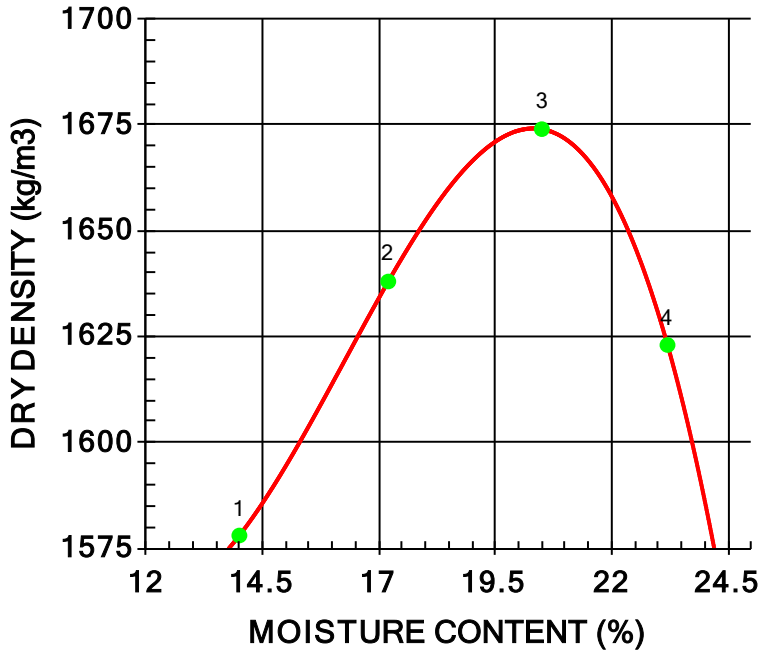
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 5 DATE SAMPLED 2025.Jan.09 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.23

INSITU MOISTURE	35.5 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Silty Clay (CL-ML)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-196, 1.0 m (Robinson St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1799	1578	14.0
2	1920	1638	17.2
3	2017	1674	20.5
4	2000	1623	23.2

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1670	20.5
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5615.



Jason Thompson, C.E.T.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

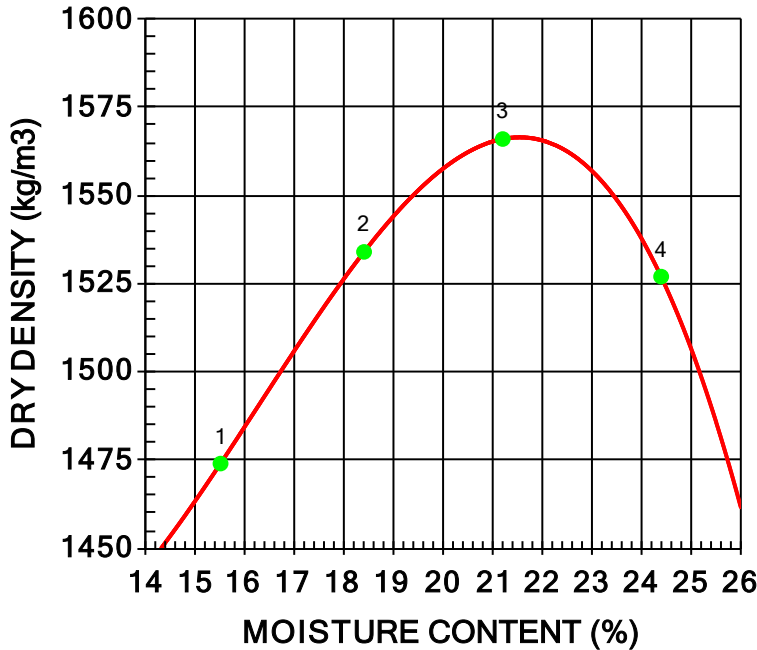
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 6 DATE SAMPLED 2025.Jan.09 DATE RECEIVED 2025.Jan.09 DATE TESTED 2025.Jan.24

INSITU MOISTURE	31.6 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Silt (ML)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-197, 1.0 m (Robinson St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1703	1474	15.5
2	1816	1534	18.4
3	1898	1566	21.2
4	1900	1527	24.4

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1570	21.5
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5616.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

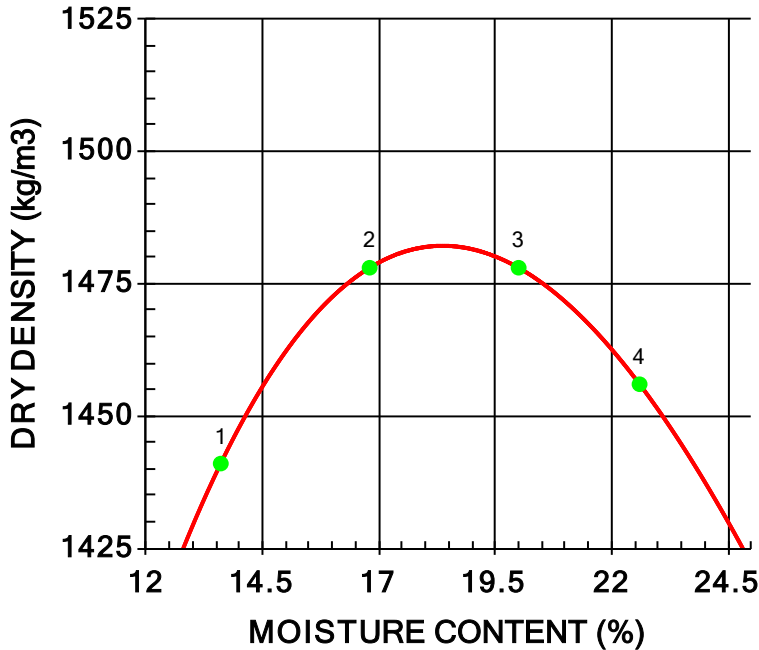
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 7 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.07 DATE TESTED 2025.Jan.24

INSITU MOISTURE	23.8 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-198, 1.2 m (Charles St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1637	1441	13.6
2	1726	1478	16.8
3	1774	1478	20.0
4	1785	1456	22.6

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1480	18.5
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5617.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

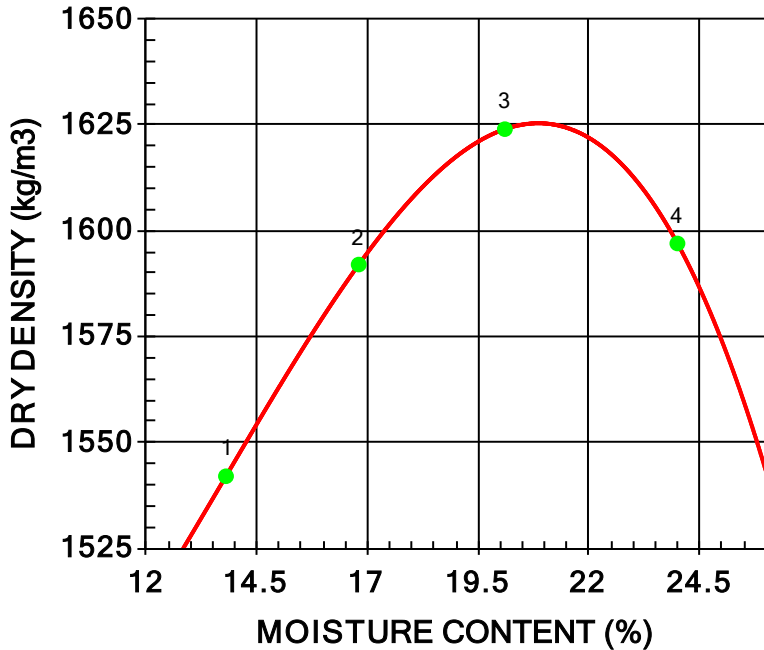
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 8 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.07 DATE TESTED 2025.Jan.24

INSITU MOISTURE	24.2 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-199, 0.9 m (Charles St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1755	1542	13.8
2	1859	1592	16.8
3	1950	1624	20.1
4	1980	1597	24.0

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1620	21.0
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5618.

PROCTOR TEST REPORT

TO City of Winnipeg
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 2P1

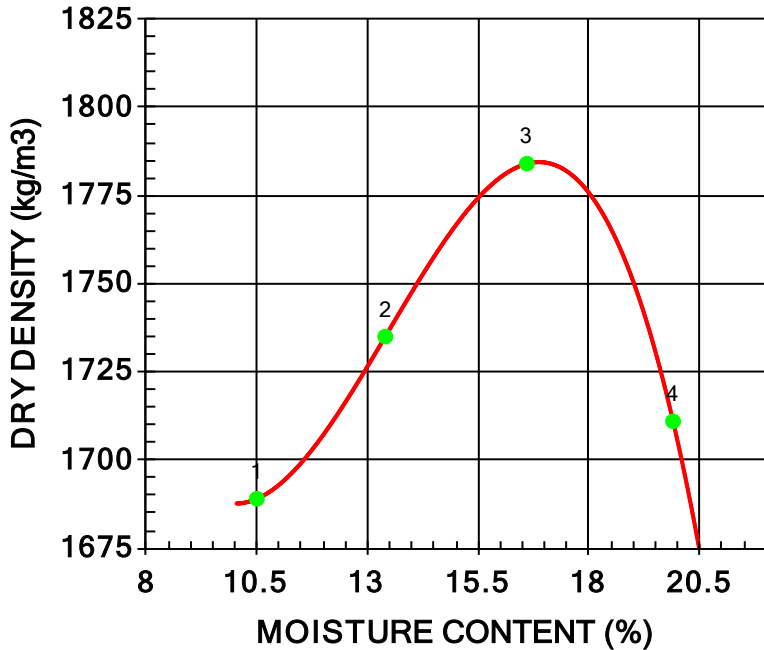
CLIENT City of Winnipeg
 C.C.

ATTN: Geoff Kerr

PROJECT 2025 Local Street Renewal Program

PROJECT NO. 123317463-2 - Contract 2
 PROCTOR NO. 9 DATE SAMPLED 2025.Jan.07 DATE RECEIVED 2025.Jan.07 DATE TESTED 2025.Jan.24

INSITU MOISTURE	23.7 %	COMPACTION STANDARD	Standard Proctor, ASTM
TESTED BY	Donald Eliazar		D698
MATERIAL IDENTIFICATION		COMPACTION PROCEDURE	A: 101.6mm Mold, Passing 4.75mm
MAJOR COMPONENT	Subgrade	RAMMER TYPE	Manual
SIZE	Fat Clay (CH)	PREPARATION	Moist
DESCRIPTION		OVERSIZE CORRECTION METHOD	None
SUPPLIER	Existing Materials	RETAINED 4.75mm SCREEN	N/A %
SOURCE	BH-200, 1.1 m (Charles St)		



TRIAL NUMBER	WET DENSITY (kg/m³)	DRY DENSITY (kg/m³)	MOISTURE CONTENT (%)
1	1866	1689	10.5
2	1967	1735	13.4
3	2080	1784	16.6
4	2051	1711	19.9

	MAXIMUM DRY DENSITY (kg/m³)	OPTIMUM MOISTURE CONTENT (%)
CALCULATED	1780	17.0
OVERSIZE CORRECTED		

COMMENTS

Stantec Sample No. 5619.

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 3P1

PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 1

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

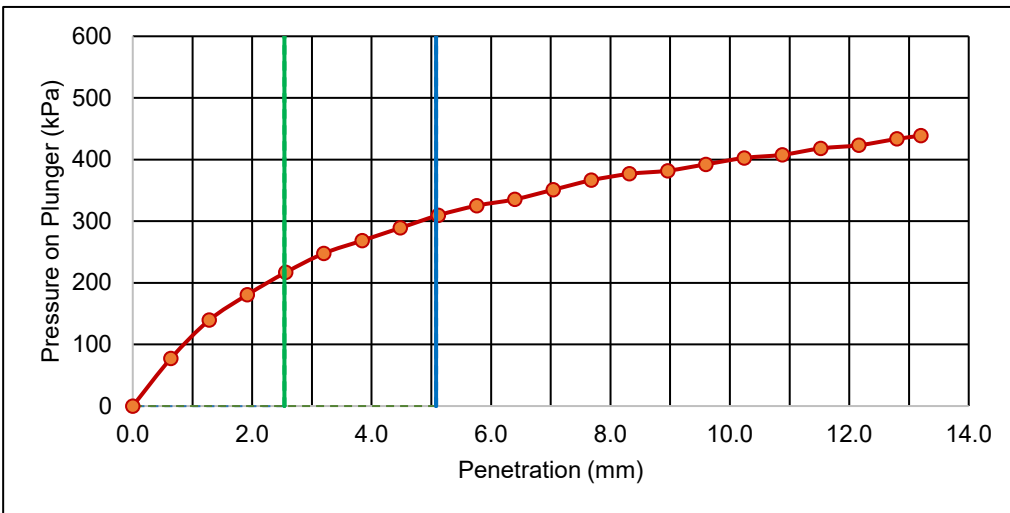
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.24
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-192, 1.2 m - Aikins St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5611

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1670 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	20.0 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1585 kg/m ³
SWELL OF SAMPLE	2.42 %	AS-COMPACTED MOISTURE	20.1 %
POST-TEST MOISTURE	28.7 %	AS-COMPACTED % COMPACTION	95 %




CBR VALUE AT 2.54 mm PENETRATION
3.1

CBR VALUE AT 5.08 mm PENETRATION
3.1

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 3P1

PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 2

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

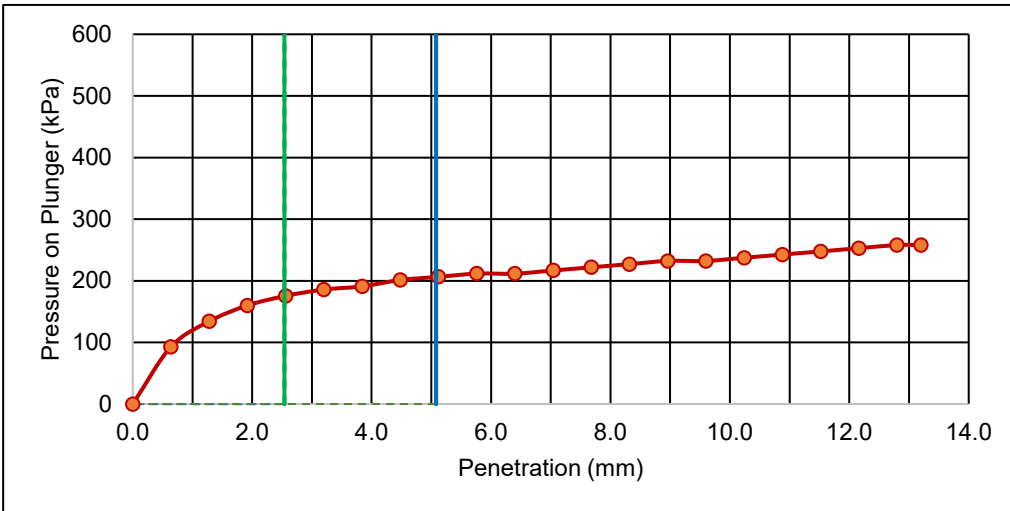
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.24
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-193, 1.2 m - Aikins St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5612

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1460 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	25.0 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1385 kg/m ³
SWELL OF SAMPLE	3.12 %	AS-COMPACTED MOISTURE	25.2 %
POST-TEST MOISTURE	41.7 %	AS-COMPACTED % COMPACTION	95 %




CBR VALUE AT 2.54 mm PENETRATION
2.5

CBR VALUE AT 5.08 mm PENETRATION
2.1

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

 TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
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 R3E 3P1

 PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 3

 DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

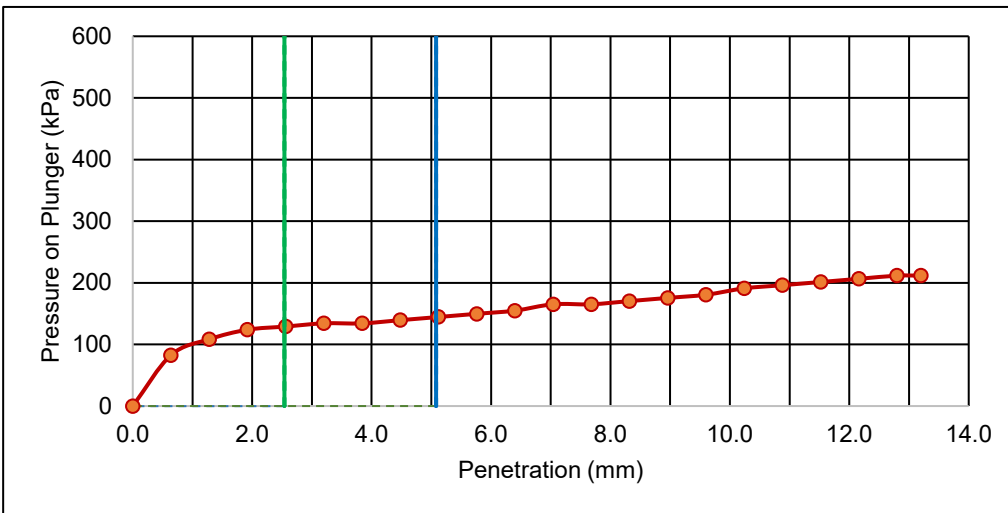
 DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

 DATE TESTED: 2025.Jan.24
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-194, 1.2 m - Aikins St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5613

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1400 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	27.0 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1330 kg/m ³
SWELL OF SAMPLE	4.30 %	AS-COMPACTED MOISTURE	27.1 %
POST-TEST MOISTURE	44.1 %	AS-COMPACTED % COMPACTION	95 %




CBR VALUE AT 2.54 mm PENETRATION 1.9

CBR VALUE AT 5.08 mm PENETRATION 1.4

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10

 REVIEWED BY 
 Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

 TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
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 R3E 3P1

 PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 4

 DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

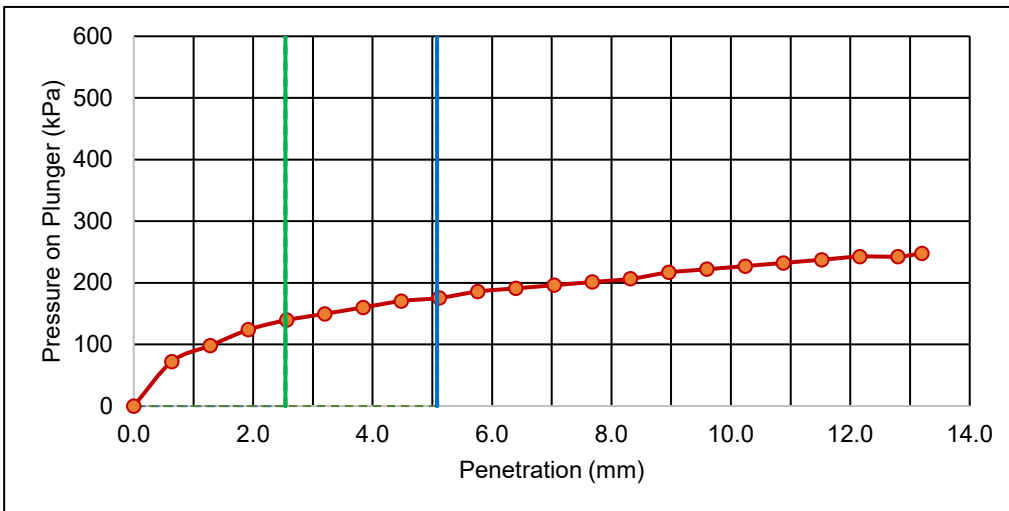
 DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

 DATE TESTED: 2025.Jan.27
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-195, 1.2 m - Robinson St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5614

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1570 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	21.5 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1490 kg/m ³
SWELL OF SAMPLE	5.34 %	AS-COMPACTED MOISTURE	21.6 %
POST-TEST MOISTURE	31.4 %	AS-COMPACTED % COMPACTION	95 %




**CBR VALUE AT 2.54 mm
PENETRATION**
2.0

**CBR VALUE AT 5.08 mm
PENETRATION**
1.8

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10

 REVIEWED BY 
 Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
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 R3E 3P1

PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 5

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

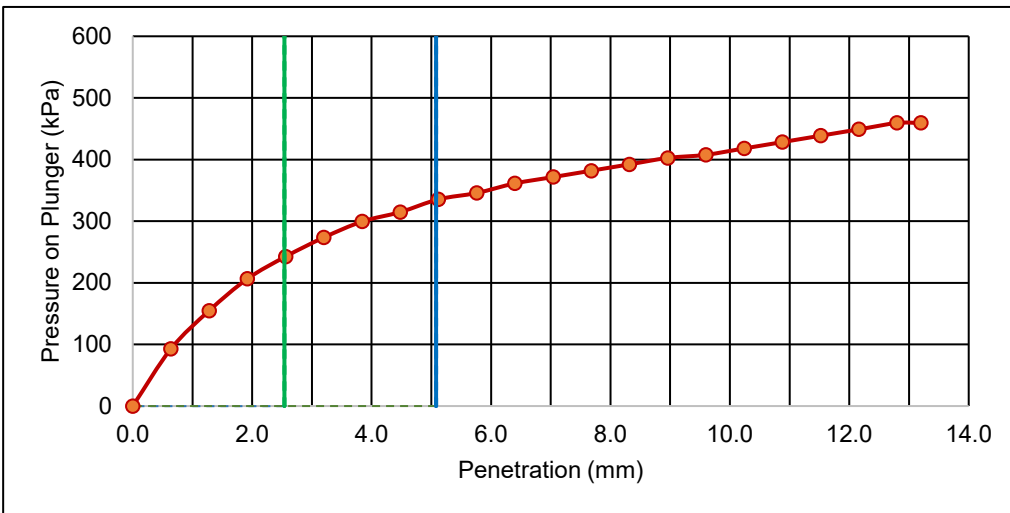
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.27
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Silty Clay (CL-ML)	SAMPLE LOCATION	BH-196, 1.0 m - Robinson St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5615

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1670 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	20.5 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1487 kg/m ³
SWELL OF SAMPLE	2.24 %	AS-COMPACTED MOISTURE	21.6 %
POST-TEST MOISTURE	24.4 %	AS-COMPACTED % COMPACTION	89 %




**CBR VALUE AT 2.54 mm
PENETRATION**
3.5

**CBR VALUE AT 5.08 mm
PENETRATION**
3.3

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
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PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 6

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

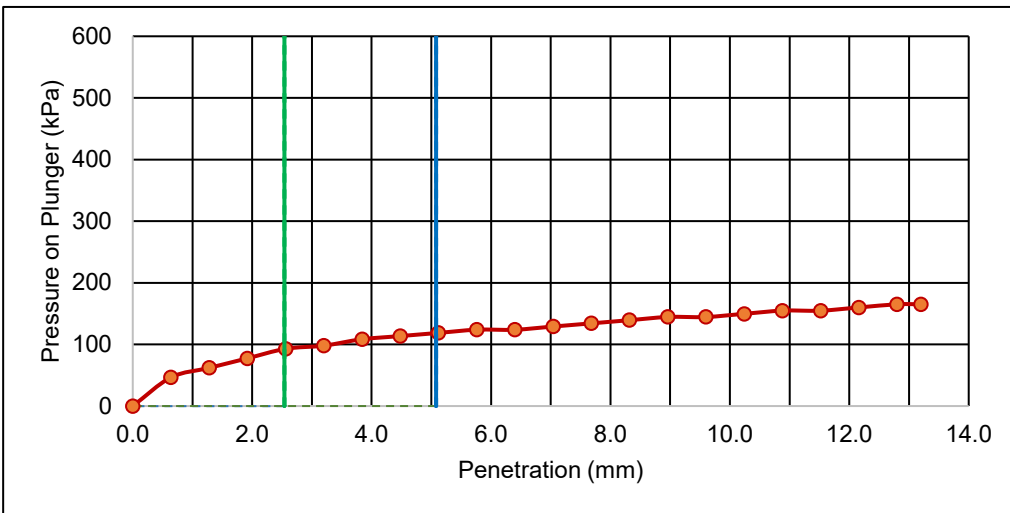
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.27
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Silt (ML)	SAMPLE LOCATION	BH-197, 1.0 m - Robinson St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5616

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1570 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	21.5 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1490 kg/m ³
SWELL OF SAMPLE	7.52 %	AS-COMPACTED MOISTURE	21.6 %
POST-TEST MOISTURE	35.6 %	AS-COMPACTED % COMPACTION	95 %




**CBR VALUE AT 2.54 mm
PENETRATION**
1.3

**CBR VALUE AT 5.08 mm
PENETRATION**
1.2

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
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 R3E 3P1

PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 7

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

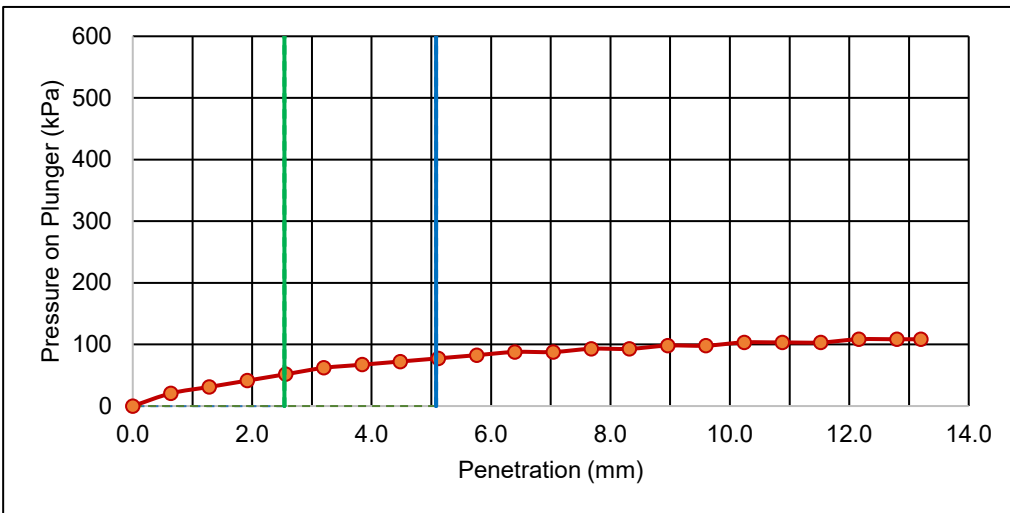
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.30
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-198, 1.2 m - Charles St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5617

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1480 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	18.5 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1406 kg/m ³
SWELL OF SAMPLE	9.26 %	AS-COMPACTED MOISTURE	18.6 %
POST-TEST MOISTURE	47.6 %	AS-COMPACTED % COMPACTION	95 %




**CBR VALUE AT 2.54 mm
PENETRATION**
0.7

**CBR VALUE AT 5.08 mm
PENETRATION**
0.8

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.
 As per ASTM D1883 10.2, the sample should be re-run to confirm the higher CBR value at 5.08 mm.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services

ASTM D1883 - CALIFORNIA BEARING RATIO (CBR) OF LABORATORY-COMPACTED SOILS

TO City of Winnipeg, Public Works Dept.
 104 - 1155 Pacific Ave.
 Winnipeg, MB
 R3E 3P1

PROJECT 2025 Local Street Renewals Program
 Contract 2

PROJECT NO. 123317463-2

ATTN Geoff Kerr

REPORT NO. 8

DATE SAMPLED: 2025.Jan.07
 SAMPLED BY: Larry Presado

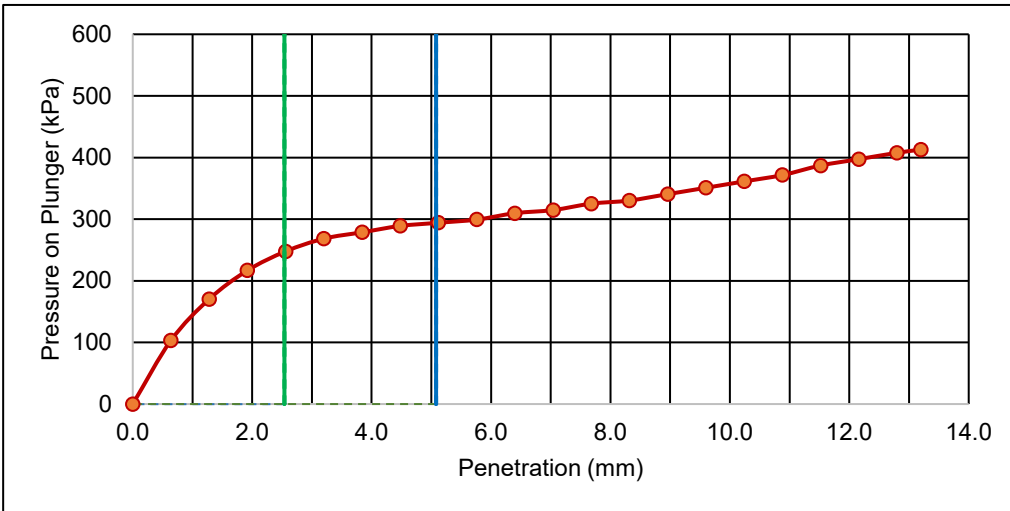
DATE RECEIVED: 2025.Jan.07
 SUBMITTED BY: Larry Presado

DATE TESTED: 2025.Jan.30
 TESTED BY: Donald Eliazar

MATERIAL IDENTIFICATION

MATERIAL USE	Subgrade	SUPPLIER	Existing Material
MAX. NOMINAL SIZE	< 4.75 mm	SOURCE	In Situ
MATERIAL TYPE	Fat Clay (CH)	SAMPLE LOCATION	BH-199, 0.9 m - Charles St.
SPECIFICATION ID	Not Applicable	STANTEC SAMPLE NO.	5618

IMMERSION PERIOD	96 ± 2 hr	TARGET MAX. DRY DENSITY	1620 kg/m ³
CONDITION OF SAMPLE	Soaked	TARGET OPTIMUM MOISTURE	21.0 %
SURCHARGE MASS	4.54 kg		
+19 mm OVERSIZE	0 %	AS-COMPACTED DRY DENSITY	1538 kg/m ³
SWELL OF SAMPLE	2.28 %	AS-COMPACTED MOISTURE	21.1 %
POST-TEST MOISTURE	28.4 %	AS-COMPACTED % COMPACTION	95 %




**CBR VALUE AT 2.54 mm
PENETRATION**
3.6

**CBR VALUE AT 5.08 mm
PENETRATION**
2.9

COMMENTS

Sample prepared to 95% of the maximum dry density at the optimum moisture content as determined from ASTM D698.

REPORT DATE 2025.Feb.10


 REVIEWED BY Guillaume Beauce, P.Eng.
 Geotechnical Engineer - Materials Testing Services