

# **APPENDIX A**

# **GEOTECHNICAL REPORT**



**Stantec Consulting Ltd.**

**2026 Local Streets Package 26-R-09**

**Prepared for:**

Ron Bruce, P.Eng.

Stantec Consulting Ltd.

500-311 Portage Ave

Winnipeg, MB

R3B 2B9

**Project Number:** 1000-240-07

**Date:** February 13, 2026



February 13, 2026

Our File No. 1000-240-07

Ron Bruce, P.Eng.  
Stantec Consulting Ltd.  
500-311 Portage Ave  
Winnipeg, MB  
R3B 2B9

**RE: 2026 Local Streets Package 26-R-09**

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TREK Engineering Inc. is pleased to submit our Final Report for the geotechnical investigation for 2026 Local Streets Package 26-R-09 project.

Please contact the undersigned should you have any questions.

Sincerely,

**TREK Engineering Inc.**  
**Per:**

A handwritten signature in blue ink, appearing to read "N. Ferreira", is written over the typed name.

Nelson John Ferreira, Ph.D., P.Eng., FEC  
Senior Geotechnical Engineer

Encl.

## Revision History

Revision No.	Author	Issue Date	Description
0	KF	February 13, 2026	Final Report

## Authorization Signatures

Prepared By: *Kate Franklin*  
Kate Franklin M.Sc. (Geotechnical Engineering)  
Technical Support Specialist



Reviewed By: \_\_\_\_\_  
Nelson John Ferreira, Ph.D., P.Eng., FEC  
Senior Geotechnical Engineer



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## 1.0 Introduction

This report summarizes the results of the road investigation completed for the City of Winnipeg Industrial Steets Renewals project. The project included drilling test holes and collecting pavement cores along several streets. The test hole information collected describes the pavement structure of the existing road as well as the soil stratigraphy beneath the pavement structure. The investigation was carried out following the City of Winnipeg RFP No. 751-2023 (Section E3 – Site Investigation Requirements).

## 2.0 Road Investigation

The investigation included coring of pavement at 79 locations on 11 different streets with drilling of test holes occurring at 6 locations along 2 streets. TH26-01 could not be drilled due to the presence of above and below ground utilities, but the pavement was cored. The investigation locations are shown on Figures 01 to 10 (attached), and the table below summarizes the investigation program per street.

**Table 1: Road Investigation Program**

2026 Local Streets Package 26-R-09	# of Locations	Investigation
Parkville Dr. – Pullberry Dr to Dunkirk Dr	4	3 Test Holes to 3.0m* 4 Cores
<del>St. Vital Park Roads (North, Central, South)</del>	<del>23</del>	<del>3 Test Holes to 3.0m 23 Cores</del>
<del>Glen Ave – Dunkirk Dr to Killarney St</del>	<del>5</del>	<del>5 Cores</del>
Beliveau Rd – Dakota St to Kearney St	9	9 Cores
Berrydale Ave – St. Anne’s Rd to East Limit	5	5 Cores
<del>Sandralin Bay – Avalon Rd to North Limit</del>	<del>1</del>	<del>1 Core</del>
<del>English Pl – Metz St to Darwin St</del>	<del>4</del>	<del>4 Cores</del>
<del>Plumtree Pl – Meadwood Dr to South Limit</del>	<del>1</del>	<del>1 Core</del>
<del>Dells Cr – Wales Ave to Hazelwood Ave</del>	<del>3</del>	<del>3 Cores</del>
Beaverhill Blvd – Shamrock Dr to Fermor Ave	23	23 Cores
Beaverhill Blvd – Frontage Rd 956/940	1	1 Core

\*Original program included 4 Test Holes. One could not be drilled due to the presence of above and below ground utilities.

The road investigation was conducted between January 6, 2026 to January 28, 2026. The pavement structure (asphalt/concrete) was cored by Tyler Green and Arshpreet Bhuller of TREK Engineering Inc. (TREK) using a portable coring press equipped with a hollow 150 mm diameter diamond core drill bit. The test holes were drilled by Maple Leaf Drilling to a depth of approximately 3.0 m below road surface using a truck mounted drill rig equipped with 150 mm diameter solid stem augers. The sub-surface conditions were observed during drilling and visually classified by Kate Franklin of TREK. Other pertinent information, such as groundwater and drilling conditions, was also recorded during the drilling investigation. Disturbed (auger cuttings) samples and bulk samples retrieved during the sub-surface investigation were transported to TREK’s material testing laboratory for further testing. Pavement core samples were also retrieved and logged at TREK’s material testing laboratory.

Core and test hole logs noted on the summary tables and test hole locations are based on UTM coordinates obtained using a hand-held GPS, and their location relative to the nearest address or intersection, measured distance from the edge of pavement, or other permanent features.

The laboratory testing program consisted of moisture content determination on all samples, as well as Atterberg Limits, and grain size analysis (mechanical sieve and hydrometer methods) on select samples between 0.6 and 0.9 m below pavement as well as Standard Proctor and CBR testing. Information gathered for each street package is included in separate appendices (Appendices A to K). The information provided in the Appendices includes test hole logs, laboratory testing summary tables and results, photos of the concrete cores, and summary of pavement core compressive strength.

Six CBR’s were completed on bulk samples of the soil units present below the pavement. Tests were performed on clay and silt layers encountered within the prescribed sample depth for CBR testing and the results are shown in the table below.

**Table 2: CBR Testing Summary**

Soil Unit	Street	Depth (m)	SPMDD (kg/m <sup>3</sup> )	Opt. Moisture (%)	Percent Proctor (%)	Moisture Content (%)	CBR Value at 2.54 mm	CBR Value at 5.08 mm
Clay	Parkville Dr: TH26-02	1.5-2.5	1358	33.7	94.8	34.1	1.3%	1.0%
Clay	Parkville Dr: TH26-03	0.6-2.5	1350	32.2	95.7	31.8	1.1%	1.0%
Clay	Parkville Dr: TH26-04	0.6-2.5	1388	30.4	94.9	30.4	1.5%	1.2%
<del>Clay</del>	<del>St Vital Park: TH26-05</del>	<del>0.6-2.5</del>	<del>1440</del>	<del>26.5</del>	<del>95.0</del>	<del>27.4</del>	<del>1.9%</del>	<del>1.6%</del>
<del>Clay</del>	<del>St Vital Park: TH26-06</del>	<del>0.6-2.5</del>	<del>1390</del>	<del>30.2</del>	<del>95.5</del>	<del>30.1</del>	<del>2.5%</del>	<del>1.8%</del>

<del>Clay</del>	<del>St Vital Park: TH26-07</del>	<del>0.6-2.5</del>	<del>1418</del>	<del>28.8</del>	<del>95.5</del>	<del>28.5</del>	<del>2.8%</del>	<del>2.1%</del>
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The test hole logs include a description of the soil units encountered during drilling and other pertinent information such as groundwater conditions and a summary of the laboratory testing results. The soils were classified in general accordance with the Unified Soil Classification System (USCS) and the AASHTO soil classification system (American Association of state highway and transportation officials). The AASHTO system classifies soils based on laboratory testing results from Atterberg Limits and grain size testing methods (hydrometer and mechanical sieve method). Where laboratory testing was not conducted, the AASHTO classification of the soils were interpreted based on a visual assessment as indicated with a (I) on the test hole logs and attached tables. For cohesive soils, the AASHTO system uses a combination of testing results to determine the Group Index of the soils and thus, were only determined where sufficient laboratory test data was available.

### 3.0 Closure

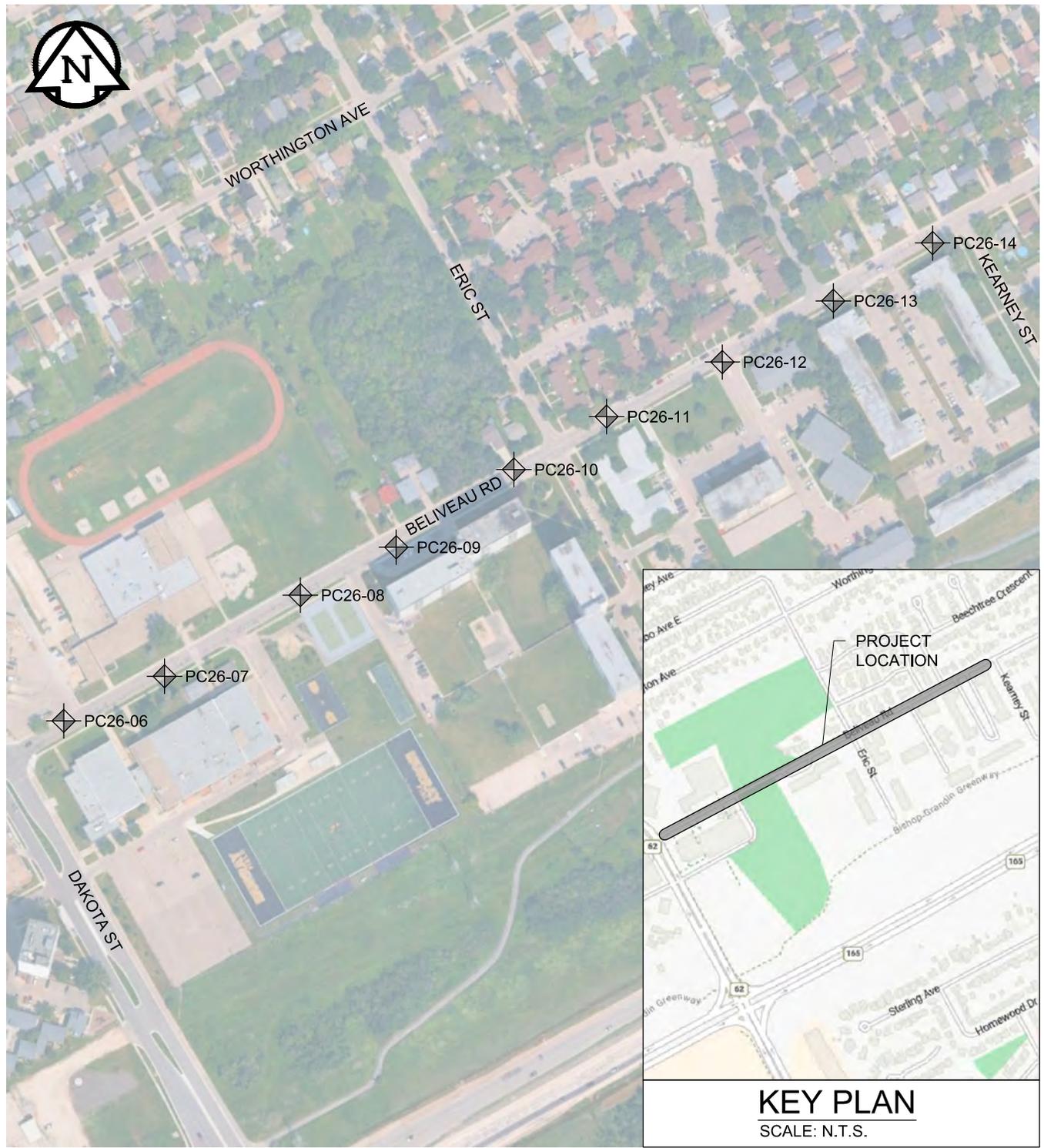
The information provided in this report is in accordance with current engineering principles and practices (Standard of Practice). The findings of this report were based on information provided (field investigation, laboratory testing, geometries). Soil conditions are natural deposits that can be highly variable across a site. If sub-surface conditions are different than the conditions previously encountered on-site or those presented here, we should be notified to adjust our findings if necessary.

All information provided in this report is subject to our standard terms and conditions for engineering services, a copy of which is provided to each of our clients with the original scope of work, or a mutually executed standard engineering services agreement. If these conditions are not attached, and you are not already in possession of such terms and conditions, contact our office and you will be promptly provided with a copy.

This report has been prepared by TREK Engineering Inc. (the Consultant) for the exclusive use of Stantec Consulting Ltd. (the Client) and their agents for the work product presented in the report. Any findings or recommendations provided in this report are not to be used or relied upon by any third parties, except as agreed to in writing by the Client and Consultant prior to use.

## Figures

Z:\Projects\1000 Soils Lab\1000-240 Stantec\1000-240 2026 Local Streets Package 26-R-09\3 Survey and Dwg\3.4 CAD\3.4.3 Working Folder\Figs 01 to 10 2026-02-10 Local Streets Package 26-R-09 0\_A 1000-240-07.dwg, 2026-02-10 11:02:32 AM



**LEGEND:**

◆ PAVEMENT CORE (TREK, 2026)

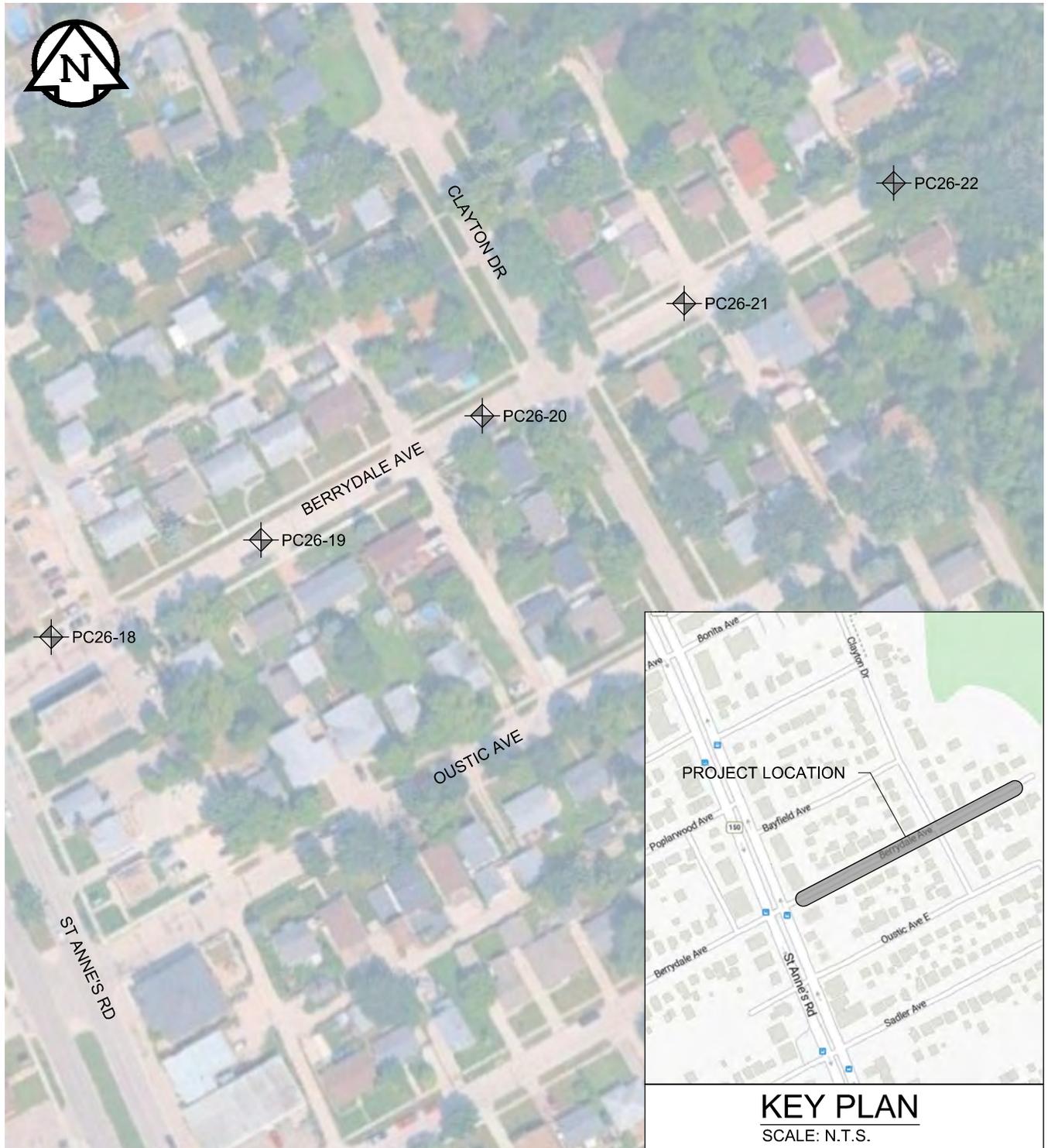
**NOTES:**

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).



**Figure 02**  
 Pavement Core Location Plan

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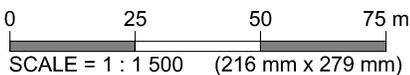


**LEGEND:**

◆ PAVEMENT CORE (TREK, 2026)

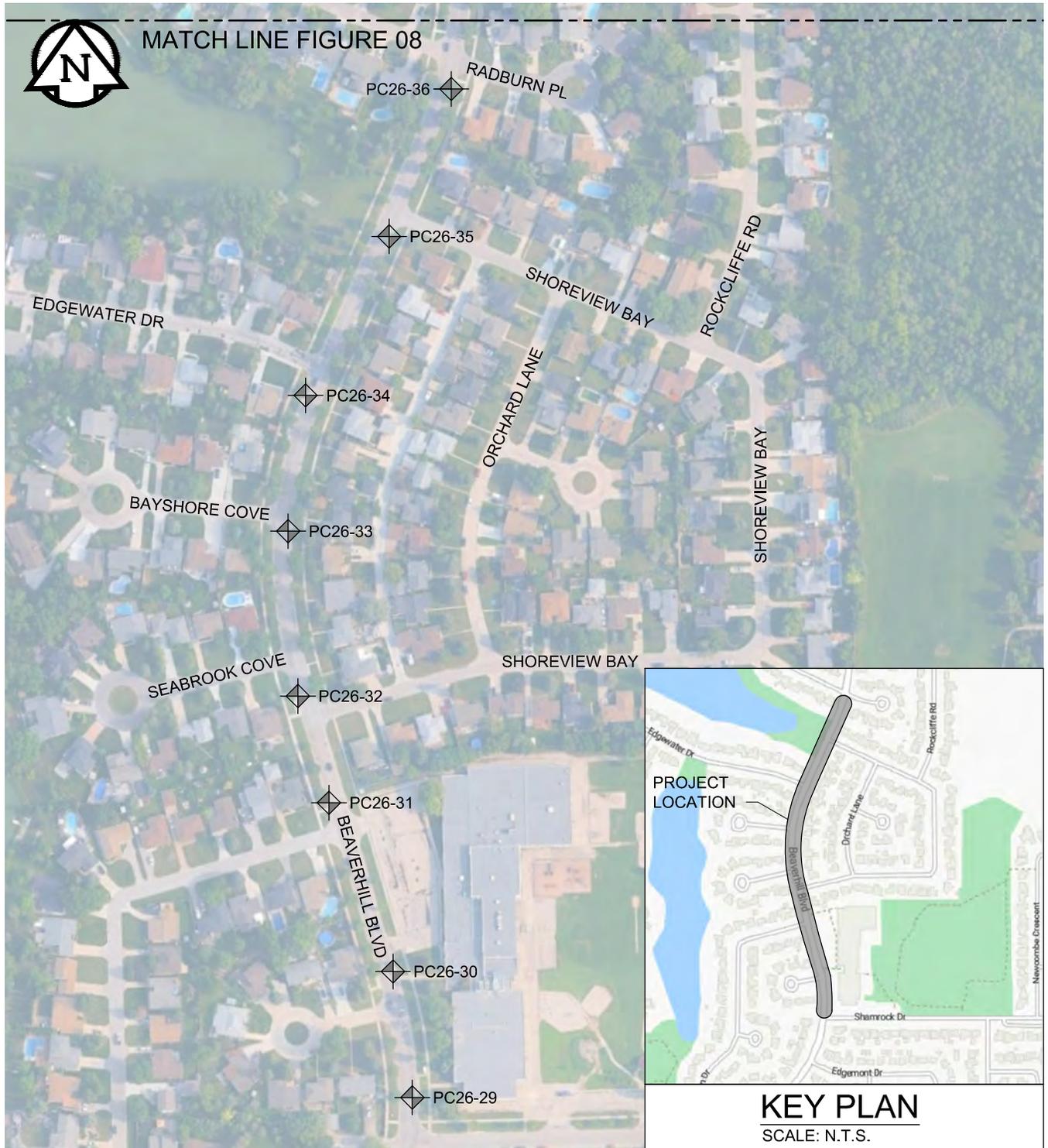
**NOTES:**

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).



**Figure 04**  
 Pavement Core Location Plan

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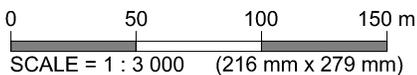


**LEGEND:**

◆ PAVEMENT CORE (TREK, 2026)

**NOTES:**

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).



**Figure 07**  
 Pavement Core Location Plan



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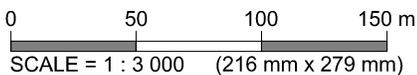


**LEGEND:**

◆ PAVEMENT CORE (TREK, 2026)

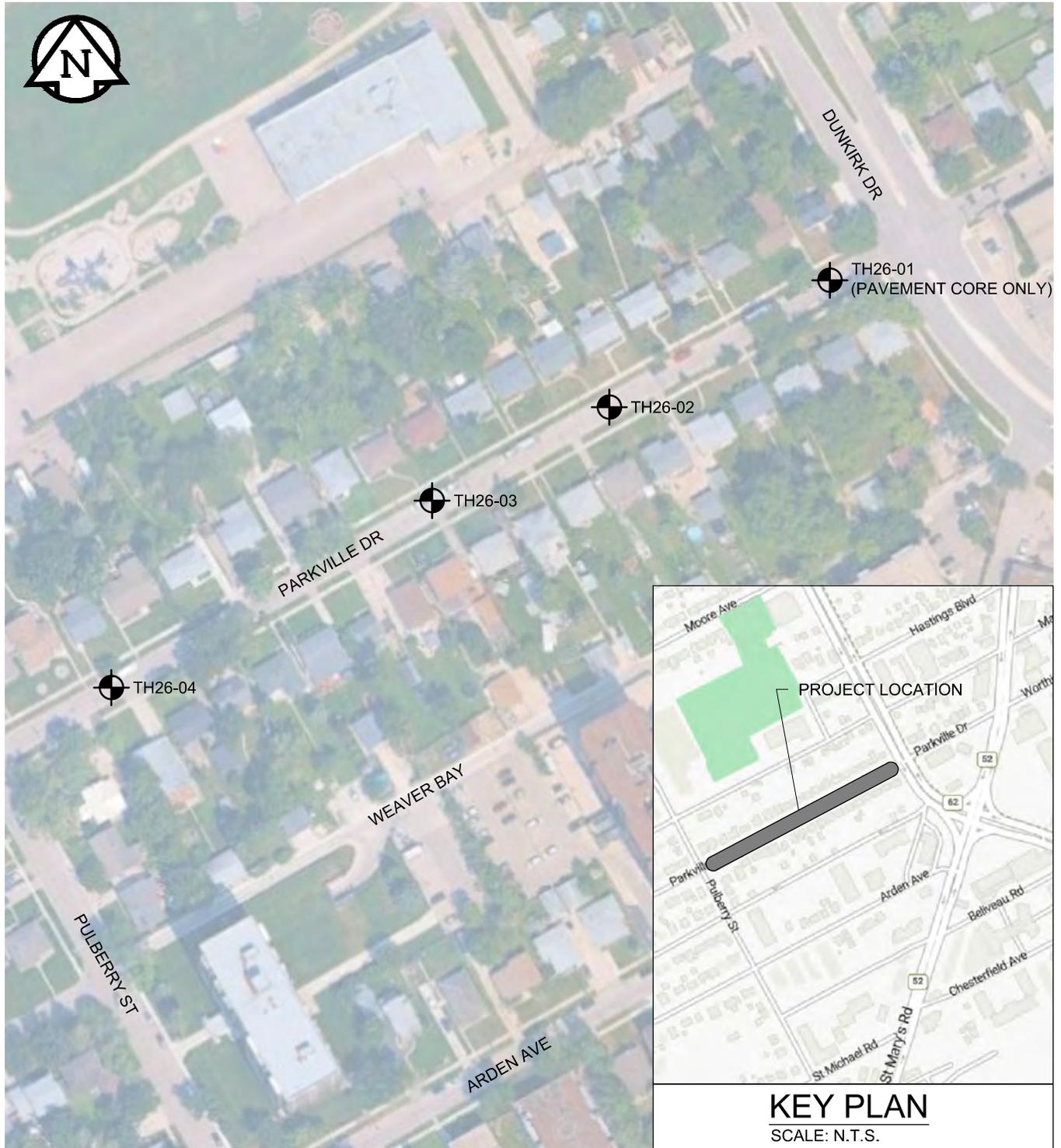
**NOTES:**

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).



**Figure 08**  
 Pavement Core Location Plan

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**LEGEND:**

TEST HOLE (TREK, 2026)

**NOTES:**

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).

0 25 50 75 m  
 SCALE = 1 : 1 750 (216 mm x 279 mm)

**Figure 10**  
 Test Hole Location Plan

**Appendix A**  
**Test Hole Logs, Summary Tables, Lab Testing Results and**  
**Photographs of Pavement Core Samples**  
**Parkville Drive – Pullberry Drive to Dunkirk Drive**

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**Client:** Stantec Consulting Ltd. **Project Number:** 1000-240-07  
**Project Name:** 2026 Locals Streets Package 26-R-09 **Location:** UTM : 5522120 m N, 635507 m E, #39 Parkville Dr  
**Contractor:** Maple Leaf Drilling Ltd. **Ground Elevation:** Top of Pavement  
**Method:** 150 mm Solid Stem Auger, B40 Truck Mount **Date Drilled:** January 14, 2026

**Sample Type:**  Grab (G)  Shelby Tube (T)  Split Spoon (SS) / SPT  Split Barrel (SB) / LPT  Core (C)

**Particle Size Legend:**  Fines  Clay  Silt  Sand  Gravel  Cobbles  Boulders

Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m <sup>3</sup> )		Particle Size (%)		Undrained Shear Strength (kPa)								
					16	17	18	19	20	21	0	50	100	150	200	250	
0.0		ASPHALT (45mm thick)															
0.0		CONCRETE (145mm thick)															
0.0		CLAY - silty, trace sand - black - frozen, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (I)															
0.7		CLAY and SILT - trace sand - light brown - frozen to 1.2m, moist, firm to stiff when thawed - low to intermediate plasticity - AASHTO: A-6 (17)	G30														
1.1		CLAY and SILT - trace sand - light brown - frozen to 1.2m, moist, firm to stiff when thawed - low to intermediate plasticity - AASHTO: A-6 (17)	G31														
1.5		CLAY - silty, trace sand - brown - moist, stiff - high plasticity - AASHTO: A-7-6 (I)	G32														
1.9		CLAY - silty, trace sand - brown - moist, stiff - high plasticity - AASHTO: A-7-6 (I)	G33														
2.3		CLAY - silty, trace sand - brown - moist, stiff - high plasticity - AASHTO: A-7-6 (I)	G34														
2.7		CLAY - silty, trace sand - brown - moist, stiff - high plasticity - AASHTO: A-7-6 (I)	G35														
3.0		SILT - trace clay, trace sand - light brown, moist, firm - low plasticity - AASHTO: A-6 (I)															

END OF TEST HOLE AT 3.0m IN SILT.

Notes:

- 1) Seepage and sloughing not observed.
- 2) Test hole dry and open to 3.0m depth immediately after drilling.
- 3) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
- 4) Bulk samples (B36 Clay, B37 Clay and Silt, and B38 Clay) obtained from 0.7m to 1.1m, 1.1m to 1.5m, and 1.5m to 2.0m.

**Logged By:** Kate Franklin **Reviewed By:** \_\_\_\_\_ **Project Engineer:** Nelson Ferreira

**Client:** Stantec Consulting Ltd.      **Project Number:** 1000-240-07  
**Project Name:** 2026 Locals Streets Package 26-R-09      **Location:** UTM : 5522084 m N, 635447 m E, #49 Parkville Dr  
**Contractor:** Maple Leaf Drilling Ltd.      **Ground Elevation:** Top of Pavement  
**Method:** 150 mm Solid Stem Auger, B40 Truck Mount      **Date Drilled:** January 14, 2026

**Sample Type:**  Grab (G)     Shelby Tube (T)     Split Spoon (SS) / SPT     Split Barrel (SB) / LPT     Core (C)

**Particle Size Legend:**  Fines     Clay     Silt     Sand     Gravel     Cobbles     Boulders

Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m <sup>3</sup> )						Undrained Shear Strength (kPa)						
					16	17	18	19	20	21	Test Type						
					Particle Size (%)												
					0	20	40	60	80	100							
					PL      MC      LL  ----- ----- ----- -----            0    20    40    60    80    100												
					0    50    100    150    200    250						□ Torvane △ ⊕ Pocket Pen. ⊕ ⊠ Qu ⊠ ○ Field Vane ○						
0.0		ASPHALT (30mm thick)															
0.0		CONCRETE (170mm thick)															
0.0		CLAY - silty, trace sand - black - frozen to 1.0m, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (53)															
0.5																	
0.9		- grey below 0.9m															
1.0			G39														
1.1			G40														
1.5		- silt pocket (100mm diam.) at 1.5m - brown below 1.6m															
1.5			G41														
2.0			G42														
2.5			G43														
3.0			G44														

END OF TEST HOLE AT 3.0m IN CLAY.

Notes:

- 1) Seepage and sloughing not observed.
- 2) Test hole dry and open to 3.0m depth immediately after drilling.
- 3) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
- 4) Bulk sample (B45 Clay) obtained from 0.6m to 2.0m.

**Client:** Stantec Consulting Ltd. **Project Number:** 1000-240-07  
**Project Name:** 2026 Locals Streets Package 26-R-09 **Location:** UTM : 5522032 m N, 635354 m E, #71 Parkville Dr  
**Contractor:** Maple Leaf Drilling Ltd. **Ground Elevation:** Top of Pavement  
**Method:** 150 mm Solid Stem Auger, B40 Truck Mount **Date Drilled:** January 14, 2026

**Sample Type:**  Grab (G)  Shelby Tube (T)  Split Spoon (SS) / SPT  Split Barrel (SB) / LPT  Core (C)

**Particle Size Legend:**  Fines  Clay  Silt  Sand  Gravel  Cobbles  Boulders

Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m <sup>3</sup> )		Particle Size (%)		Undrained Shear Strength (kPa)								
					16	17	18	19	20	21	0	50	100	150	200	250	
0.0 - 0.05		ASPHALT (50mm thick)															
0.05 - 0.1		CONCRETE (150mm thick)															
0.1 - 0.6		CLAY - silty, trace sand - black - frozen to 1.0m, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (51)															
0.6 - 0.9		- grey below 0.9m		G23													
0.9 - 1.3		- brown, very stiff below 1.3m		G24													
1.3 - 2.0		- stiff below 2.0m		G25													
2.0 - 2.2				G26													
2.2 - 2.5				G27													
2.5 - 3.0				G28													

END OF TEST HOLE AT 3.0m IN CLAY.

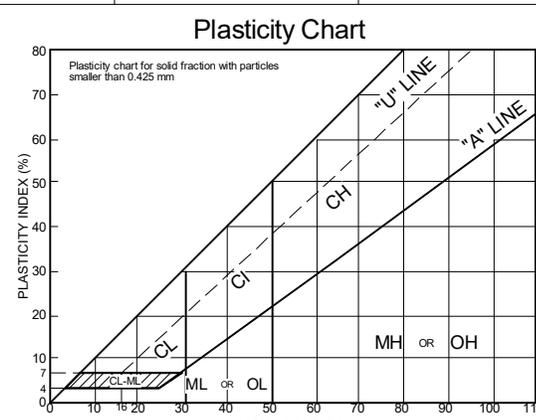
Notes:

- 1) Seepage and sloughing not observed.
- 2) Test hole dry and open to 3.0m depth immediately after drilling.
- 3) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
- 4) Bulk sample (B29 Clay) obtained from 0.6m to 2.0m.

SUB-SURFACE LOG LOGS 2026-01-14 LOCAL STREETS 1000-240-07 0 - A KF GPJ TREK.GDT 2/12/26

### GENERAL NOTES

- Classifications are based on the United Soil Classification System and include consistency, moisture, and color. Field descriptions have been modified to reflect results of laboratory tests where deemed appropriate.
- Descriptions on these test hole logs apply only at the specific test hole locations and at the time the test holes were drilled. Variability of soil and groundwater conditions may exist between test hole locations.
- When the following classification terms are used in this report or test hole logs, the primary and secondary soil fractions may be visually estimated.

Major Divisions	USCS Classification	Symbols	Typical Names	Laboratory Classification Criteria		Particle Size			
Coarse-Grained soils (More than half the material is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction is larger than 4.75 mm)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Determine percentages of sand and gravel from grain size curve, depending on percentage of fines (fraction smaller than No. 200 sieve) coarse-grained soils are classified as follows:  Less than 5 percent..... GW, GP, SW, SP More than 12 percent..... GM, GC, SM, SC 6 to 12 percent..... Borderline cases requiring dual symbols*	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3	ASTM Sieve sizes #10 to #4 #40 to #10 #200 to #40 < #200			
		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines		Not meeting all gradation requirements for GW				
		GM	Silty gravels, gravel-sand-silt mixtures		Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols			
		GC	Clayey gravels, gravel-sand-silt mixtures		Atterberg limits above "A" line or P.I. greater than 7				
	Sands (More than half of coarse fraction is smaller than 4.75 mm)	Clean sands (Little or no fines)	SW		Well-graded sands, gravelly sands, little or no fines	$C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3	mm 2.00 to 4.75 0.425 to 2.00 0.075 to 0.425 < 0.075		
			SP		Poorly-graded sands, gravelly sands, little or no fines	Not meeting all gradation requirements for SW			
		Sands with fines (Appreciable amount of fines)	SM		Silty sands, sand-silt mixtures	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols		
			SC		Clayey sands, sand-clay mixtures	Atterberg limits above "A" line or P.I. greater than 7			
			Fine-Grained soils (More than half the material is smaller than No. 200 sieve size)		Sils and Clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock floor, silty or clayey fine sands or clayey silts with slight plasticity		Particle Size ASTM Sieve Sizes mm > 300 75 to 300 19 to 75 4.75 to 19
						CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays		
OL	Organic silts and organic silty clays of low plasticity								
Sils and Clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or distomaceous fine sandy or silty soils, organic silts							
	CH	Inorganic clays of high plasticity, fat clays							
	OH	Organic clays of medium to high plasticity, organic silts							
	Pt	Peat and other highly organic soils		Von Post Classification Limit	Strong colour or odour, and often fibrous texture				
Highly Organic Soils					Material Boulders Cobbles Gravel Coarse Fine				

\* Borderline classifications used for soils possessing characteristics of two groups are designated by combinations of groups symbols. For example; GW-GC, well-graded gravel-sand mixture with clay binder.

### Other Symbol Types

	Asphalt		Bedrock (undifferentiated)		Cobbles
	Concrete		Limestone Bedrock		Boulders and Cobbles
	Fill		Cemented Shale		Silt Till
			Non-Cemented Shale		Clay Till

**LEGEND OF ABBREVIATIONS AND SYMBOLS**

LL - Liquid Limit (%)	▽ Water Level at Time of Drilling
PL - Plastic Limit (%)	▼ Water Level at End of Drilling
PI - Plasticity Index (%)	▽ Water Level After Drilling as Indicated on Test Hole Logs
MC - Moisture Content (%)	
SPT - Standard Penetration Test	
RQD- Rock Quality Designation	
Qu - Unconfined Compression	
Su - Undrained Shear Strength	
VW - Vibrating Wire Piezometer	
SI - Slope Incliner	

**FRACTION OF SECONDARY SOIL CONSTITUENTS ARE BASED ON THE FOLLOWING TERMINOLOGY**

TERM	EXAMPLES	PERCENTAGE
and	and CLAY	35 to 50 percent
"y" or "ey"	clayey, silty	20 to 35 percent
some	some silt	10 to 20 percent
trace	trace gravel	1 to 10 percent

**TERMS DESCRIBING CONSISTENCY OR COMPACTION CONDITION**

The Standard Penetration Test blow count (N) of a non-cohesive soil can be related to compactness condition as follows:

<u>Descriptive Terms</u>	<u>SPT (N) (Blows/300 mm)</u>
Very loose	< 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very dense	> 50

The Standard Penetration Test blow count (N) of a cohesive soil can be related to its consistency as follows:

<u>Descriptive Terms</u>	<u>SPT (N) (Blows/300 mm)</u>
Very soft	< 2
Soft	2 to 4
Firm	4 to 8
Stiff	8 to 15
Very stiff	15 to 30
Hard	> 30

The undrained shear strength (Su) of a cohesive soil can be related to its consistency as follows:

<u>Descriptive Terms</u>	<u>Undrained Shear Strength (kPa)</u>
Very soft	< 12
Soft	12 to 25
Firm	25 to 50
Stiff	50 to 100
Very stiff	100 to 200
Hard	> 200





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**Moisture Content Report  
 ASTM D2216-98**

**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Street Package 26-R-09 (Parkville Drive)

**Sample Date** 14-Jan-26  
**Test Date** 29-Jan-26  
**Technician** D. Sellick

Test Hole	TH26-02	TH26-02	TH26-02	TH26-02	TH26-02	TH26-02
Depth (m)	0.8 - 0.9	1.1 - 1.2	1.4 - 1.5	1.8 - 1.9	2.1 - 2.3	2.6 - 2.7
Sample #	G30	G31	G32	G33	G34	G35
Tare ID	E66	E12	W11	QT2	D228	FR67
Mass of tare	7.1	6.8	8.5	8.3	6.7	8.4
Mass wet + tare	166.5	467.2	175.2	165.0	207.7	180.0
Mass dry + tare	121.3	384.3	144.8	117.3	147.4	122.7
Mass water	45.2	82.9	30.4	47.7	60.3	57.3
Mass dry soil	114.2	377.5	136.3	109.0	140.7	114.3
Moisture %	39.6%	22.0%	22.3%	43.8%	42.9%	50.1%

Test Hole	TH26-03	TH26-03	TH26-03	TH26-03	TH26-03	TH26-03
Depth (m)	0.8 - 0.9	1.1 - 1.2	1.4 - 1.5	1.8 - 1.9	2.1 - 2.3	2.6 - 2.7
Sample #	G39	G40	G41	G42	G43	G44
Tare ID	E133	R34	F113	M08	P25	E57
Mass of tare	8.5	7.1	8.3	7.0	8.6	7.0
Mass wet + tare	160.3	458.2	170.7	185.4	175.5	169.7
Mass dry + tare	121.5	341.5	129.9	132.5	119.3	113.9
Mass water	38.8	116.7	40.8	52.9	56.2	55.8
Mass dry soil	113.0	334.4	121.6	125.5	110.7	106.9
Moisture %	34.3%	34.9%	33.6%	42.2%	50.8%	52.2%

Test Hole	TH26-04	TH26-04	TH26-04	TH26-04	TH26-04	TH26-04
Depth (m)	0.8 - 0.9	1.1 - 1.2	1.4 - 1.5	1.8 - 1.9	2.1 - 2.3	2.6 - 2.7
Sample #	G23	G24	G25	G26	G27	G28
Tare ID	QT81	QW16	QT87	ZD	N45	W55
Mass of tare	8.3	6.7	8.5	6.7	8.5	8.5
Mass wet + tare	198.1	471.1	179.4	201.7	161.8	182.1
Mass dry + tare	147.1	348.4	136.1	152.2	115.4	128.5
Mass water	51.0	122.7	43.3	49.5	46.4	53.6
Mass dry soil	138.8	341.7	127.6	145.5	106.9	120.0
Moisture %	36.7%	35.9%	33.9%	34.0%	43.4%	44.7%



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**Atterberg Limits**  
**ASTM D4318-17e1**

**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)

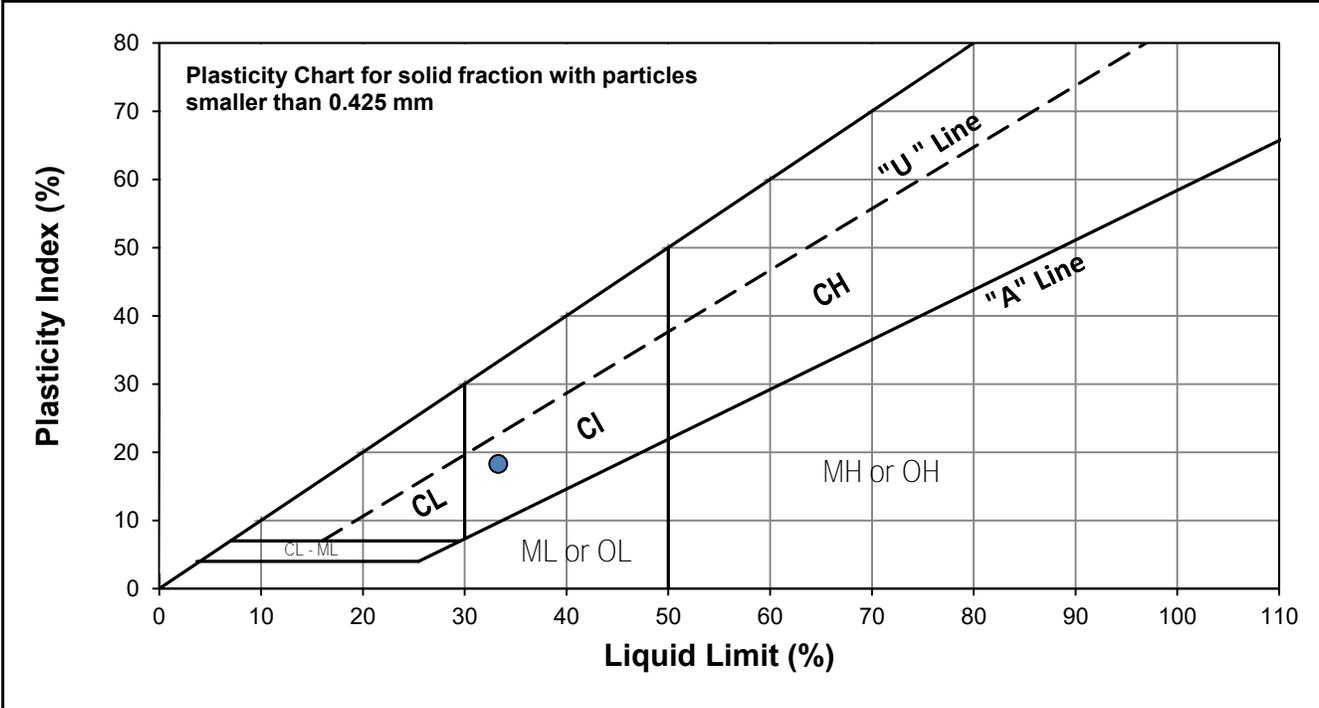


**Test Hole** TH26-02  
**Sample #** G31  
**Depth (m)** 1.1 - 1.2  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Liquid Limit</b>	33
<b>Plastic Limit</b>	15
<b>Plasticity Index</b>	18

**Liquid Limit**

Trial #	1	2	3
<b>Number of Blows (N)</b>	22	28	34
<b>Mass Tare (g)</b>	13.951	14.136	14.075
<b>Mass Wet Soil + Tare (g)</b>	25.231	27.747	26.901
<b>Mass Dry Soil + Tare (g)</b>	22.368	24.382	23.858
<b>Mass Water (g)</b>	2.863	3.365	3.043
<b>Mass Dry Soil (g)</b>	8.417	10.246	9.783
<b>Moisture Content (%)</b>	34.014	32.842	31.105



**Plastic Limit**

Trial #	1	2	3	4	5
<b>Mass Tare (g)</b>	13.734	14.029			
<b>Mass Wet Soil + Tare (g)</b>	22.165	21.199			
<b>Mass Dry Soil + Tare (g)</b>	21.045	20.279			
<b>Mass Water (g)</b>	1.120	0.920			
<b>Mass Dry Soil (g)</b>	7.311	6.250			
<b>Moisture Content (%)</b>	15.319	14.720			

Note: Additional information recorded/measured for this test is available upon request.

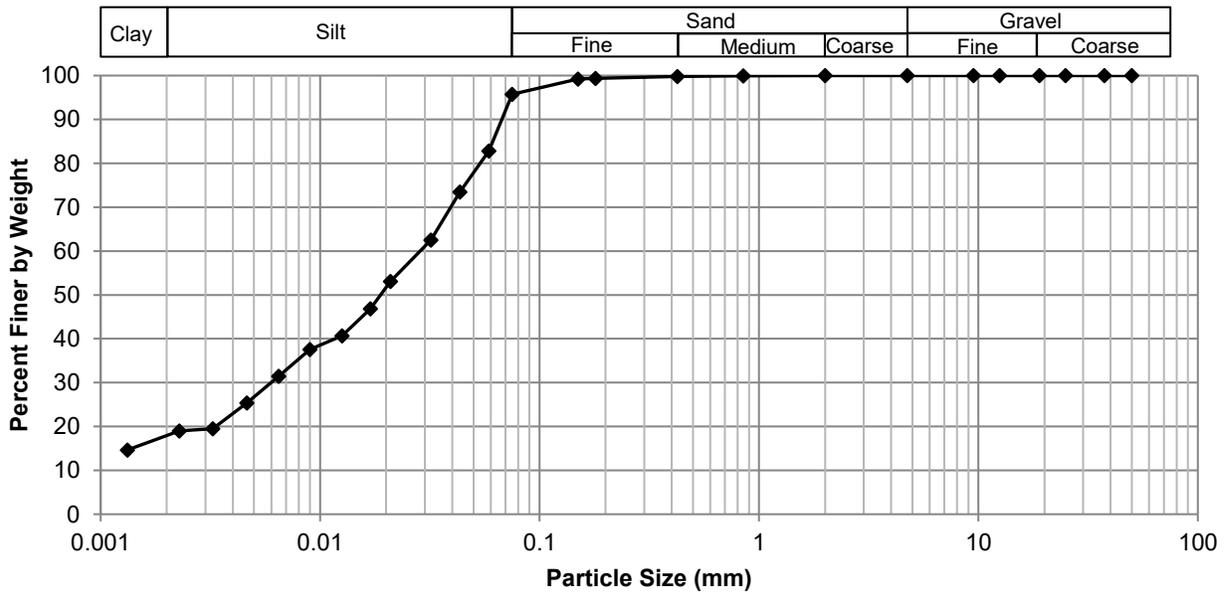
**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)



**Test Hole** TH26-02  
**Sample #** G31  
**Depth (m)** 1.1 - 1.2  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Gravel</b>	0.0%
<b>Sand</b>	4.3%
<b>Silt</b>	45.8%
<b>Clay</b>	49.9%

**Particle Size Distribution Curve**



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	95.72
37.5	100.00	2.00	100.00	0.0588	82.82
25.0	100.00	0.850	99.91	0.0433	73.44
19.0	100.00	0.425	99.82	0.0320	62.50
12.5	100.00	0.180	99.38	0.0210	53.12
9.50	100.00	0.150	99.22	0.0169	46.86
4.75	100.00	0.075	95.72	0.0126	40.66
				0.0090	37.59
				0.0065	31.44
				0.0046	25.34
				0.0032	19.51
				0.0023	18.99
				0.0013	14.66

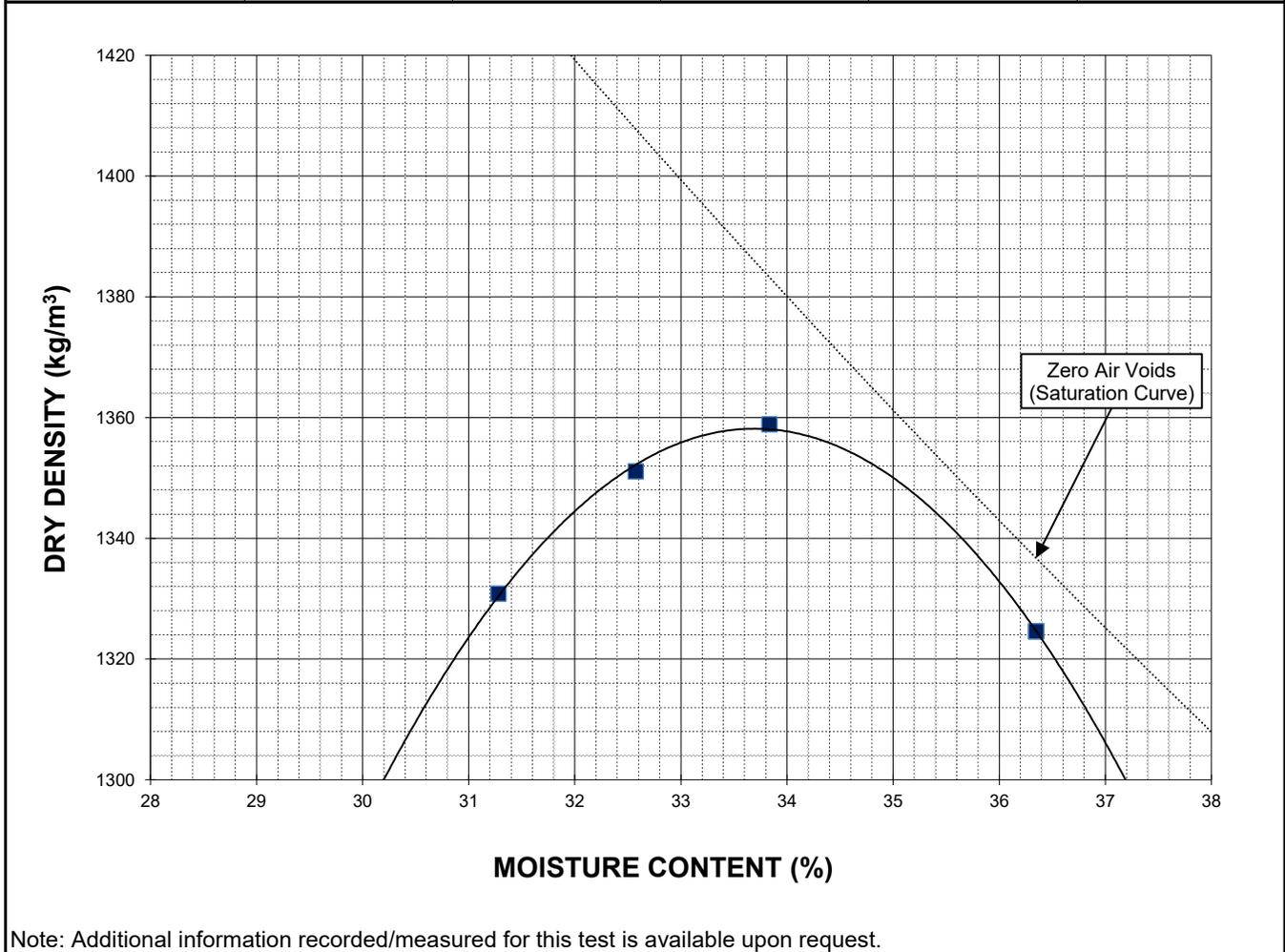
**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09



**Sample #** B38  
**Source** TH26-02 (Parkville Drive)  
**Material** Clay  
**Sample Date** 14-Jan-26  
**Test Date** 28-Jan-26  
**Technician** A. Dustmamatov

<b>Maximum Dry Density (kg/m<sup>3</sup>)</b>	1358
<b>Optimum Moisture (%)</b>	33.7

Trial Number	1	2	3	4
<b>Wet Density (kg/m<sup>3</sup>)</b>	1747	1791	1819	1806
<b>Dry Density (kg/m<sup>3</sup>)</b>	1331	1351	1359	1325
<b>Moisture Content (%)</b>	31.3	32.6	33.8	36.3





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## California Bearing Ratio Test Data Sheet

ASTM D1883-21

<b>Project No.</b>	1000-240-07	<b>Source</b>	TH26-02 (Parkville Drive)
<b>Client</b>	Stantec Consulting Ltd.	<b>Material</b>	Clay
<b>Project</b>	2026 Local Streets Package 26-R-09	<b>Sample Date</b>	14-Jan-26
<b>Sample #</b>	B38	<b>Test Date</b>	30-Jan-26
		<b>Technician</b>	A. Dustmamatov

### Proctor Results (ASTM D698)

Maximum Dry Density	1358 kg/m <sup>3</sup>
Optimum Moisture Content	33.7 %
Material Retained on 19 mm Sieve	0.0 %

### CBR Sample Compaction

Dry Density	1287 kg/m <sup>3</sup>
Initial Moisture Content	34.1 %
Relative Density	94.8 % SPMDD

### Soaking Results

Surcharge	4.54 kg
Swell	6.7 %
Moisture Content in top 25 mm	53.9 %
Immersion Period	96 h

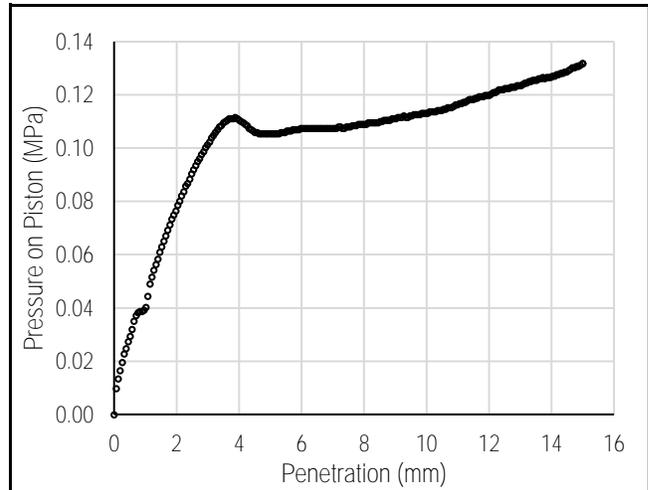
### CBR Results

CBR at 2.54 mm	1.3 %
CBR at 5.08 mm	1.0 %
Zero Correction	0 mm

**Test Data**

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.04	0.04
1.27	0.05	0.05
1.91	0.08	0.08
2.54	0.09	0.09
3.18	0.11	0.11
3.81	0.11	0.11
4.45	0.11	0.11
5.08	0.11	0.11
7.62	0.11	0.11
10.16	0.11	0.11
12.70	0.12	0.12

**Load/Penetration Curve**



**Comments:**



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**Atterberg Limits**  
**ASTM D4318-17e1**

**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)

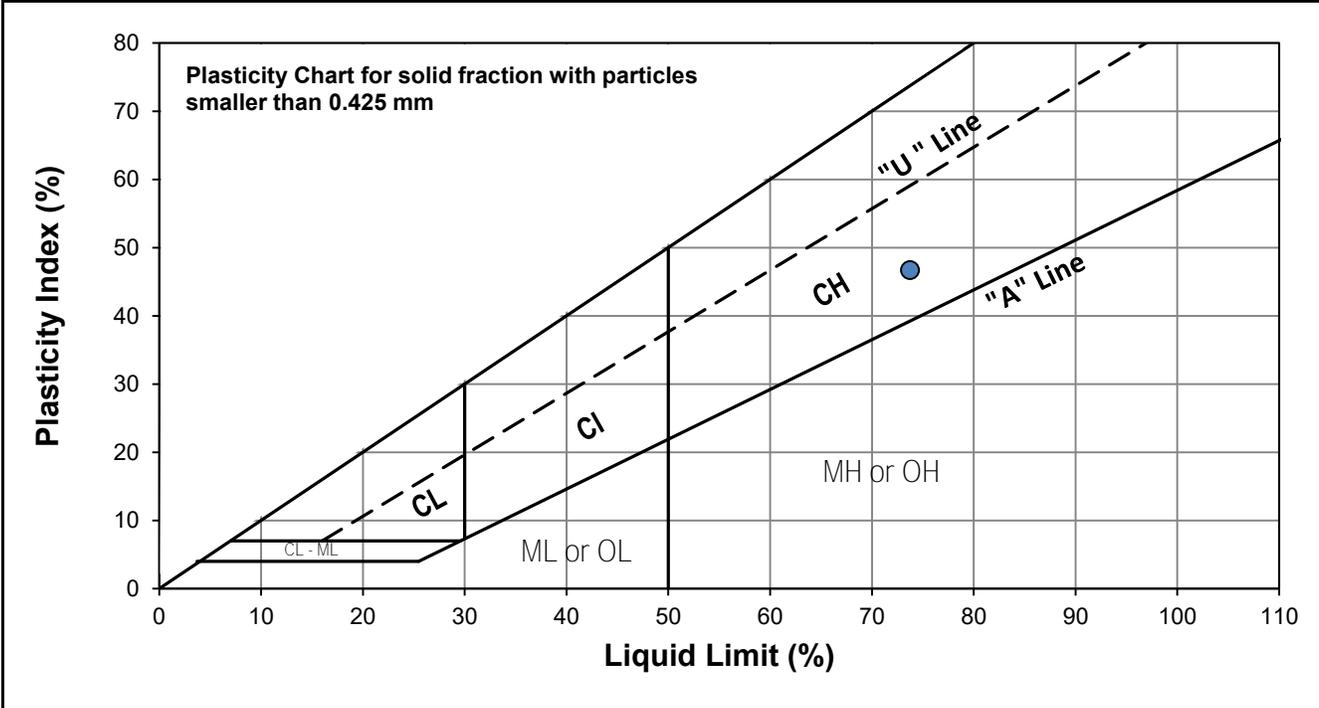


**Test Hole** TH26-03  
**Sample #** G40  
**Depth (m)** 0.9 - 1.1  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Liquid Limit</b>	74
<b>Plastic Limit</b>	27
<b>Plasticity Index</b>	47

**Liquid Limit**

Trial #	1	2	3
<b>Number of Blows (N)</b>	17	28	35
<b>Mass Tare (g)</b>	14.019	13.949	14.159
<b>Mass Wet Soil + Tare (g)</b>	21.610	23.219	22.398
<b>Mass Dry Soil + Tare (g)</b>	18.369	19.286	18.925
<b>Mass Water (g)</b>	3.241	3.933	3.473
<b>Mass Dry Soil (g)</b>	4.350	5.337	4.766
<b>Moisture Content (%)</b>	74.506	73.693	72.870



**Plastic Limit**

Trial #	1	2	3	4	5
<b>Mass Tare (g)</b>	13.642	13.565			
<b>Mass Wet Soil + Tare (g)</b>	21.640	22.329			
<b>Mass Dry Soil + Tare (g)</b>	19.960	20.439			
<b>Mass Water (g)</b>	1.680	1.890			
<b>Mass Dry Soil (g)</b>	6.318	6.874			
<b>Moisture Content (%)</b>	26.591	27.495			

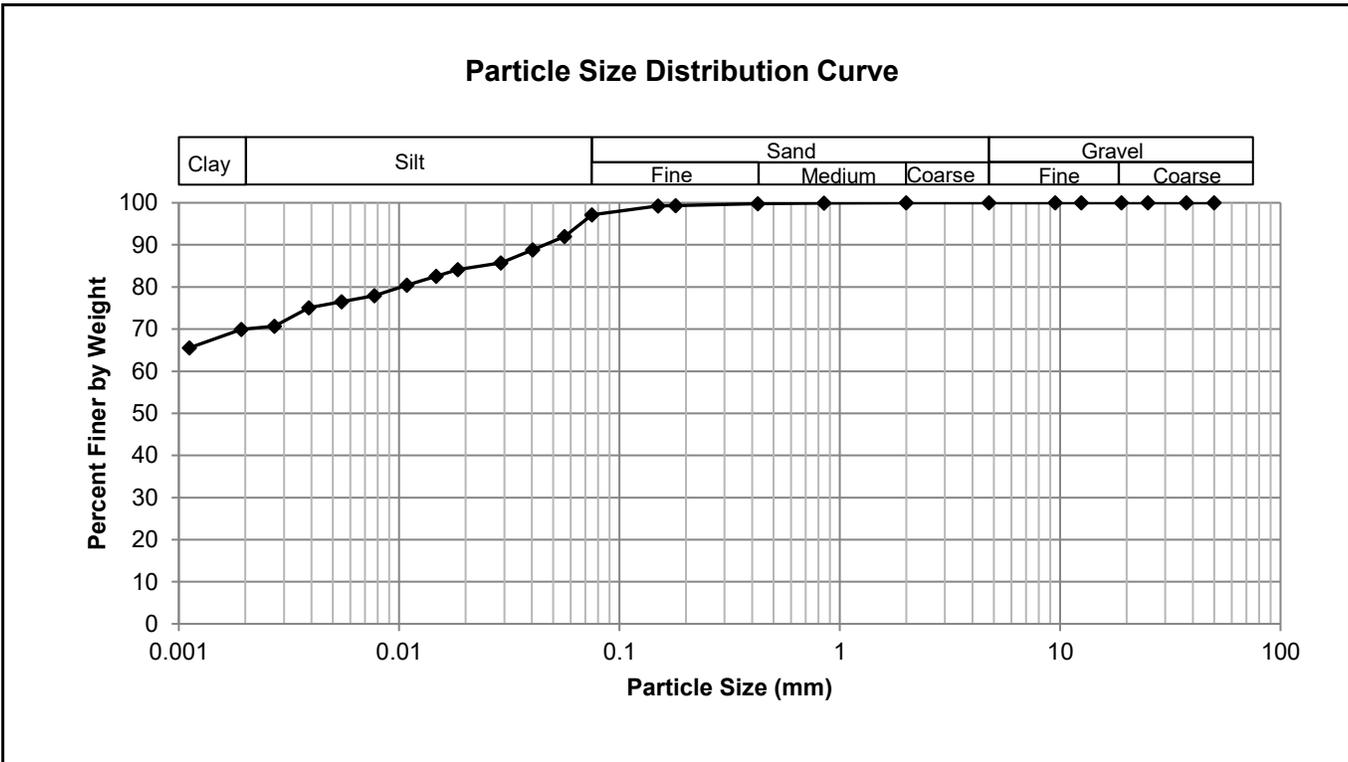
Note: Additional information recorded/measured for this test is available upon request.

**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)



**Test Hole** TH26-03  
**Sample #** G40  
**Depth (m)** 1.1 - 1.2  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Gravel</b>	0.0%
<b>Sand</b>	2.9%
<b>Silt</b>	27.1%
<b>Clay</b>	70.0%



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	97.12
37.5	100.00	2.00	100.00	0.0563	91.97
25.0	100.00	0.850	99.96	0.0404	88.84
19.0	100.00	0.425	99.81	0.0290	85.71
12.5	100.00	0.180	99.36	0.0185	84.15
9.50	100.00	0.150	99.25	0.0147	82.59
4.75	100.00	0.075	97.12	0.0108	80.41
				0.0077	77.98
				0.0055	76.50
				0.0039	75.06
				0.0027	70.70
				0.0019	69.97
				0.0011	65.57

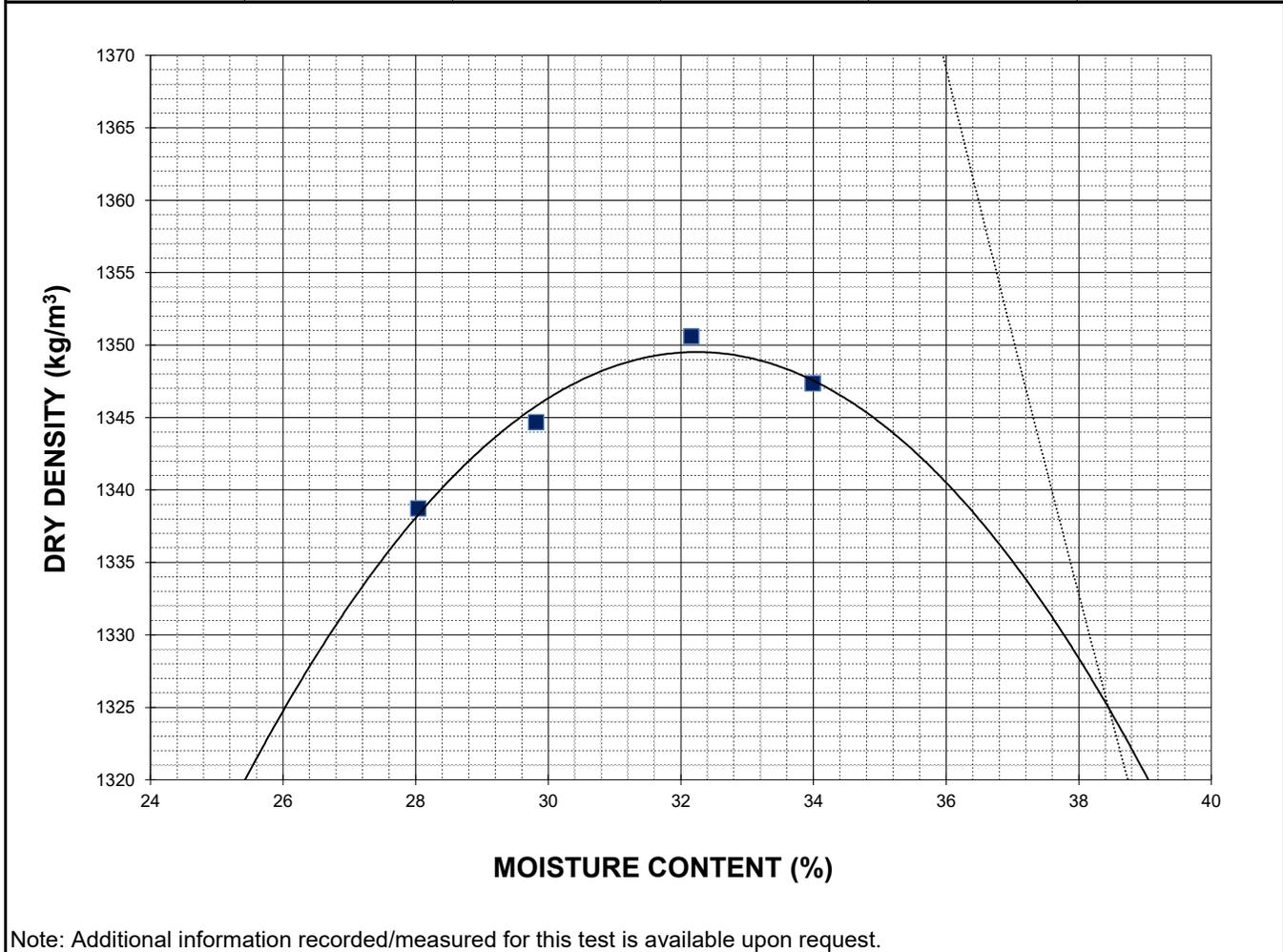
**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09



**Sample #** B45  
**Source** TH26-03 (Parkville Drive)  
**Material** Clay  
**Sample Date** 14-Jan-26  
**Test Date** 28-Jan-26  
**Technician** K. Desikan

<b>Maximum Dry Density (kg/m<sup>3</sup>)</b>	1350
<b>Optimum Moisture (%)</b>	32.2

Trial Number	1	2	3	4
<b>Wet Density (kg/m<sup>3</sup>)</b>	1714	1746	1785	1805
<b>Dry Density (kg/m<sup>3</sup>)</b>	1339	1345	1351	1347
<b>Moisture Content (%)</b>	28.0	29.8	32.2	34.0





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## California Bearing Ratio Test Data Sheet

ASTM D1883-21

<b>Project No.</b>	1000-240-07	<b>Source</b>	TH26-03 (Parkville Drive)
<b>Client</b>	Stantec Consulting Ltd.	<b>Material</b>	Clay
<b>Project</b>	2026 Local Streets Package 26-R-09	<b>Sample Date</b>	14-Jan-26
<b>Sample #</b>	B45	<b>Test Date</b>	02-Feb-26
		<b>Technician</b>	A. Dustmamatov

### Proctor Results (ASTM D698)

Maximum Dry Density	1350 kg/m <sup>3</sup>
Optimum Moisture Content	32.2 %
Material Retained on 19 mm Sieve	0.0 %

### CBR Sample Compaction

Dry Density	1292 kg/m <sup>3</sup>
Initial Moisture Content	31.8 %
Relative Density	95.7 % SPMDD

### Soaking Results

Surcharge	4.54 kg
Swell	8.5 %
Moisture Content in top 25 mm	57.6 %
Immersion Period	96 h

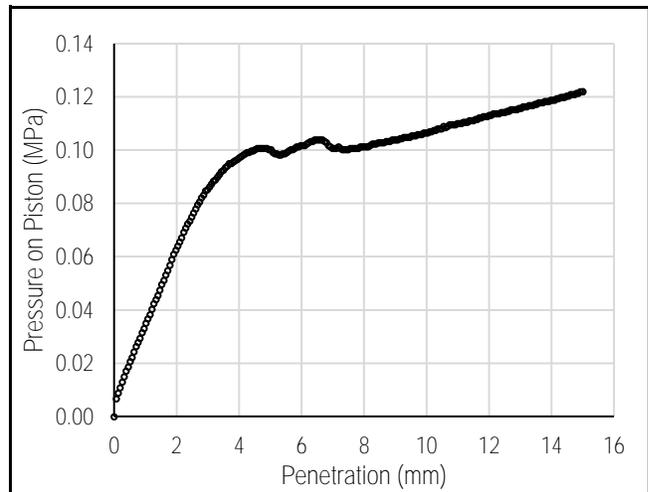
### CBR Results

CBR at 2.54 mm	1.1 %
CBR at 5.08 mm	1.0 %
Zero Correction	0 mm

**Test Data**

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.02	0.02
1.27	0.04	0.04
1.91	0.06	0.06
2.54	0.08	0.08
3.18	0.09	0.09
3.81	0.10	0.10
4.45	0.10	0.10
5.08	0.10	0.10
7.62	0.10	0.10
10.16	0.11	0.11
12.70	0.12	0.12

**Load/Penetration Curve**



**Comments:**



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**Atterberg Limits**  
**ASTM D4318-17e1**

**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)

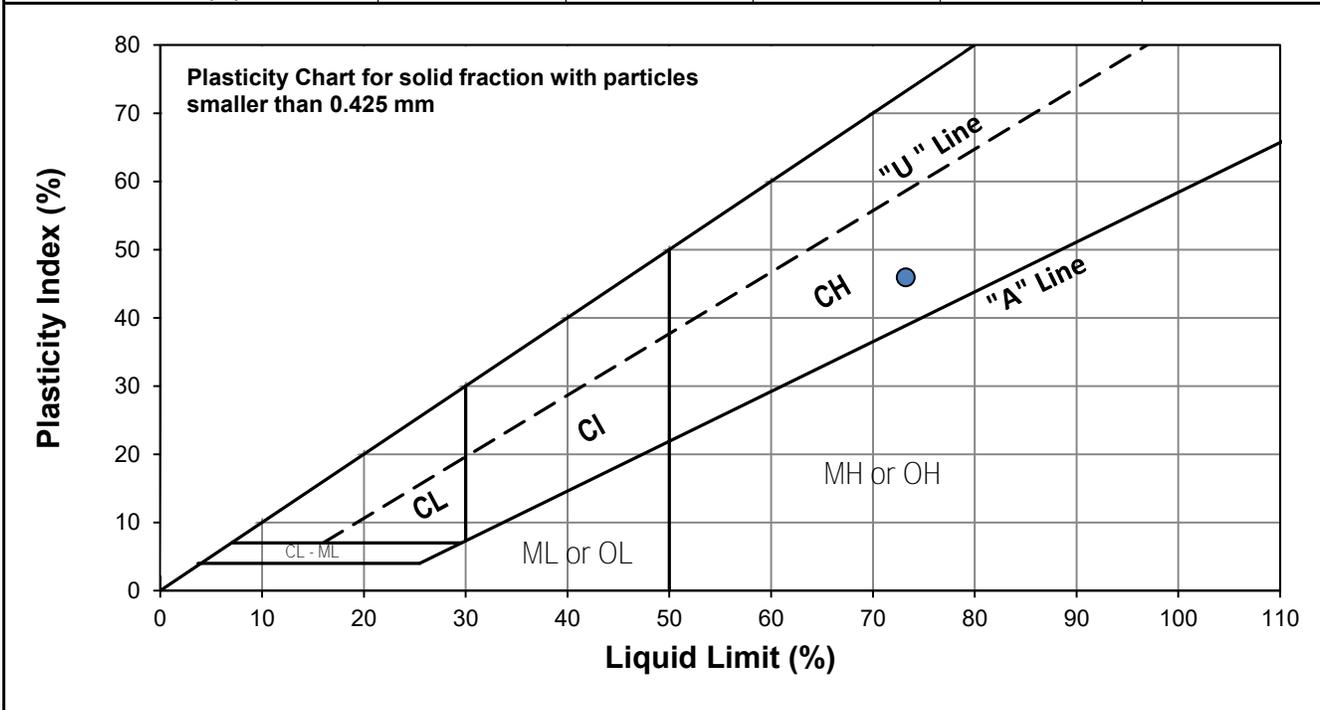


**Test Hole** TH26-04  
**Sample #** G24  
**Depth (m)** 1.1 - 1.2  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Liquid Limit</b>	73
<b>Plastic Limit</b>	27
<b>Plasticity Index</b>	46

**Liquid Limit**

Trial #	1	2	3
<b>Number of Blows (N)</b>	19	21	33
<b>Mass Tare (g)</b>	13.856	14.172	13.964
<b>Mass Wet Soil + Tare (g)</b>	24.230	24.471	25.534
<b>Mass Dry Soil + Tare (g)</b>	19.788	20.083	20.706
<b>Mass Water (g)</b>	4.442	4.388	4.828
<b>Mass Dry Soil (g)</b>	5.932	5.911	6.742
<b>Moisture Content (%)</b>	74.882	74.234	71.611



**Plastic Limit**

Trial #	1	2	3	4	5
<b>Mass Tare (g)</b>	13.796	13.738			
<b>Mass Wet Soil + Tare (g)</b>	21.902	21.670			
<b>Mass Dry Soil + Tare (g)</b>	20.183	19.946			
<b>Mass Water (g)</b>	1.719	1.724			
<b>Mass Dry Soil (g)</b>	6.387	6.208			
<b>Moisture Content (%)</b>	26.914	27.771			

Note: Additional information recorded/measured for this test is available upon request.

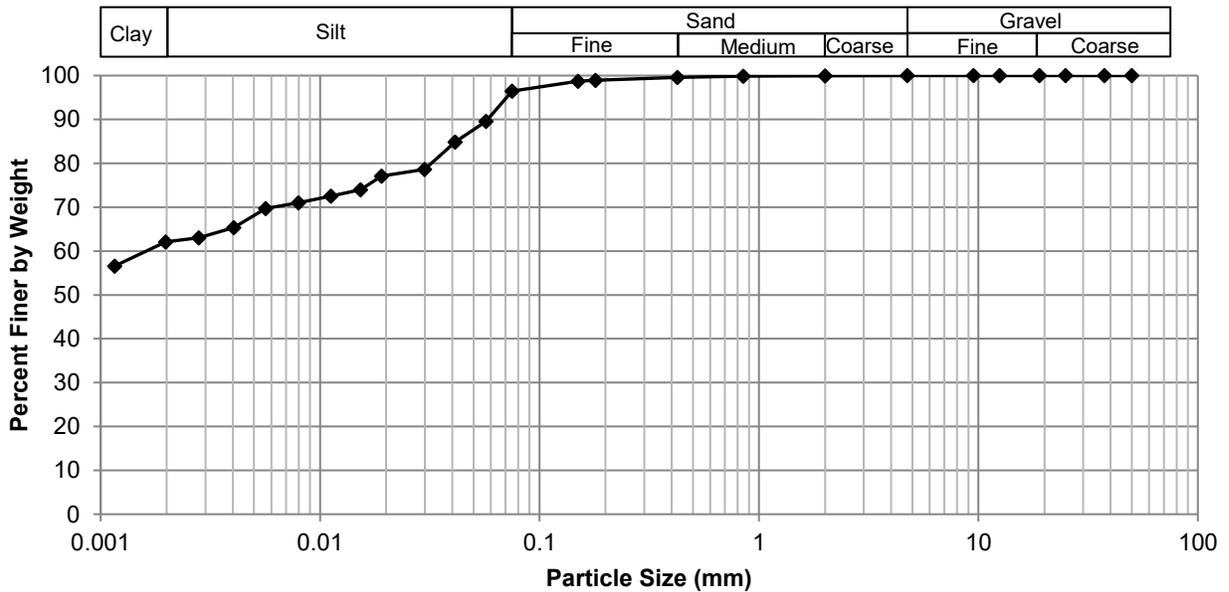
**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09 (Parkville Drive)



**Test Hole** TH26-04  
**Sample #** G24  
**Depth (m)** 1.1 - 1.2  
**Sample Date** 14-Jan-26  
**Test Date** 02-Feb-26  
**Technician** Z.Dypiangco

<b>Gravel</b>	0.0%
<b>Sand</b>	3.6%
<b>Silt</b>	34.3%
<b>Clay</b>	62.1%

**Particle Size Distribution Curve**



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	96.44
37.5	100.00	2.00	99.97	0.0570	89.58
25.0	100.00	0.850	99.85	0.0412	84.89
19.0	100.00	0.425	99.61	0.0300	78.64
12.5	100.00	0.180	98.92	0.0191	77.07
9.50	100.00	0.150	98.68	0.0153	73.95
4.75	100.00	0.075	96.44	0.0112	72.49
				0.0080	71.04
				0.0057	69.69
				0.0041	65.32
				0.0028	63.06
				0.0020	62.08
				0.0012	56.58

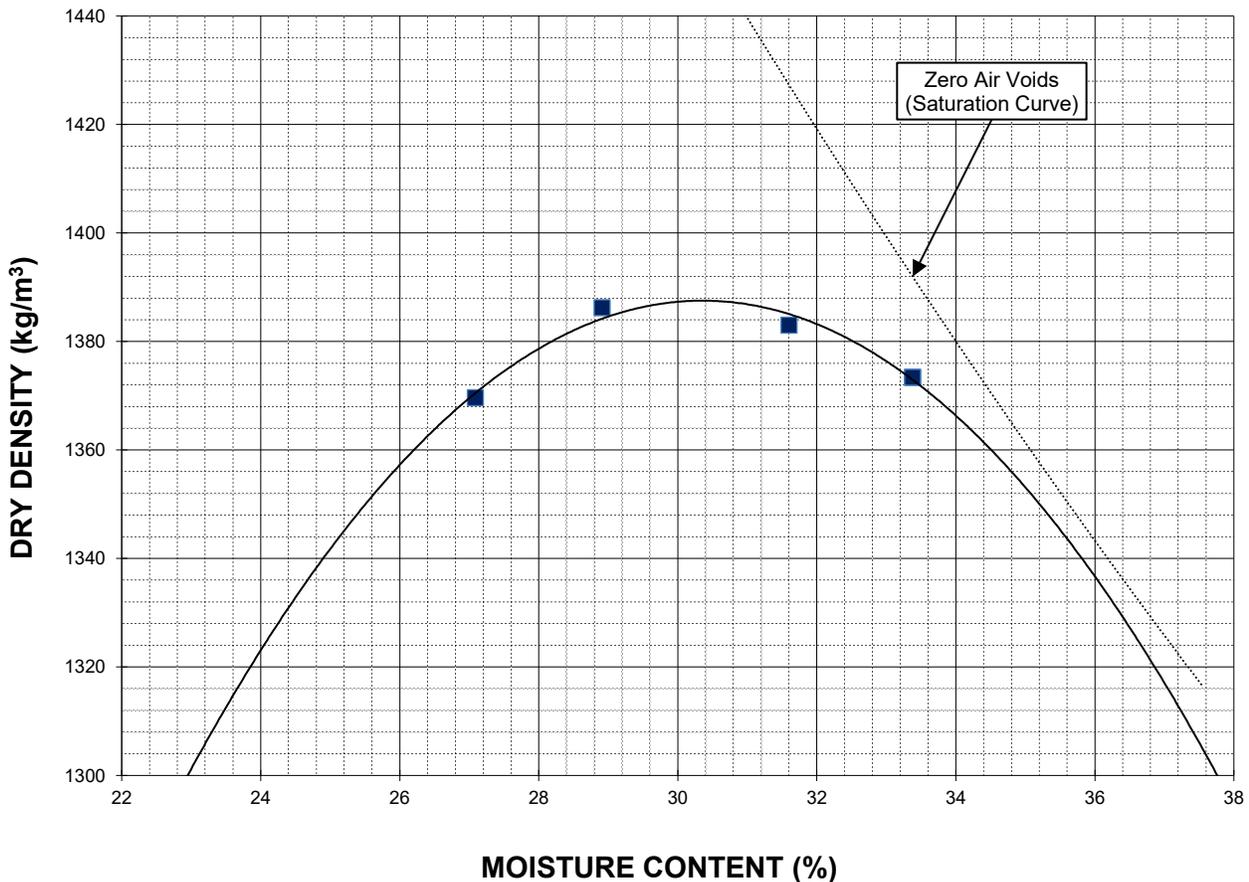
**Project No.** 1000-240-07  
**Client** Stantec Consulting Ltd.  
**Project** 2026 Local Streets Package 26-R-09



**Sample #** B29  
**Source** TH26-04 (Parkville Drive)  
**Material** Clay  
**Sample Date** 14-Jan-26  
**Test Date** 28-Jan-26  
**Technician** K. Desikan

<b>Maximum Dry Density (kg/m<sup>3</sup>)</b>	1388
<b>Optimum Moisture (%)</b>	30.4

Trial Number	1	2	3	4
<b>Wet Density (kg/m<sup>3</sup>)</b>	1741	1787	1820	1832
<b>Dry Density (kg/m<sup>3</sup>)</b>	1370	1386	1383	1373
<b>Moisture Content (%)</b>	27.1	28.9	31.6	33.4



Note: Additional information recorded/measured for this test is available upon request.



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## California Bearing Ratio Test Data Sheet

ASTM D1883-21

<b>Project No.</b>	1000-240-07	<b>Source</b>	TH26-04 (Parkville Drive)
<b>Client</b>	Stantec Consulting Ltd.	<b>Material</b>	Clay
<b>Project</b>	2026 Local Streets Package 26-R-09	<b>Sample Date</b>	14-Jan-26
<b>Sample #</b>	B29	<b>Test Date</b>	30-Jan-26
		<b>Technician</b>	A. Dustmamatov

### Proctor Results (ASTM D698)

Maximum Dry Density	1388 kg/m <sup>3</sup>
Optimum Moisture Content	30.4 %
Material Retained on 19 mm Sieve	0.0 %

### CBR Sample Compaction

Dry Density	1318 kg/m <sup>3</sup>
Initial Moisture Content	30.4 %
Relative Density	94.9 % SPMDD

### Soaking Results

Surcharge	4.54 kg
Swell	8.2 %
Moisture Content in top 25 mm	48.7 %
Immersion Period	96 h

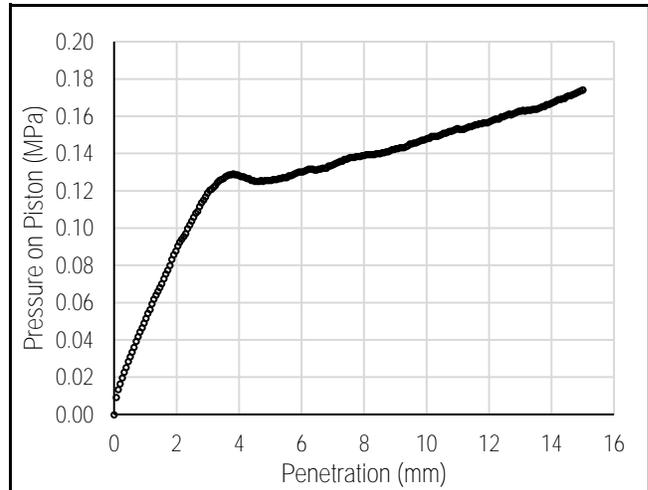
### CBR Results

CBR at 2.54 mm	1.5 %
CBR at 5.08 mm	1.2 %
Zero Correction	0 mm

**Test Data**

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.04	0.04
1.27	0.06	0.06
1.91	0.09	0.09
2.54	0.11	0.11
3.18	0.12	0.12
3.81	0.13	0.13
4.45	0.13	0.13
5.08	0.13	0.13
7.62	0.14	0.14
10.16	0.15	0.15
12.70	0.16	0.16

**Load/Penetration Curve**



**Comments:**



Photo 1: Pavement Core Sample at TH26-02



Photo 2: Pavement Core Sample at TH26-03



Photo 3: Pavement Core Sample at TH26-04



2026 Local Streets Package 26-R-09  
Parkville Dr - Pulberry Dr to Dunkirk Dr

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-101	UTM : 5522141 m N, 635557 m E; Located at #31 Parkville Dr, Westbound curb lane, 1.9 m South of North curb.	Asphalt	75	Concrete	140



Photo 1: Pavement Core Sample at CP26-101

**Appendix D**  
**Summary Table and Photographs of Pavement Core Samples**  
**Beliveau Road – Dakota Street to Kearney Street**

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**2026 Local Streets Package 26-R-09  
Beliveau Rd - Dakota St to Kearney St**

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-06	UTM : 5522054 m N, 635875 m E; Located 30 m East of Dakota St, Westbound curb lane, 1.1 m South of North curb.	Asphalt	60	Concrete	200
PC26-07	UTM : 5522080 m N, 635936 m E; Located 20 m West of crosswalk, Eastbound curb lane, 1.2 m North of South curb.	Asphalt	30	Concrete	200
PC26-08	UTM : 5522130 m N, 636018 m E; Located North West corner of tennis court, Eastbound curb lane, 1.6 m North of South curb.	Asphalt	50	Concrete	175
PC26-09	UTM : 5522159 m N, 636076 m E; Located at #125 Beliveau Rd, Eastbound curb lane, 2.3 m North of South curb.	Asphalt	65	Concrete	210
PC26-10	UTM : 5522206 m N, 636147 m E; Located at #147 Beliveau Rd, Westbound curb lane, 1.7 m South of North curb.	Asphalt	70	Concrete	210
PC26-11	UTM : 5522338 m N, 636203 m E; Located at #100 Beliveau Rd, Westbound curb lane, 2.0 m South of North curb.	Asphalt	100	Concrete	200
PC26-12	UTM : 5522271 m N, 636273 m E; Located at #187B Beliveau Rd, Eastbound curb lane, 2.2 m North of South curb.	Asphalt	55	Concrete	210
PC26-13	UTM : 5522308 m N, 636340 m E; Located at #211 Beliveau Rd, Eastbound curb lane, 2.6 m North of South curb.	Asphalt	85	Concrete	190
PC26-14	UTM : 5522343 m N, 636400 m E; Located at #227 Beliveau Rd, Eastbound curb lane, 1.7 m North of South curb.	Asphalt	25	Concrete	225

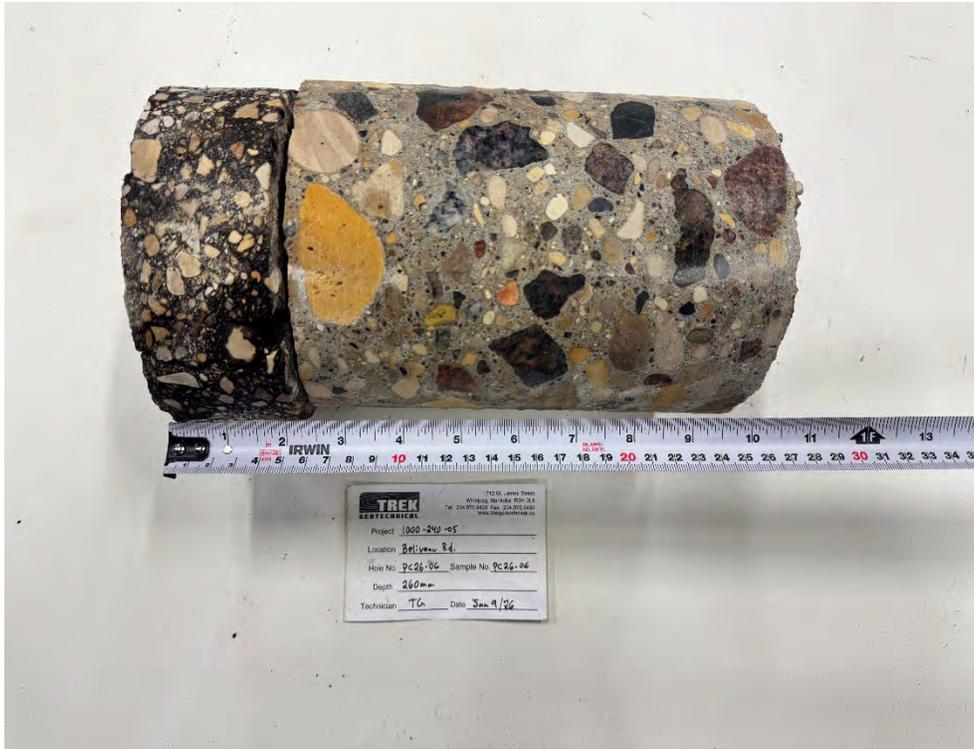


Photo 1: Pavement Core Sample at PC26-06



Photo 2: Pavement Core Sample at PC26-07



Photo 3: Pavement Core Sample at PC26-08



Photo 4: Pavement Core Sample at PC26-09



Photo 5: Pavement Core Sample at PC26-10



Photo 6: Pavement Core Sample at PC26-11



Photo 7: Pavement Core Sample at PC26-12



Photo 8: Pavement Core Sample at PC26-13



Photo 9: Pavement Core Sample at PC26-14

**Appendix E**  
**Summary Table and Photographs of Pavement Core Samples**

**Berrydale Avenue – St. Anne’s Road to East Limit**

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**2026 Local Streets Package 26-R-09**  
**Berrydale Ave - St Anne's Rd to East Limit**

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-18	UTM : 5523362 m N, 636780 m E; Located 20 m East of St. Anne's Rd, Westbound lane, 4.5 m South of North curb.	Asphalt	-	Concrete	130
PC26-19	UTM : 5523387 m N, 636834 m E; Located at #175 Berrydale Ave, Westbound lane, 2.1 m South of North curb.	Asphalt	-	Concrete	145
PC26-20	UTM : 5523419 m N, 636891 m E; Located at the side of #96 Clayton Ave, Westbound lane, 2.5 m South of North curb.	Asphalt	55	Concrete	155
PC26-21	UTM : 5523448 m N, 636943 m E; Located near the garage of #97 Clayton Ave, Westbound lane, 1.8 m South of North curb.	Asphalt	-	Concrete	160
PC26-22	UTM : 5523473 m N, 636997 m E; Located at #199 Berrydale Ave, Westbound lane, 2.4 m South of North curb.	Asphalt	-	Concrete	150



Photo 1: Pavement Core Sample at PC26-18



Photo 2: Pavement Core Sample at PC26-19



Photo 3: Pavement Core Sample at PC26-20



Photo 4: Pavement Core Sample at PC26-21



Photo 5: Pavement Core Sample at PC26-22

**Appendix J**  
**Summary Table and Photographs of Pavement Core Samples**  
**Beaverhill Boulevard – Shamrock Drive to Fermor Avenue**

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**2026 Local Streets Package 26-R-09**  
**Beaverhill Blvd - Shamrock Dr to Fermor Ave**

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-29	UTM : 5523306 m N, 639174 m E; Located 13 m North of Shamrock Dr, Northbound curb lane, 2.0 m West of East curb.	Asphalt	25	Concrete	200
PC26-30	UTM : 5523371 m N, 639164 m E; Located 20 m North of Courtwood Pl, Northbound curb lane, 2.1 m West of East curb.	Asphalt	50	Concrete	190
PC26-31	UTM : 5523458 m N, 639132 m E; Located at the side of #96 Lake Lawn Rd, Southbound curb lane, 1.3 m East of West curb.	Asphalt	55	Concrete	195
PC26-32	UTM : 5523513 m N, 639115 m E; Located at the side of #2 Seabrook Cove, Southbound curb lane, 1.4 m East of West curb.	Asphalt	40	Concrete	160
PC26-33	UTM : 5523598 m N, 639110 m E; Located at #879 Beaverhill Blvd, Northbound curb lane, 2.2 m West of East curb.	Asphalt	50	Concrete	210
PC26-34	UTM : 5523668 m N, 639119 m E; Located at the side of #242 Edgewater Dr, Southbound curb lane, 1.6 m East of West curb.	Asphalt	40	Concrete	180
PC26-35	UTM : 5523750 m N, 639162 m E; Located at #911 Beaverhill Blvd, Northbound curb lane, 2.7 m West of East curb.	Asphalt	65	Concrete	190
PC26-36	UTM : 5523826 m N, 639194 m E; Located at the side of #2 Park Terrace Dr, Southbound curb lane, 1.8 m East of West curb.	Asphalt	60	Concrete	220
PC26-37	UTM : 5523943 m N, 639201 m E; Located at the side #86 Rockcliffe Rd, Northbound curb lane, 2.6 m West of East curb.	Asphalt	60	Concrete	180
PC26-39	UTM : 5523978 m N, 639178 m E; Located at the side of #996 Beaverhill Blvd, Southbound curb lane, 1.2 m East of West curb.	Asphalt	35	Concrete	185
PC26-40	UTM : 5524037 m N, 639145 m E; Located at the side #1 Everglade Pl, Northbound curb lane, 2.8 m West of East curb.	Asphalt	60	Concrete	195
PC26-41	UTM : 5524061 m N, 639124 m E; Located at the side #3 Everglade Pl, Northbound curb lane, 2.0 m West of East curb.	Asphalt	30	Concrete	200
PC26-42	UTM : 5524123 m N, 639082 m E; Located at #911 Beaverhill Blvd, Northbound median lane, 1.7 m East of West curb.	Asphalt	40	Concrete	220
PC26-43	UTM : 5524236 m N, 639073 m E; Located 20 m North of Vermillion Rd, Northbound median lane, 2.1 m East of West curb.	Asphalt	50	Concrete	190



**2026 Local Streets Package 26-R-09**  
**Beaverhill Blvd - Shamrock Dr to Fermor Ave**

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-44	UTM : 5524236 m N, 639075 m E; Located at #1035 Beaverhill Blvd, Northbound curb lane, 4.0 m West of East curb.	Asphalt	50	Concrete	195
PC26-45	UTM : 5524309 m N, 639073 m E; Located at #1037 Beaverhill Blvd, Northbound curb lane, 2.5 m West of East curb.	Asphalt	55	Concrete	195
PC26-46	UTM : 5524353 m N, 639066 m E; Located at #1041 Beaverhill Blvd, Northbound median lane, 2.4 m East of West curb.	Asphalt	45	Concrete	180
PC26-47	UTM : 5524386 m N, 639054 m E; Located 30 m South of Fermor Ave, Southbound curb lane, 2.8 m East of West curb.	Asphalt	135	Concrete	190
PC26-48	UTM : 5524323 m N, 639057 m E; Located at #1040 Beaverhill Blvd, Southbound curb lane, 2.4 m East of West curb.	Asphalt	65	Concrete	210
PC26-49	UTM : 5524250 m N, 639059 m E; Located at #1034 Beaverhill Blvd, Southbound curb lane, 2.8 m East of West curb.	Asphalt	45	Concrete	195
PC26-50	UTM : 5524202 m N, 639065 m E; Located 20 m North of Vermillion Rd, Southbound median lane, 2.6 m West of East curb.	Asphalt	40	Concrete	205
PC26-51	UTM : 5524127 m N, 639071 m E; Located at #991 Beaverhill Blvd, Southbound median lane, 1.6 m West of East curb.	Asphalt	55	Concrete	190
PC26-52	UTM : 5524017 m N, 639153 m E; Located at the side of #75 Park Terrace Pl, Southbound curb lane, 1.5 m East of West curb.	Asphalt	50	Concrete	190



Photo 1: Pavement Core Sample at PC26-29



Photo 2: Pavement Core Sample at PC26-30



Photo 3: Pavement Core Sample at PC26-31



Photo 4: Pavement Core Sample at PC26-32



Photo 5: Pavement Core Sample at PC26-33



Photo 6: Pavement Core Sample at PC26-34



Photo 7: Pavement Core Sample at PC26-35



Photo 8: Pavement Core Sample at PC26-36



Photo 9: Pavement Core Sample at PC26-37



Photo 10: Pavement Core Sample at PC26-38



Photo 11: Pavement Core Sample at PC26-39



Photo 12: Pavement Core Sample at PC26-40



Photo 13: Pavement Core Sample at PC26-41



Photo 14: Pavement Core Sample at PC26-42



Photo 15: Pavement Core Sample at PC26-43



Photo 16: Pavement Core Sample at PC26-44



Photo 17: Pavement Core Sample at PC26-45



Photo 18: Pavement Core Sample at PC26-46



Photo 19: Pavement Core Sample at PC26-47



Photo 20: Pavement Core Sample at PC26-48



Photo 21: Pavement Core Sample at PC26-49



Photo 22: Pavement Core Sample at PC26-50



Photo 23: Pavement Core Sample at PC26-51



Photo 24: Pavement Core Sample at PC26-52

**Appendix K**  
**Summary Table and Photographs of Pavement Core Samples**

**Beaverhill Boulevard – Frontage Road 956/940**

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2026 Local Streets Package 26-R-09  
Beaverhill Blvd - Frontage Rd 956/940

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-38	UTM : 5523943 m N, 639170 m E; Located at #944 Beaverhill Blvd, Eastbound curb lane, 3.0 m South of North curb.	Asphalt	70	Concrete	140



Photo 1: Pavement Core Sample at PC26-38