

APPENDIX A

GEOTECHNICAL REPORT



Stantec Consulting Ltd.

2026 Local Streets Package 26-R-09

Prepared for:

Ron Bruce, P.Eng.

Stantec Consulting Ltd.

500-311 Portage Ave

Winnipeg, MB

R3B 2B9

Project Number: 1000-240-07

Date: February 13, 2026



February 13, 2026

Our File No. 1000-240-07

Ron Bruce, P.Eng.
Stantec Consulting Ltd.
500-311 Portage Ave
Winnipeg, MB
R3B 2B9

RE: 2026 Local Streets Package 26-R-09

TREK Engineering Inc. is pleased to submit our Final Report for the geotechnical investigation for 2026 Local Streets Package 26-R-09 project.

Please contact the undersigned should you have any questions.

Sincerely,

TREK Engineering Inc.
Per:

A handwritten signature in blue ink, appearing to read "N. Ferreira", is written over the printed name below.

Nelson John Ferreira, Ph.D., P.Eng., FEC
Senior Geotechnical Engineer

Encl.

Revision History

Revision No.	Author	Issue Date	Description
0	KF	February 13, 2026	Final Report

Authorization Signatures

Prepared By: *Kate Franklin*
Kate Franklin M.Sc. (Geotechnical Engineering)
Technical Support Specialist



Reviewed By: _____
Nelson John Ferreira, Ph.D., P.Eng., FEC
Senior Geotechnical Engineer



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1.0 Introduction

This report summarizes the results of the road investigation completed for the City of Winnipeg Industrial Steets Renewals project. The project included drilling test holes and collecting pavement cores along several streets. The test hole information collected describes the pavement structure of the existing road as well as the soil stratigraphy beneath the pavement structure. The investigation was carried out following the City of Winnipeg RFP No. 751-2023 (Section E3 – Site Investigation Requirements).

2.0 Road Investigation

The investigation included coring of pavement at 79 locations on 11 different streets with drilling of test holes occurring at 6 locations along 2 streets. TH26-01 could not be drilled due to the presence of above and below ground utilities, but the pavement was cored. The investigation locations are shown on Figures 01 to 10 (attached), and the table below summarizes the investigation program per street.

Table 1: Road Investigation Program

2026 Local Streets Package 26-R-09	# of Locations	Investigation
Parkville Dr. – Pullberry Dr to Dunkirk Dr	4	3 Test Holes to 3.0m* 4 Cores
St. Vital Park Roads (North, Central, South)	23	3 Test Holes to 3.0m 23 Cores
Glen Ave – Dunkirk Dr to Killarney St	5	5 Cores
Beliveau Rd – Dakota St to Kearney St	9	9 Cores
Berrydale Ave – St. Anne’s Rd to East Limit	5	5 Cores
Sandralin Bay – Avalon Rd to North Limit	1	1 Core
English Pl – Metz St to Darwin St	4	4 Cores
Plumtree Pl – Meadwood Dr to South Limit	1	1 Core
Dells Cr – Wales Ave to Hazelwood Ave	3	3 Cores
Beaverhill Blvd – Shamrock Dr to Fermor Ave	23	23 Cores
Beaverhill Blvd – Frontage Rd 956/940	1	1 Core

*Original program included 4 Test Holes. One could not be drilled due to the presence of above and below ground utilities.

The road investigation was conducted between January 6, 2026 to January 28, 2026. The pavement structure (asphalt/concrete) was cored by Tyler Green and Arshpreet Bhuller of TREK Engineering Inc. (TREK) using a portable coring press equipped with a hollow 150 mm diameter diamond core drill bit. The test holes were drilled by Maple Leaf Drilling to a depth of approximately 3.0 m below road surface using a truck mounted drill rig equipped with 150 mm diameter solid stem augers. The sub-surface conditions were observed during drilling and visually classified by Kate Franklin of TREK. Other pertinent information, such as groundwater and drilling conditions, was also recorded during the drilling investigation. Disturbed (auger cuttings) samples and bulk samples retrieved during the sub-surface investigation were transported to TREK’s material testing laboratory for further testing. Pavement core samples were also retrieved and logged at TREK’s material testing laboratory.

Core and test hole logs noted on the summary tables and test hole locations are based on UTM coordinates obtained using a hand-held GPS, and their location relative to the nearest address or intersection, measured distance from the edge of pavement, or other permanent features.

The laboratory testing program consisted of moisture content determination on all samples, as well as Atterberg Limits, and grain size analysis (mechanical sieve and hydrometer methods) on select samples between 0.6 and 0.9 m below pavement as well as Standard Proctor and CBR testing. Information gathered for each street package is included in separate appendices (Appendices A to K). The information provided in the Appendices includes test hole logs, laboratory testing summary tables and results, photos of the concrete cores, and summary of pavement core compressive strength.

Six CBR’s were completed on bulk samples of the soil units present below the pavement. Tests were performed on clay and silt layers encountered within the prescribed sample depth for CBR testing and the results are shown in the table below.

Table 2: CBR Testing Summary

Soil Unit	Street	Depth (m)	SPMDD (kg/m ³)	Opt. Moisture (%)	Percent Proctor (%)	Moisture Content (%)	CBR Value at 2.54 mm	CBR Value at 5.08 mm
Clay	Parkville Dr: TH26-02	1.5-2.5	1358	33.7	94.8	34.1	1.3%	1.0%
Clay	Parkville Dr: TH26-03	0.6-2.5	1350	32.2	95.7	31.8	1.1%	1.0%
Clay	Parkville Dr: TH26-04	0.6-2.5	1388	30.4	94.9	30.4	1.5%	1.2%
Clay	St Vital Park: TH26-05	0.6-2.5	1440	26.5	95.0	27.4	1.9%	1.6%
Clay	St Vital Park: TH26-06	0.6-2.5	1390	30.2	95.5	30.1	2.5%	1.8%

Clay	St Vital Park: TH26-07	0.6-2.5	1418	28.8	95.5	28.5	2.8%	2.1%
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The test hole logs include a description of the soil units encountered during drilling and other pertinent information such as groundwater conditions and a summary of the laboratory testing results. The soils were classified in general accordance with the Unified Soil Classification System (USCS) and the AASHTO soil classification system (American Association of state highway and transportation officials). The AASHTO system classifies soils based on laboratory testing results from Atterberg Limits and grain size testing methods (hydrometer and mechanical sieve method). Where laboratory testing was not conducted, the AASHTO classification of the soils were interpreted based on a visual assessment as indicated with a (I) on the test hole logs and attached tables. For cohesive soils, the AASHTO system uses a combination of testing results to determine the Group Index of the soils and thus, were only determined where sufficient laboratory test data was available.

3.0 Closure

The information provided in this report is in accordance with current engineering principles and practices (Standard of Practice). The findings of this report were based on information provided (field investigation, laboratory testing, geometries). Soil conditions are natural deposits that can be highly variable across a site. If sub-surface conditions are different than the conditions previously encountered on-site or those presented here, we should be notified to adjust our findings if necessary.

All information provided in this report is subject to our standard terms and conditions for engineering services, a copy of which is provided to each of our clients with the original scope of work, or a mutually executed standard engineering services agreement. If these conditions are not attached, and you are not already in possession of such terms and conditions, contact our office and you will be promptly provided with a copy.

This report has been prepared by TREK Engineering Inc. (the Consultant) for the exclusive use of Stantec Consulting Ltd. (the Client) and their agents for the work product presented in the report. Any findings or recommendations provided in this report are not to be used or relied upon by any third parties, except as agreed to in writing by the Client and Consultant prior to use.

Figures

Z:\Projects\1000 Soils Lab\1000 Lab Projects\1000-240 Stantec\1000-240-07 2026 Local Streets Package 26-R-09\3 Survey and Dwg\3.4 CAD\3.4.3 Working Folder\Figs 01 to 10 2026-02-10 Local Streets Package 26-R-09 0_A 1000-240-07.dwg, 2026-02-10 10:42:43 AM



LEGEND:

PAVEMENT CORE (TREK, 2026)

NOTES:

1. AERIAL IMAGERY FROM GOOGLE EARTH (2024).

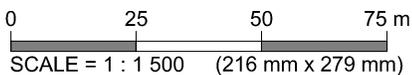


Figure 01
 Pavement Core Location Plan

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LEGEND:

◆ PAVEMENT CORE (TREK, 2026)

NOTES:

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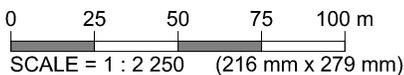


Figure 03
 Pavement Core Location Plan

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LEGEND:

PAVEMENT CORE (TREK, 2026)

NOTES:

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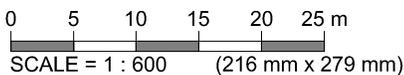
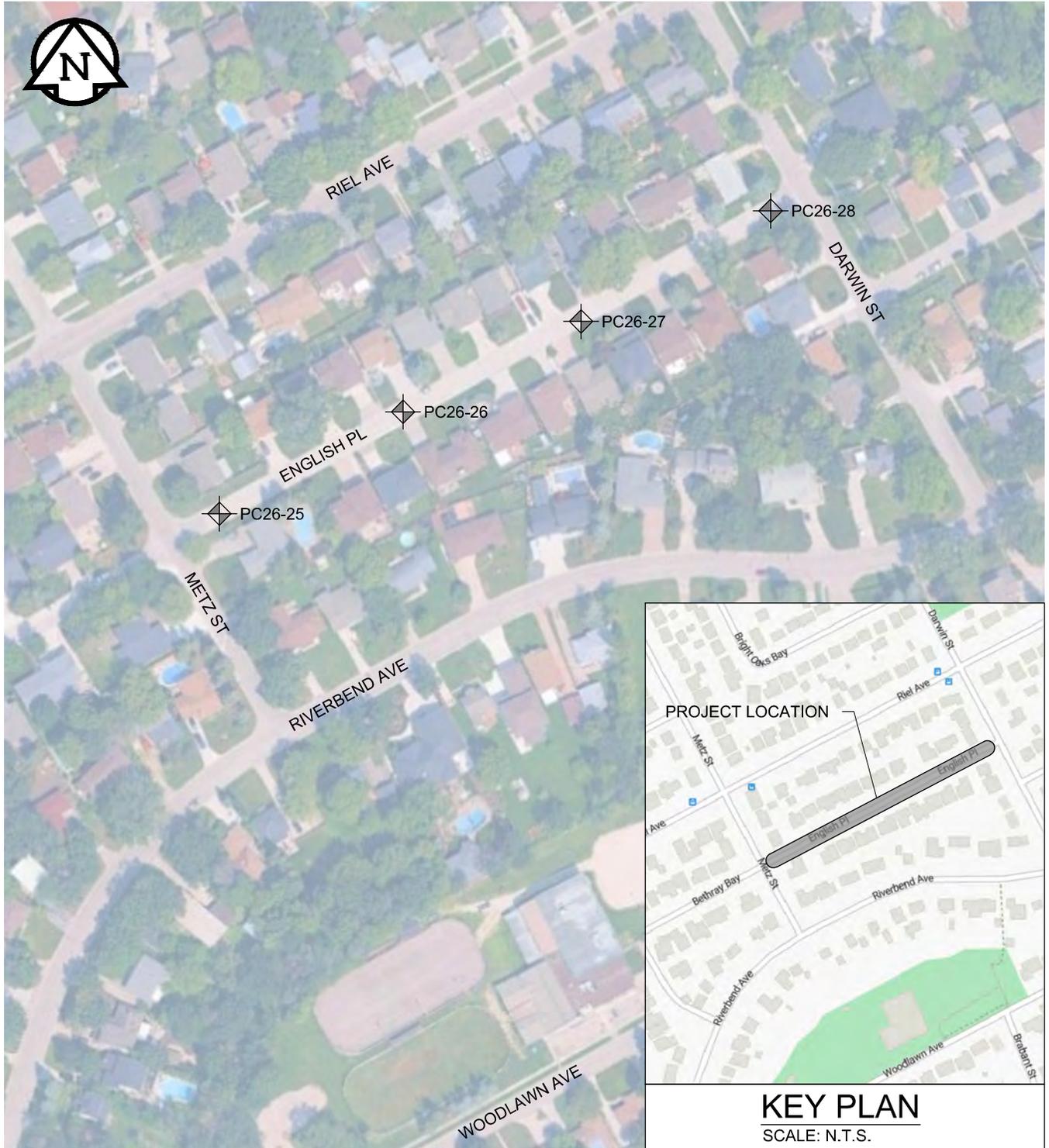


Figure 05
 Pavement Core Location Plan

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LEGEND:

◆ PAVEMENT CORE (TREK, 2026)

NOTES:

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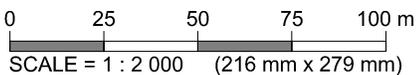
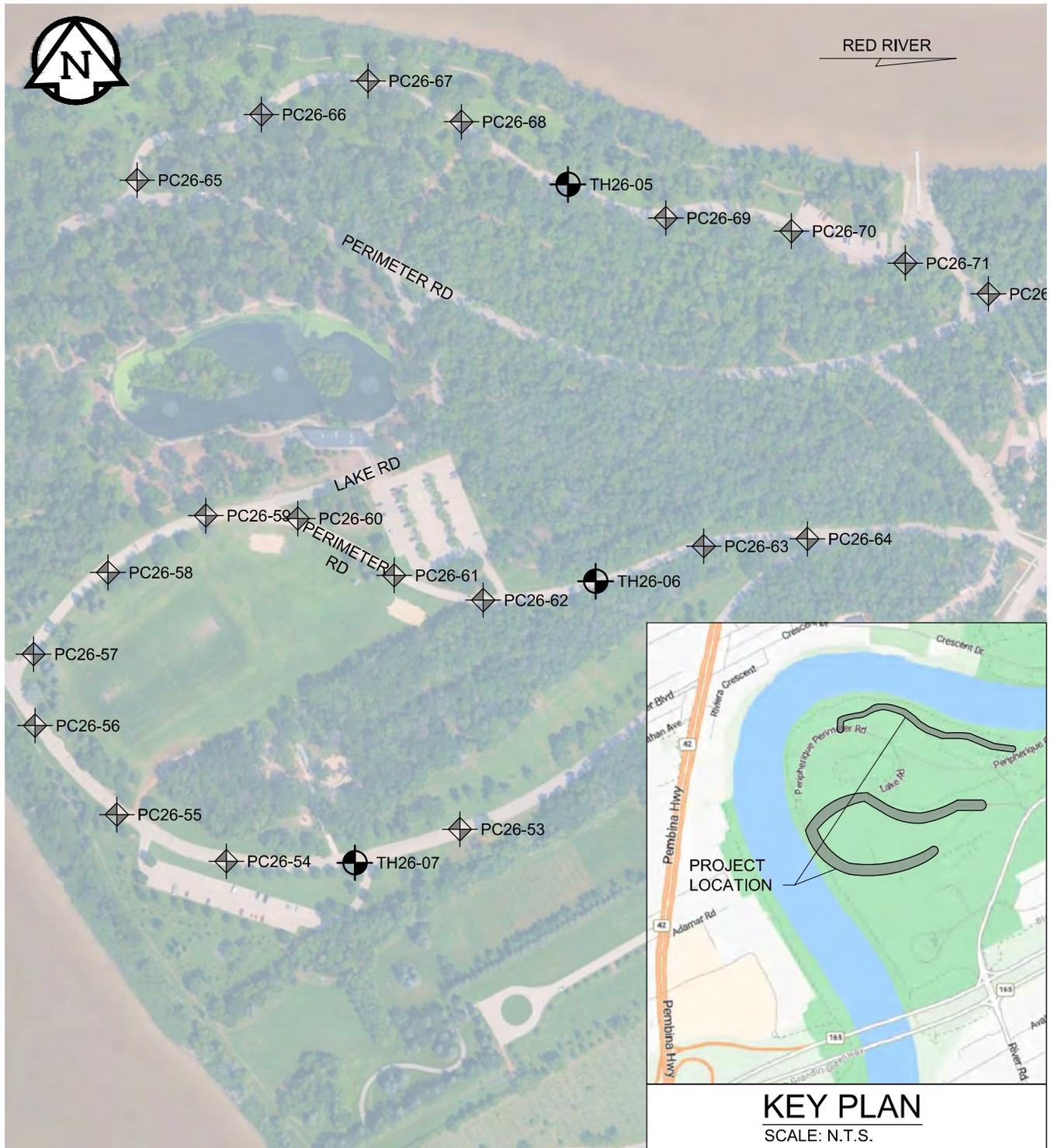


Figure 06
 Pavement Core Location Plan

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LEGEND:

- PAVEMENT CORE (TREK, 2026)
- TEST HOLE (TREK, 2026)

NOTES:

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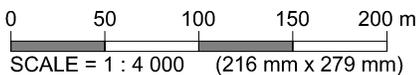


Figure 09
 Pavement Core and
 Test Hole Location Plan

Appendix B
Test Hole Logs, Summary Table, Lab Testing Results and
Photographs of Pavement Core Samples
St. Vital Park Roads – North, Central, South

Client: Stantec Consulting Ltd. **Project Number:** 1000-240-07
Project Name: 2026 Locals Streets Package 26-R-09 **Location:** UTM : 5521458 m N, 633416 m E, St Vital Park North Roadway
Contractor: Maple Leaf Drilling Ltd. **Ground Elevation:** Top of Pavement
Method: 150 mm Solid Stem Auger, B40 Truck Mount **Date Drilled:** January 14, 2026

Sample Type: Grab (G) Shelby Tube (T) Split Spoon (SS) / SPT Split Barrel (SB) / LPT Core (C)

Particle Size Legend: Fines Clay Silt Sand Gravel Cobbles Boulders

Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m ³)		Particle Size (%)		Undrained Shear Strength (kPa)								
					16	17	18	19	20	21	0	50	100	150	200	250	
0.0		ASPHALT (55mm thick)															
0.0		SAND (FILL) - trace gravel (<20mm diam.), some fines, brown, frozen - moist, compact when thawed, poorly graded, sub-rounded, "pit run", AASHTO: A-3 (I)															
0.0		CLAY - with silt, trace sand - grey - frozen to 1.4m, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (52)															
0.6			G01														
1.0			G02														
1.4			G03														
1.6			G04														
1.8			G05														
2.5			G06														
2.9		- silty, firm below 1.8m															

END OF TEST HOLE AT 3.0m IN CLAY.
 Notes:
 1) Seepage and sloughing not observed.
 2) Test hole open to 3.0m depth immediately after drilling.
 3) Water level at 2.9m immediately after drilling.
 4) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
 4) Bulk sample (B07 Clay) obtained from 0.6m to 2.0m.

SUB-SURFACE LOG LOGS 2026-01-14 LOCAL STREETS 1000-240-07 0 - A KF GPJ TREK.GDT 2/12/26

Client: Stantec Consulting Ltd. **Project Number:** 1000-240-07
Project Name: 2026 Locals Streets Package 26-R-09 **Location:** UTM : 5521894 m N, 635642 m E, St Vital Park Central Roadway
Contractor: Maple Leaf Drilling Ltd. **Ground Elevation:** Top of Pavement
Method: 150 mm Solid Stem Auger, B40 Truck Mount **Date Drilled:** January 14, 2026

Sample Type: Grab (G) Shelby Tube (T) Split Spoon (SS) / SPT Split Barrel (SB) / LPT Core (C)
Particle Size Legend: Fines Clay Silt Sand Gravel Cobbles Boulders

Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m ³)		Particle Size (%)		Undrained Shear Strength (kPa)	
					16	17	18	19	20	21
0.0		ASPHALT (75mm thick)								
0.0		SAND (FILL) - trace gravel (<20mm diam.), some fines, brown, frozen - moist, compact when thawed, poorly graded, sub-rounded, "pit run", AASHTO: A-3 (I)	G8							
0.0 - 0.5		CLAY - silty, trace sand - black - frozen to 1.4m, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (55)	G9							
0.5 - 1.0		- greyish brown below 0.9m	G10							
1.0 - 1.5			G11							△
1.5 - 2.0			G12							△
2.0 - 2.5		- stiff to very stiff below 1.8m	G13							△
2.5 - 3.0			G14							△

END OF TEST HOLE AT 3.0m IN CLAY.

Notes:

- 1) Seepage and sloughing not observed.
- 2) Test hole dry and open to 3.0m depth immediately after drilling.
- 3) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
- 4) Bulk sample (B15 Clay) obtained from 0.6m to 2.0m.

Client: Stantec Consulting Ltd. **Project Number:** 1000-240-07
Project Name: 2026 Locals Streets Package 26-R-09 **Location:** UTM : 5521610 m N, 635570 m E, St Vital Park South Roadway
Contractor: Maple Leaf Drilling Ltd. **Ground Elevation:** Top of Pavement
Method: 150 mm Solid Stem Auger, B40 Truck Mount **Date Drilled:** January 14, 2026

Sample Type: Grab (G) Shelby Tube (T) Split Spoon (SS) / SPT Split Barrel (SB) / LPT Core (C)

Particle Size Legend: Fines Clay Silt Sand Gravel Cobbles Boulders

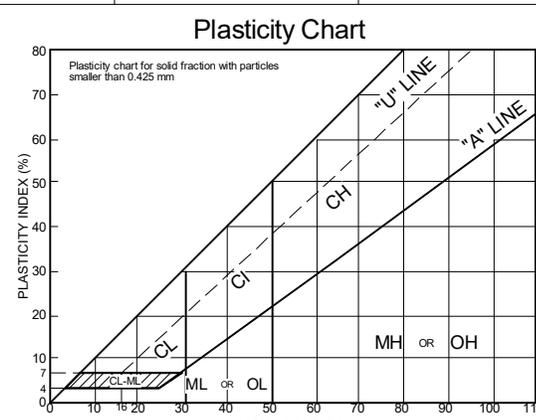
Depth (m)	Soil Symbol	MATERIAL DESCRIPTION	Sample Type	Sample Number	Bulk Unit Wt (kN/m ³)		Particle Size (%)		Undrained Shear Strength (kPa)								
					16	17	18	19	20	21	0	50	100	150	200	250	
0.0 - 0.05		ASPHALT (60mm thick)															
0.05 - 0.3		SAND (FILL) - trace gravel (<20mm diam.), some fines, brown, frozen - moist, compact when thawed, poorly graded, sub-rounded, "pit run", AASHTO: A-3 (I)															
0.3 - 1.5		CLAY and SILT - greyish brown - frozen to 1.4m, moist, stiff when thawed - high plasticity - AASHTO: A-7-6 (I)															
1.5 - 3.0		- very stiff below 1.5m															
0.6 - 1.0			G16														
0.9 - 1.1			G17														
1.2 - 1.4			G18														
1.7 - 1.9			G19														
2.1 - 2.3			G20														
2.4 - 2.6			G21														

END OF TEST HOLE AT 3.0m IN CLAY.
 Notes:
 1) Seepage and sloughing not observed.
 2) Test hole dry and open to 3.0m depth immediately after drilling.
 3) Test hole backfilled with cuttings, bentonite and cold patch asphalt.
 4) Bulk sample (B22 Clay) obtained from 0.6m to 2.0m.

SUB-SURFACE LOG LOGS 2026-01-14 LOCAL STREETS 1000-240-07 0 - A KF GPJ TREK.GDT 2/12/26

GENERAL NOTES

- Classifications are based on the United Soil Classification System and include consistency, moisture, and color. Field descriptions have been modified to reflect results of laboratory tests where deemed appropriate.
- Descriptions on these test hole logs apply only at the specific test hole locations and at the time the test holes were drilled. Variability of soil and groundwater conditions may exist between test hole locations.
- When the following classification terms are used in this report or test hole logs, the primary and secondary soil fractions may be visually estimated.

Major Divisions	USCS Classification	Symbols	Typical Names	Laboratory Classification Criteria		Particle Size			
Coarse-Grained soils (More than half the material is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction is larger than 4.75 mm)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Determine percentages of sand and gravel from grain size curve, depending on percentage of fines (fraction smaller than No. 200 sieve) coarse-grained soils are classified as follows: Less than 5 percent..... GW, GP, SW, SP More than 12 percent..... GM, GC, SM, SC 6 to 12 percent..... Borderline cases requiring dual symbols*	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3	ASTM Sieve sizes #10 to #4 #40 to #10 #200 to #40 < #200			
		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines		Not meeting all gradation requirements for GW				
		GM	Silty gravels, gravel-sand-silt mixtures		Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols			
		GC	Clayey gravels, gravel-sand-silt mixtures		Atterberg limits above "A" line or P.I. greater than 7				
	Sands (More than half of coarse fraction is smaller than 4.75 mm)	Clean sands (Little or no fines)	SW		Well-graded sands, gravelly sands, little or no fines	$C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3	mm 2.00 to 4.75 0.425 to 2.00 0.075 to 0.425 < 0.075		
			SP		Poorly-graded sands, gravelly sands, little or no fines	Not meeting all gradation requirements for SW			
		Sands with fines (Appreciable amount of fines)	SM		Silty sands, sand-silt mixtures	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols		
			SC		Clayey sands, sand-clay mixtures	Atterberg limits above "A" line or P.I. greater than 7			
			Fine-Grained soils (More than half the material is smaller than No. 200 sieve size)		Sils and Clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock floor, silty or clayey fine sands or clayey silts with slight plasticity		Particle Size ASTM Sieve Sizes mm > 300 75 to 300 19 to 75 4.75 to 19
						CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays		
OL	Organic silts and organic silty clays of low plasticity								
Sils and Clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or distomaceous fine sandy or silty soils, organic silts		Material Sand Coarse Medium Fine					
	CH	Inorganic clays of high plasticity, fat clays							
	OH	Organic clays of medium to high plasticity, organic silts							
	Pt	Peat and other highly organic soils			Material Boulders Cobbles Gravel Coarse Fine				
Highly Organic Soils				Von Post Classification Limit	Strong colour or odour, and often fibrous texture				

* Borderline classifications used for soils possessing characteristics of two groups are designated by combinations of groups symbols. For example; GW-GC, well-graded gravel-sand mixture with clay binder.

Other Symbol Types

	Asphalt		Bedrock (undifferentiated)		Cobbles
	Concrete		Limestone Bedrock		Boulders and Cobbles
	Fill		Cemented Shale		Silt Till
			Non-Cemented Shale		Clay Till

LEGEND OF ABBREVIATIONS AND SYMBOLS

- LL - Liquid Limit (%)
 - PL - Plastic Limit (%)
 - PI - Plasticity Index (%)
 - MC - Moisture Content (%)
 - SPT - Standard Penetration Test
 - RQD- Rock Quality Designation
 - Qu - Unconfined Compression
 - Su - Undrained Shear Strength
 - VW - Vibrating Wire Piezometer
 - SI - Slope Incliner
- ∇ Water Level at Time of Drilling
 - ▼ Water Level at End of Drilling
 - ∇ Water Level After Drilling as Indicated on Test Hole Logs

FRACTION OF SECONDARY SOIL CONSTITUENTS ARE BASED ON THE FOLLOWING TERMINOLOGY

TERM	EXAMPLES	PERCENTAGE
and	and CLAY	35 to 50 percent
"y" or "ey"	clayey, silty	20 to 35 percent
some	some silt	10 to 20 percent
trace	trace gravel	1 to 10 percent

TERMS DESCRIBING CONSISTENCY OR COMPACTION CONDITION

The Standard Penetration Test blow count (N) of a non-cohesive soil can be related to compactness condition as follows:

<u>Descriptive Terms</u>	<u>SPT (N) (Blows/300 mm)</u>
Very loose	< 4
Loose	4 to 10
Compact	10 to 30
Dense	30 to 50
Very dense	> 50

The Standard Penetration Test blow count (N) of a cohesive soil can be related to its consistency as follows:

<u>Descriptive Terms</u>	<u>SPT (N) (Blows/300 mm)</u>
Very soft	< 2
Soft	2 to 4
Firm	4 to 8
Stiff	8 to 15
Very stiff	15 to 30
Hard	> 30

The undrained shear strength (Su) of a cohesive soil can be related to its consistency as follows:

<u>Descriptive Terms</u>	<u>Undrained Shear Strength (kPa)</u>
Very soft	< 12
Soft	12 to 25
Firm	25 to 50
Stiff	50 to 100
Very stiff	100 to 200
Hard	> 200



**2026 Local Streets Package 26-R-09
St Vital Park Roads - North, Central, South
Sub-Surface Investigation**

Test Hole No.	Test Hole Location	Pavement Surface		Pavement Structure Material		Subgrade Description	Sample Depth (m)		Moisture Content (%)	Grain Size Analysis				Atterberg Limits			
		Type	Thickness (mm)	Type	Thickness (mm)		Top (m)	Bottom (m)		Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Plastic	Liquid	Plasticity Index	
TH25-05	UTM : 5521458 m N, 633416 m E Located at St Vital Park North Roadway, Westbound curb lane, 2.4 m West of East edge of road.	Asphalt	55	Concrete	-	Clay with Silt, AASHTO: A-7-6 (52)	0.6	0.8	34								
						Clay with Silt, AASHTO: A-7-6 (52)	0.9	1.1	34	39	59	2	0	28	73	45	
						Clay with Silt, AASHTO: A-7-6 (52)	1.2	1.4	31								
						Clay with Silt, AASHTO: A-7-6 (52)	1.6	1.8	32								
						Clay with Silt, AASHTO: A-7-6 (52)	2.0	2.1	36								
						Clay with Silt, AASHTO: A-7-6 (52)	2.4	2.6	37								
TH25-06	UTM : 5521894 m N, 635642 m E Located at St Vital Park Central Roadway, Northbound curb lane, 1.6 m West of East curb.	Asphalt	75	Concrete	-	Sand, AASHTO: A-3 (I)	0.2	0.2	8	20		73		20			
						Clay, AASHTO: A-7-6 (55)	0.6	0.8	39								
						Clay, AASHTO: A-7-6 (55)	0.9	1.1	39	60	29	1		31	84	53	
						Clay, AASHTO: A-7-6 (55)	1.2	1.4	35								
						Clay, AASHTO: A-7-6 (55)	1.6	1.8	33								
						Clay, AASHTO: A-7-6 (55)	2.0	2.1	32								
TH25-07	UTM : 5521610 m N, 635570 m E Located St Vital Park South Roadway, Northbound curb lane, 1.5 m West of East curb.	Asphalt	60	Concrete	-	Clay and Silt, AASHTO: A-7-6 (53)	0.6	0.8	36								
						Clay and Silt, AASHTO: A-7-6 (53)	0.9	1.1	34	52	47	1		23	70	47	
						Clay and Silt, AASHTO: A-7-6 (53)	1.2	1.4	31								
						Clay and Silt, AASHTO: A-7-6 (53)	1.6	1.8	32								
						Clay and Silt, AASHTO: A-7-6 (53)	2.0	2.1	32								
						Clay and Silt, AASHTO: A-7-6 (53)	2.4	2.6	32								



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**Moisture Content Report
 ASTM D2216-98**

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Street Package 26-R-09 (St Vital Park Roads)

Sample Date 14-Jan-26
Test Date 28-Jan-26
Technician Z.Dypiangco

Test Hole	TH26-05	TH26-05	TH26-05	TH26-05	TH26-05	TH26-05
Depth (m)	0.6 - 0.8	0.9 - 1.1	1.2 - 1.4	1.6 - 1.8	2.0 - 2.1	2.4 - 2.6
Sample #	G01	G02	G03	G04	G05	G06
Tare ID	P12	J67	E76	E35	N70	F116
Mass of tare	8.7	6.9	6.9	8.6	8.4	8.4
Mass wet + tare	168.1	475.7	165.3	202.9	204.6	148.8
Mass dry + tare	128.0	355.6	127.8	155.4	152.4	111.0
Mass water	40.1	120.1	37.5	47.5	52.2	37.8
Mass dry soil	119.3	348.7	120.9	146.8	144.0	102.6
Moisture %	33.6%	34.4%	31.0%	32.4%	36.3%	36.8%

Test Hole	TH26-06	TH26-06	TH26-06	TH26-06	TH26-06	TH26-06
Depth (m)	0.2 - 0.2	0.6 - 0.8	0.9 - 1.1	1.2 - 1.4	1.6 - 1.8	2.0 - 2.1
Sample #	G08	G09	G10	G11	G12	G13
Tare ID	costo	W42	A	H10	W94	QT33
Mass of tare	169.4	8.5	6.8	8.5	8.6	8.4
Mass wet + tare	812.6	178.3	464.5	208.6	133.9	164.3
Mass dry + tare	765.2	130.6	335.2	156.8	103.0	126.6
Mass water	47.4	47.7	129.3	51.8	30.9	37.7
Mass dry soil	595.8	122.1	328.4	148.3	94.4	118.2
Moisture %	8.0%	39.1%	39.4%	34.9%	32.7%	31.9%

Test Hole	TH26-06	TH26-07	TH26-07	TH26-07	TH26-07	TH26-07
Depth (m)	2.4 - 2.6	0.6 - 0.8	0.9 - 1.1	1.2 - 1.4	1.6 - 1.8	2.0 - 2.1
Sample #	G14	G16	G17	G18	G19	G20
Tare ID	F13	AA10	QT13	E87	N93	MD
Mass of tare	7.0	6.8	8.4	6.9	8.5	7.0
Mass wet + tare	173.9	176.1	465.3	224.4	184.3	167.1
Mass dry + tare	131.8	131.4	350.2	172.8	141.4	128.6
Mass water	42.1	44.7	115.1	51.6	42.9	38.5
Mass dry soil	124.8	124.6	341.8	165.9	132.9	121.6
Moisture %	33.7%	35.9%	33.7%	31.1%	32.3%	31.7%



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Moisture Content Report ASTM D2216-98

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Street Package 26-R-09 (St Vital Park Roads)

Sample Date 14-Jan-26
Test Date 28-Jan-26
Technician Z.Dypiangco

Test Hole	TH26-07					
Depth (m)	2.4 - 2.6					
Sample #	G21					
Tare ID	K17					
Mass of tare	8.4					
Mass wet + tare	213.1					
Mass dry + tare	163.8					
Mass water	49.3					
Mass dry soil	155.4					
Moisture %	31.7%					



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Atterberg Limits
ASTM D4318-17e1

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St. Vital Park Road)

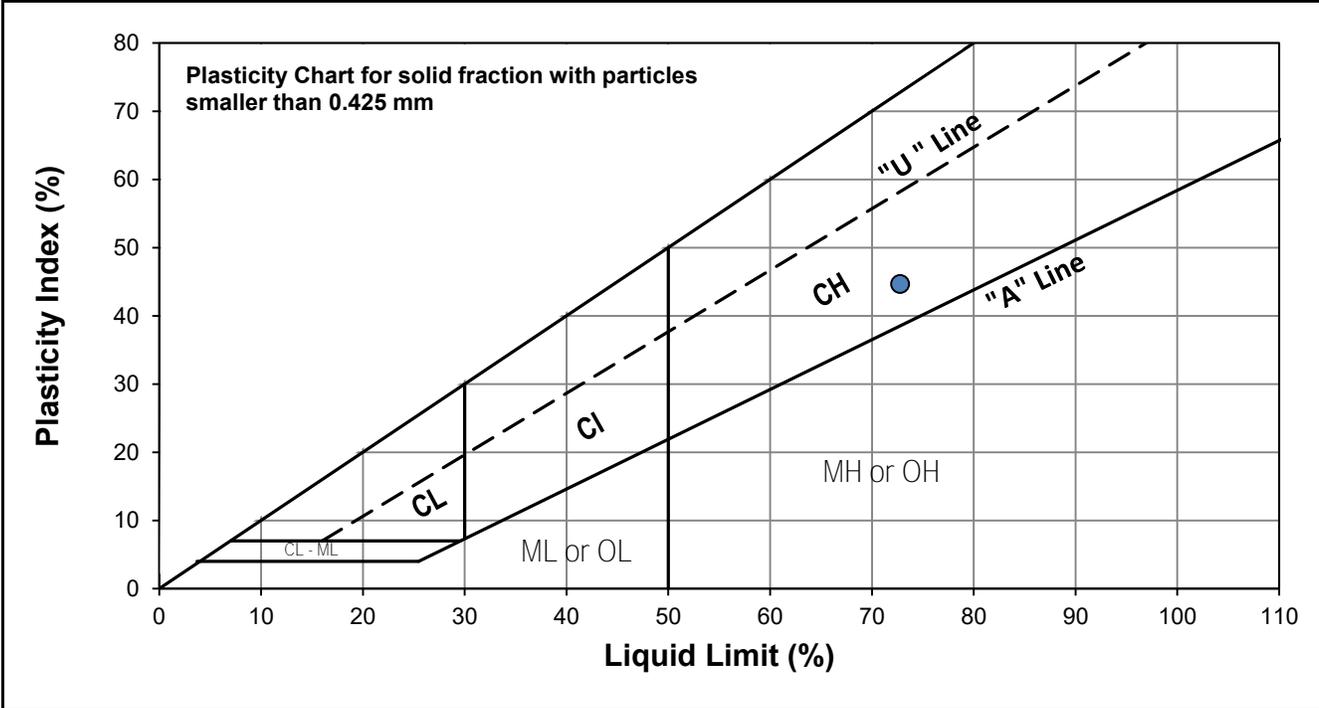


Test Hole TH26-05
Sample # G02
Depth (m) 0.9 - 1.1
Sample Date 16-Jan-26
Test Date 30-Jan-26
Technician Z.Dypiangco

Liquid Limit	73
Plastic Limit	28
Plasticity Index	45

Liquid Limit

Trial #	1	2	3
Number of Blows (N)	19	25	31
Mass Tare (g)	14.158	14.062	14.042
Mass Wet Soil + Tare (g)	22.170	23.470	20.527
Mass Dry Soil + Tare (g)	18.754	19.511	17.820
Mass Water (g)	3.416	3.959	2.707
Mass Dry Soil (g)	4.596	5.449	3.778
Moisture Content (%)	74.326	72.656	71.652



Plastic Limit

Trial #	1	2	3	4	5
Mass Tare (g)	14.217	13.902			
Mass Wet Soil + Tare (g)	21.476	21.825			
Mass Dry Soil + Tare (g)	19.868	20.100			
Mass Water (g)	1.608	1.725			
Mass Dry Soil (g)	5.651	6.198			
Moisture Content (%)	28.455	27.832			

Note: Additional information recorded/measured for this test is available upon request.

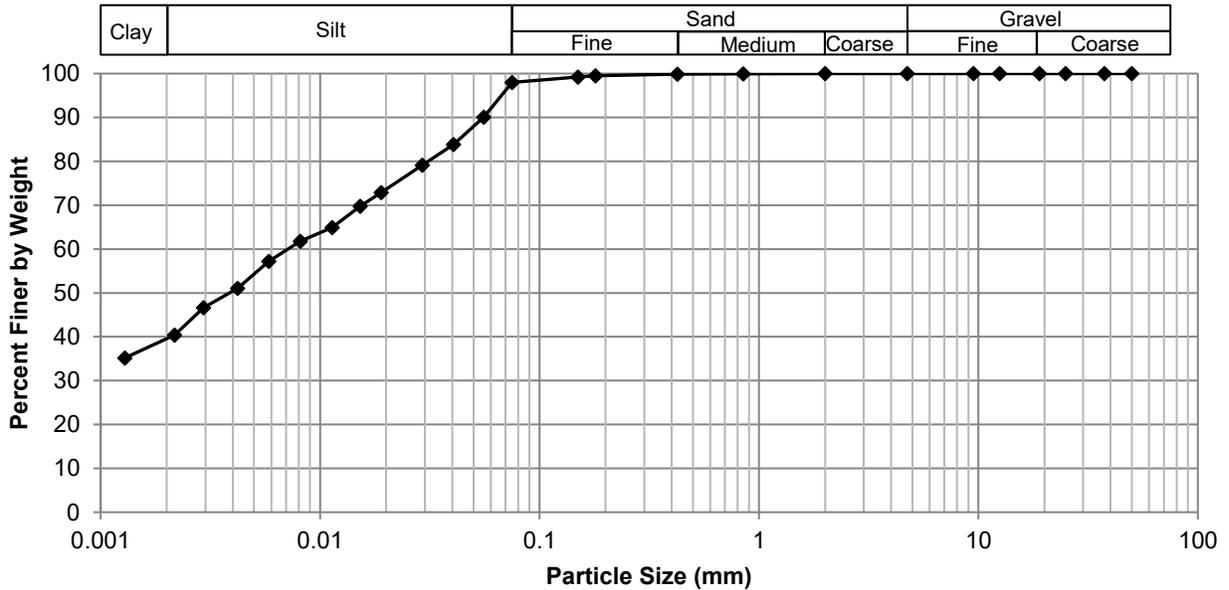
Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St Vital Park Road)



Test Hole TH26-05
Sample # G02
Depth (m) 1.1 - 1.2
Sample Date 16-Jan-26
Test Date 30-Jan-26
Technician Z.Dypiangco

Gravel	0.0%
Sand	2.0%
Silt	58.6%
Clay	39.4%

Particle Size Distribution Curve



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	97.99
37.5	100.00	2.00	100.00	0.0557	90.06
25.0	100.00	0.850	99.94	0.0405	83.81
19.0	100.00	0.425	99.87	0.0293	79.12
12.5	100.00	0.180	99.49	0.0190	72.87
9.50	100.00	0.150	99.22	0.0152	69.74
4.75	100.00	0.075	97.99	0.0114	64.94
				0.0081	61.81
				0.0058	57.23
				0.0042	51.09
				0.0029	46.62
				0.0022	40.42
				0.0013	35.18

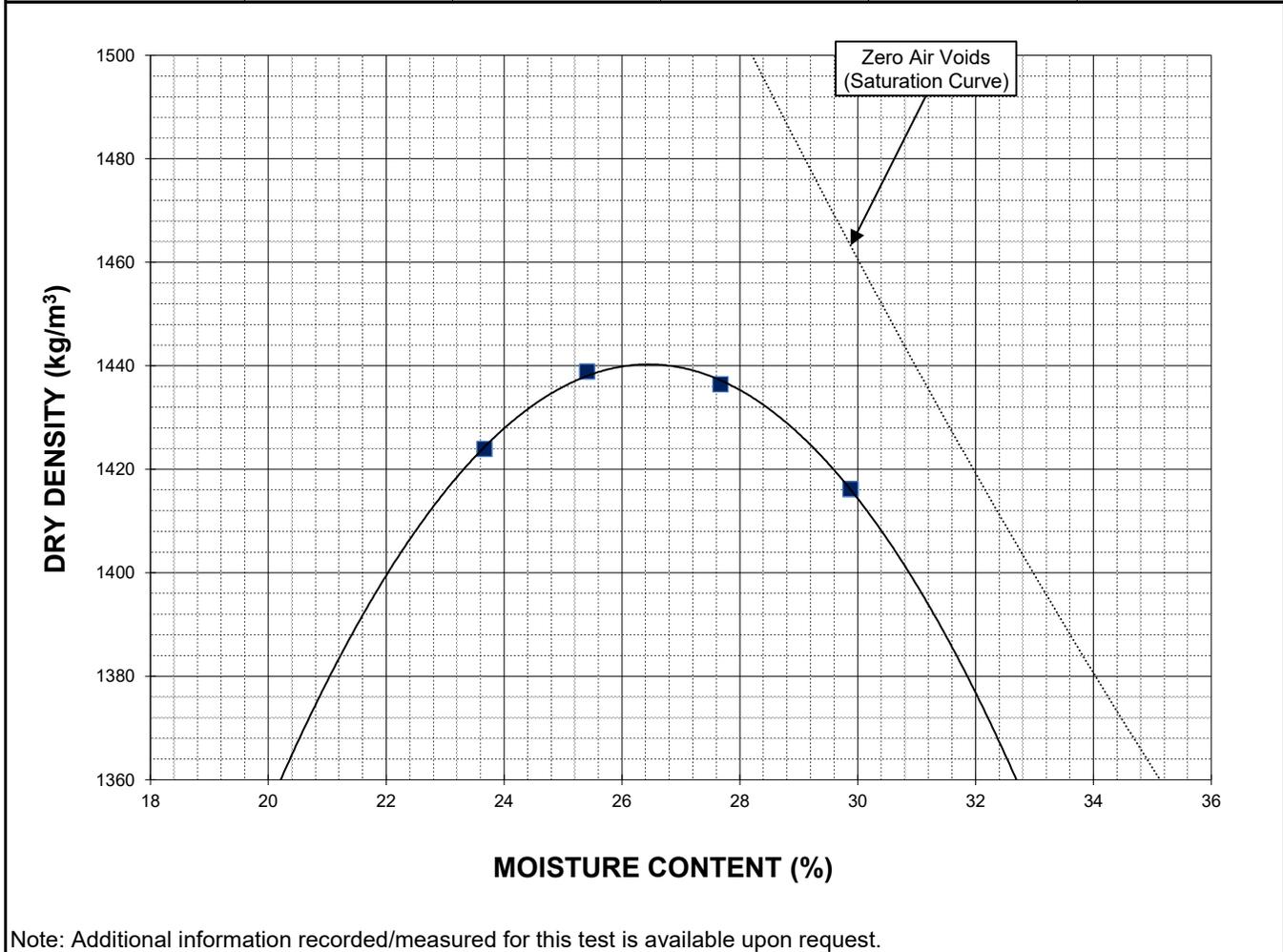
Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09



Sample # B07
Source TH26-05 (St. Vital Park North Road)
Material Clay
Sample Date 14-Jan-26
Test Date 28-Jan-26
Technician A. Dustmamatov

Maximum Dry Density (kg/m³)	1440
Optimum Moisture (%)	26.5

Trial Number	1	2	3	4
Wet Density (kg/m³)	1761	1805	1834	1839
Dry Density (kg/m³)	1424	1439	1436	1416
Moisture Content (%)	23.7	25.4	27.7	29.9



Note: Additional information recorded/measured for this test is available upon request.



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California Bearing Ratio Test Data Sheet

ASTM D1883-21

Project No.	1000-240-07	Source	TH26-05 (St. Vital Park Road)
Client	Stantec Consulting Ltd.	Material	Clay
Project	2026 Local Streets Package 26-R-09	Sample Date	14-Jan-26
Sample #	B07	Test Date	30-Jan-26
		Technician	A. Dustmamatov

Proctor Results (ASTM D698)

Maximum Dry Density	1440 kg/m ³
Optimum Moisture Content	26.5 %
Material Retained on 19 mm Sieve	0.0 %

CBR Sample Compaction

Dry Density	1367 kg/m ³
Initial Moisture Content	27.4 %
Relative Density	95.0 % SPMDD

Soaking Results

Surcharge	4.54 kg
Swell	4.3 %
Moisture Content in top 25 mm	42.2 %
Immersion Period	96 h

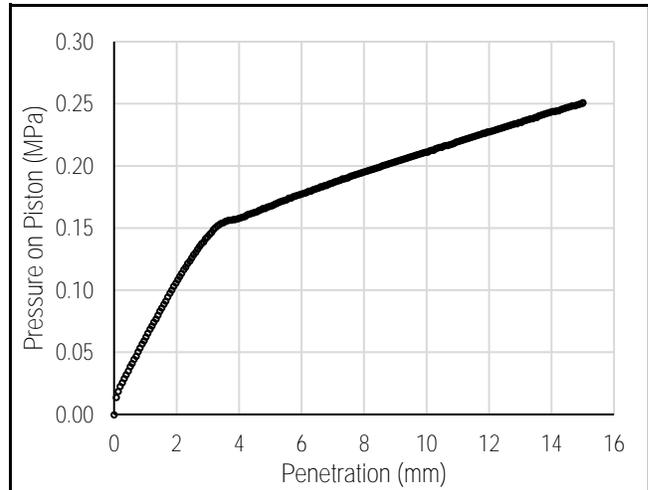
CBR Results

CBR at 2.54 mm	1.9 %
CBR at 5.08 mm	1.6 %
Zero Correction	0 mm

Test Data

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.04	0.04
1.27	0.07	0.07
1.91	0.10	0.10
2.54	0.13	0.13
3.18	0.15	0.15
3.81	0.16	0.16
4.45	0.16	0.16
5.08	0.17	0.17
7.62	0.19	0.19
10.16	0.21	0.21
12.70	0.23	0.23

Load/Penetration Curve

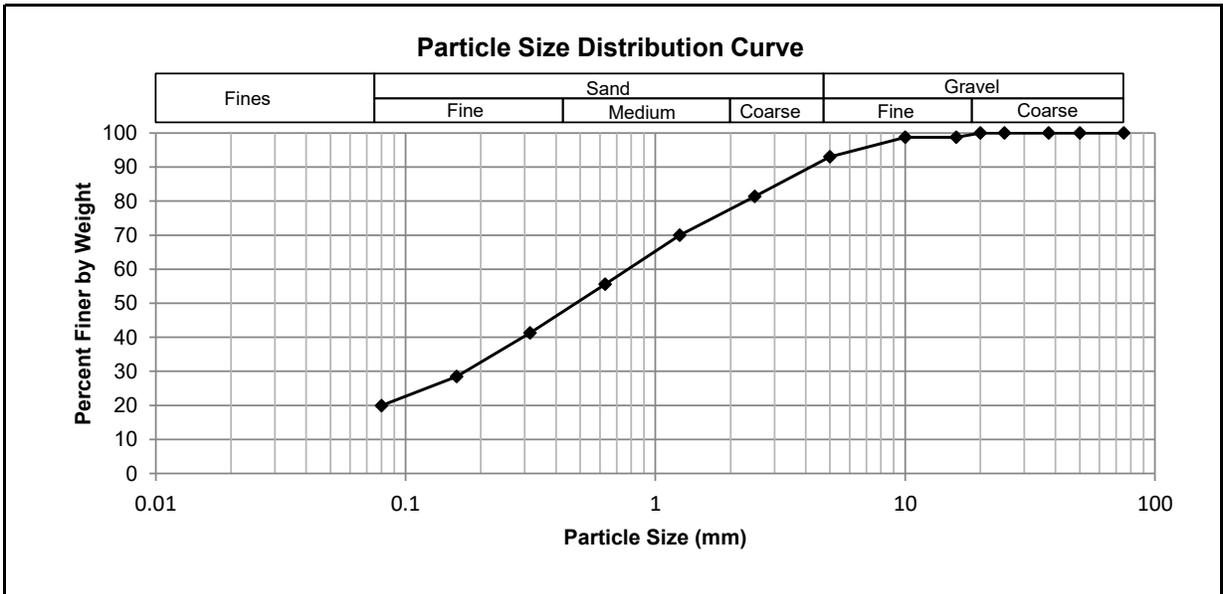


Comments:

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Street Package 26-R-09 (St. Vital Park Road)

Test Hole TH26-06
Sample # G08
Depth 0.15 - 0.25 m
Date Sampled 14-Jan-26
Date Tested 30-Jan-26
Technician Z.Dypiangco

Total Weight (g)	595.8
Gravel %	7.0
Sand %	73.1
Fines %	19.9



Sieve Opening (mm)	Percent Passing	Specification (Min-Max)
37.5	100	-
25.0	100	-
20.0	100	-
16.0	99	-
10.0	99	-
5.0	93	-
2.5	81	-
1.25	70	-
0.630	56	-
0.315	41	-
0.160	28	-
0.080	20	-



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Atterberg Limits
ASTM D4318-17e1

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St. Vital Park Road)

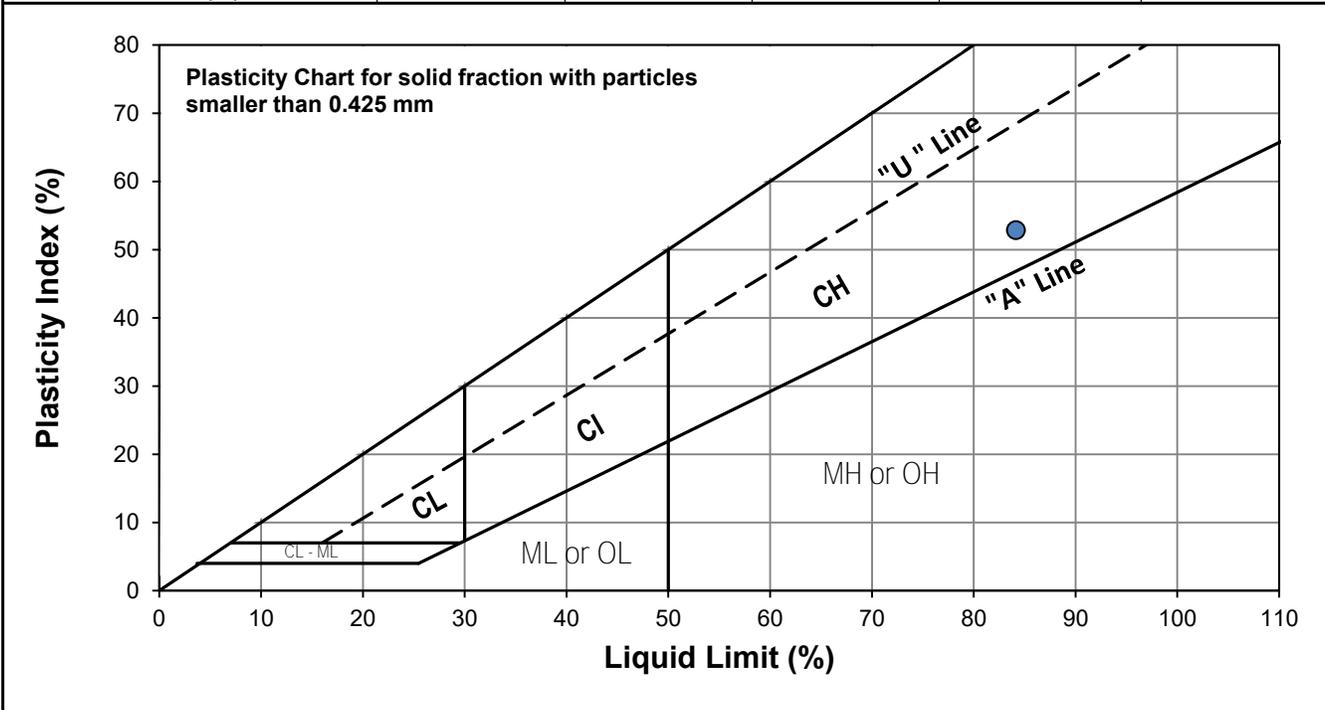


Test Hole TH26-06
Sample # G10
Depth (m) 0.9 - 1.1
Sample Date 16-Jan-26
Test Date 30-Jan-26
Technician Z.Dypiangco

Liquid Limit	84
Plastic Limit	31
Plasticity Index	53

Liquid Limit

Trial #	1	2	3
Number of Blows (N)	19	23	32
Mass Tare (g)	13.952	13.918	13.735
Mass Wet Soil + Tare (g)	23.630	25.016	22.507
Mass Dry Soil + Tare (g)	19.119	19.917	18.573
Mass Water (g)	4.511	5.099	3.934
Mass Dry Soil (g)	5.167	5.999	4.838
Moisture Content (%)	87.304	84.997	81.315



Plastic Limit

Trial #	1	2	3	4	5
Mass Tare (g)	13.923	14.136			
Mass Wet Soil + Tare (g)	21.961	22.854			
Mass Dry Soil + Tare (g)	20.045	20.771			
Mass Water (g)	1.916	2.083			
Mass Dry Soil (g)	6.122	6.635			
Moisture Content (%)	31.297	31.394			

Note: Additional information recorded/measured for this test is available upon request.

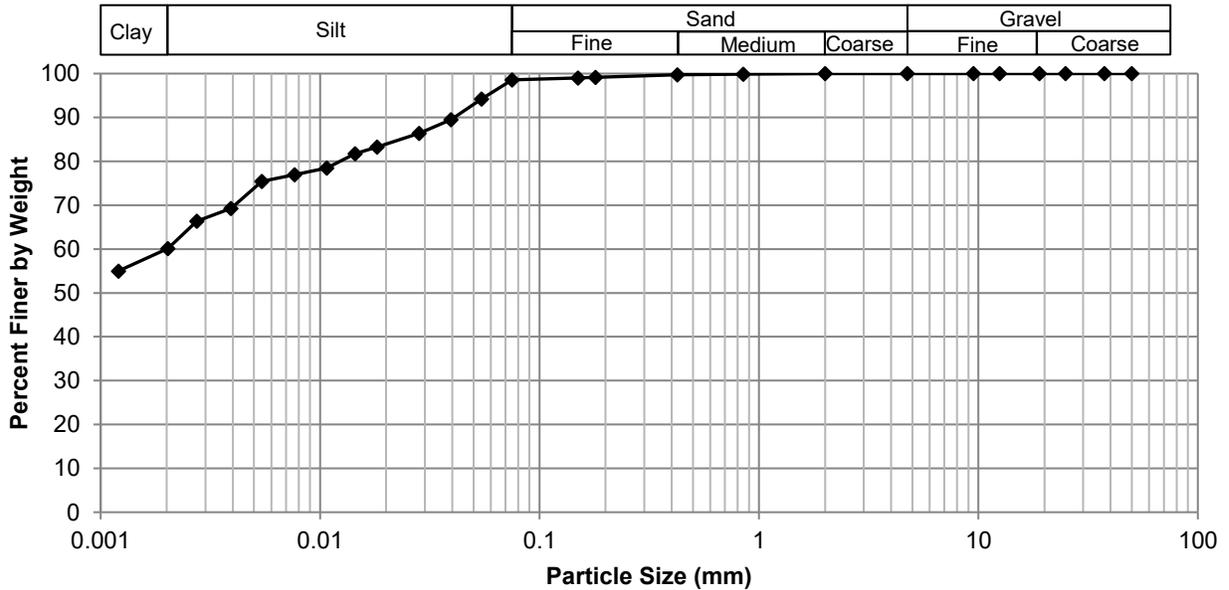
Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St Vital Park Roads)

Test Hole TH26-06
Sample # G10
Depth (m) 1.1 - 1.2
Sample Date 16-Jan-26
Test Date 30-Jan-26
Technician Z.Dypiangco



Gravel	0.0%
Sand	1.4%
Silt	38.6%
Clay	60.0%

Particle Size Distribution Curve



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	98.59
37.5	100.00	2.00	100.00	0.0545	94.21
25.0	100.00	0.850	99.87	0.0395	89.52
19.0	100.00	0.425	99.71	0.0283	86.40
12.5	100.00	0.180	99.16	0.0182	83.27
9.50	100.00	0.150	99.00	0.0145	81.71
4.75	100.00	0.075	98.59	0.0107	78.50
				0.0076	76.93
				0.0054	75.45
				0.0039	69.29
				0.0027	66.33
				0.0020	60.12
				0.0012	55.00

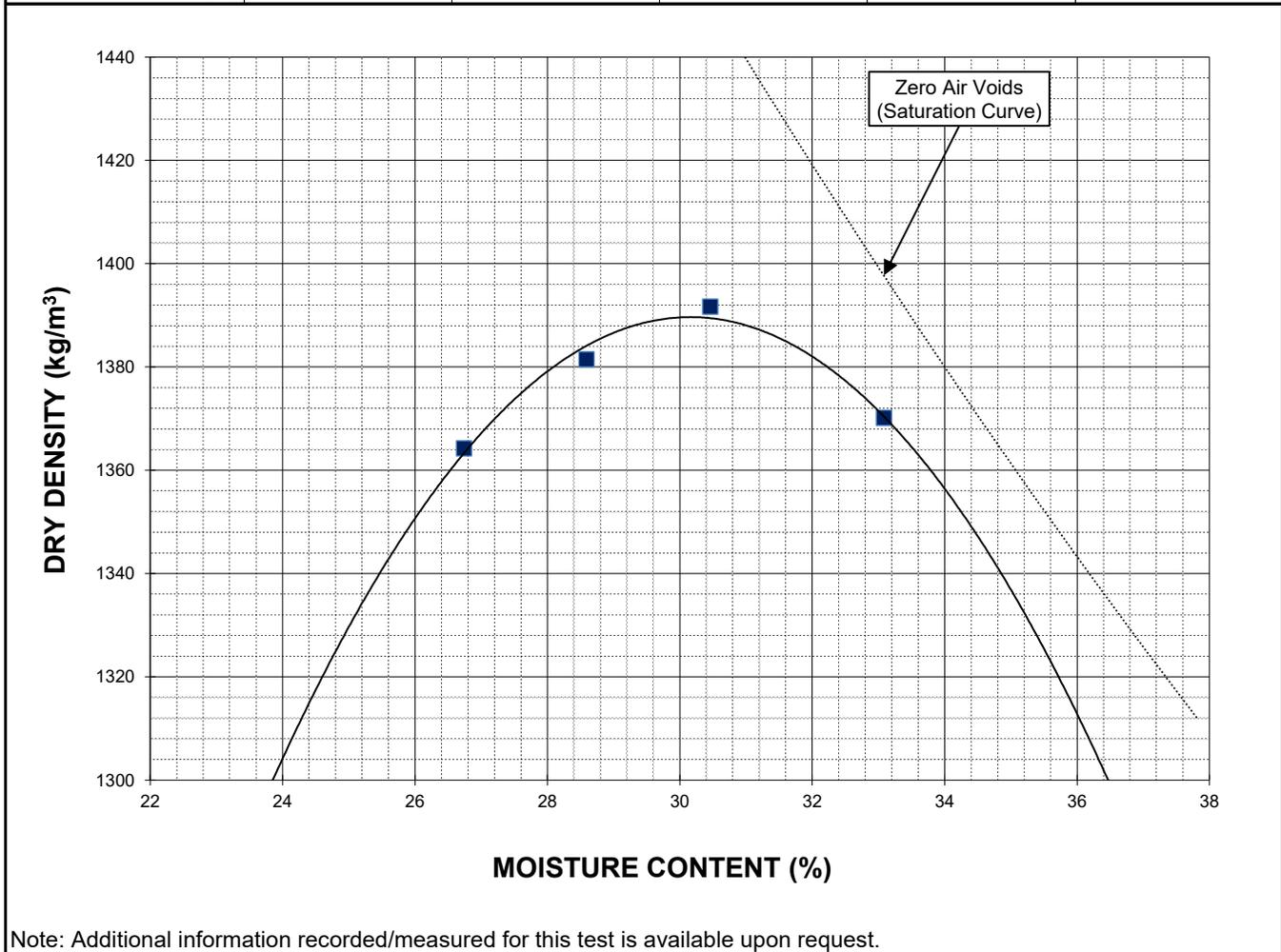
Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09



Sample # B15
Source TH26-06 (St. Vital Park Central Road)
Material Clay
Sample Date 14-Jan-26
Test Date 29-Jan-26
Technician A. Dustmamatov

Maximum Dry Density (kg/m³)	1390
Optimum Moisture (%)	30.2

Trial Number	1	2	3	4
Wet Density (kg/m³)	1729	1776	1816	1823
Dry Density (kg/m³)	1364	1381	1392	1370
Moisture Content (%)	26.7	28.6	30.5	33.1



Note: Additional information recorded/measured for this test is available upon request.



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California Bearing Ratio Test Data Sheet

ASTM D1883-21

Project No.	1000-240-07	Source	TH26-06 (St.Vital Park Road)
Client	Stantec Consulting Ltd.	Material	Clay
Project	2026 Local Streets Package 26-R-09	Sample Date	14-Jan-26
Sample #	B15	Test Date	02-Feb-26
		Technician	A. Dustmamatov

Proctor Results (ASTM D698)

Maximum Dry Density	1390 kg/m ³
Optimum Moisture Content	30.2 %
Material Retained on 19 mm Sieve	0.0 %

CBR Sample Compaction

Dry Density	1327 kg/m ³
Initial Moisture Content	30.1 %
Relative Density	95.5 % SPMDD

Soaking Results

Surcharge	4.54 kg
Swell	4.0 %
Moisture Content in top 25 mm	44.3 %
Immersion Period	96 h

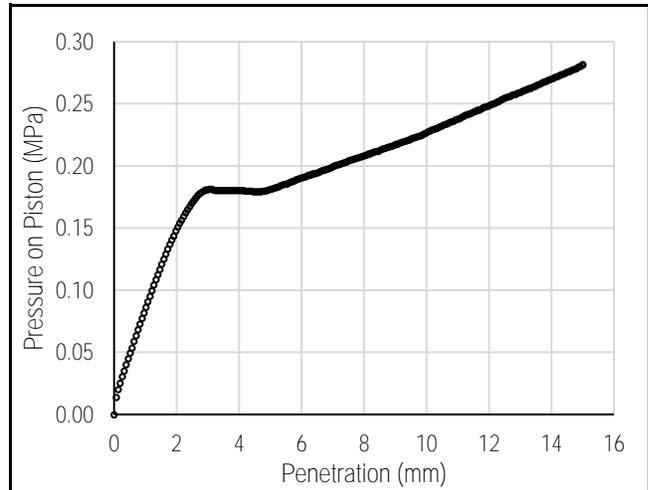
CBR Results

CBR at 2.54 mm	2.5 %
CBR at 5.08 mm	1.8 %
Zero Correction	0 mm

Test Data

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.06	0.06
1.27	0.10	0.10
1.91	0.14	0.14
2.54	0.17	0.17
3.18	0.18	0.18
3.81	0.18	0.18
4.45	0.18	0.18
5.08	0.18	0.18
7.62	0.21	0.21
10.16	0.23	0.23
12.70	0.26	0.26

Load/Penetration Curve



Comments:



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Atterberg Limits
ASTM D4318-17e1

Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St. Vital Park Road)

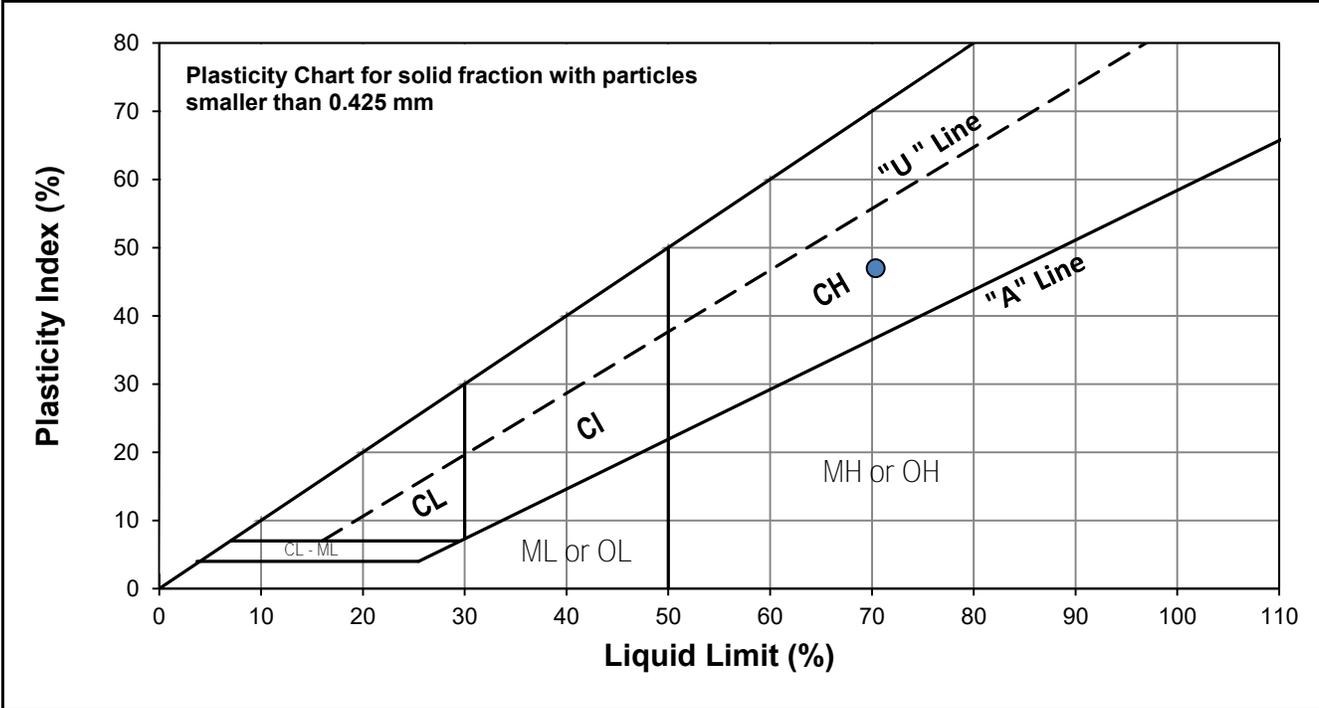


Test Hole TH26-07
Sample # G17
Depth (m) 0.9 - 1.1
Sample Date 14-Jan-26
Test Date 02-Feb-26
Technician A.Bhullar

Liquid Limit	70
Plastic Limit	23
Plasticity Index	47

Liquid Limit

Trial #	1	2	3
Number of Blows (N)	15	28	32
Mass Tare (g)	14.068	14.056	13.911
Mass Wet Soil + Tare (g)	27.773	25.895	28.088
Mass Dry Soil + Tare (g)	22.049	21.017	22.265
Mass Water (g)	5.724	4.878	5.823
Mass Dry Soil (g)	7.981	6.961	8.354
Moisture Content (%)	71.720	70.076	69.703



Plastic Limit

Trial #	1	2	3	4	5
Mass Tare (g)	14.065	13.903			
Mass Wet Soil + Tare (g)	21.158	22.474			
Mass Dry Soil + Tare (g)	19.810	20.849			
Mass Water (g)	1.348	1.625			
Mass Dry Soil (g)	5.745	6.946			
Moisture Content (%)	23.464	23.395			

Note: Additional information recorded/measured for this test is available upon request.

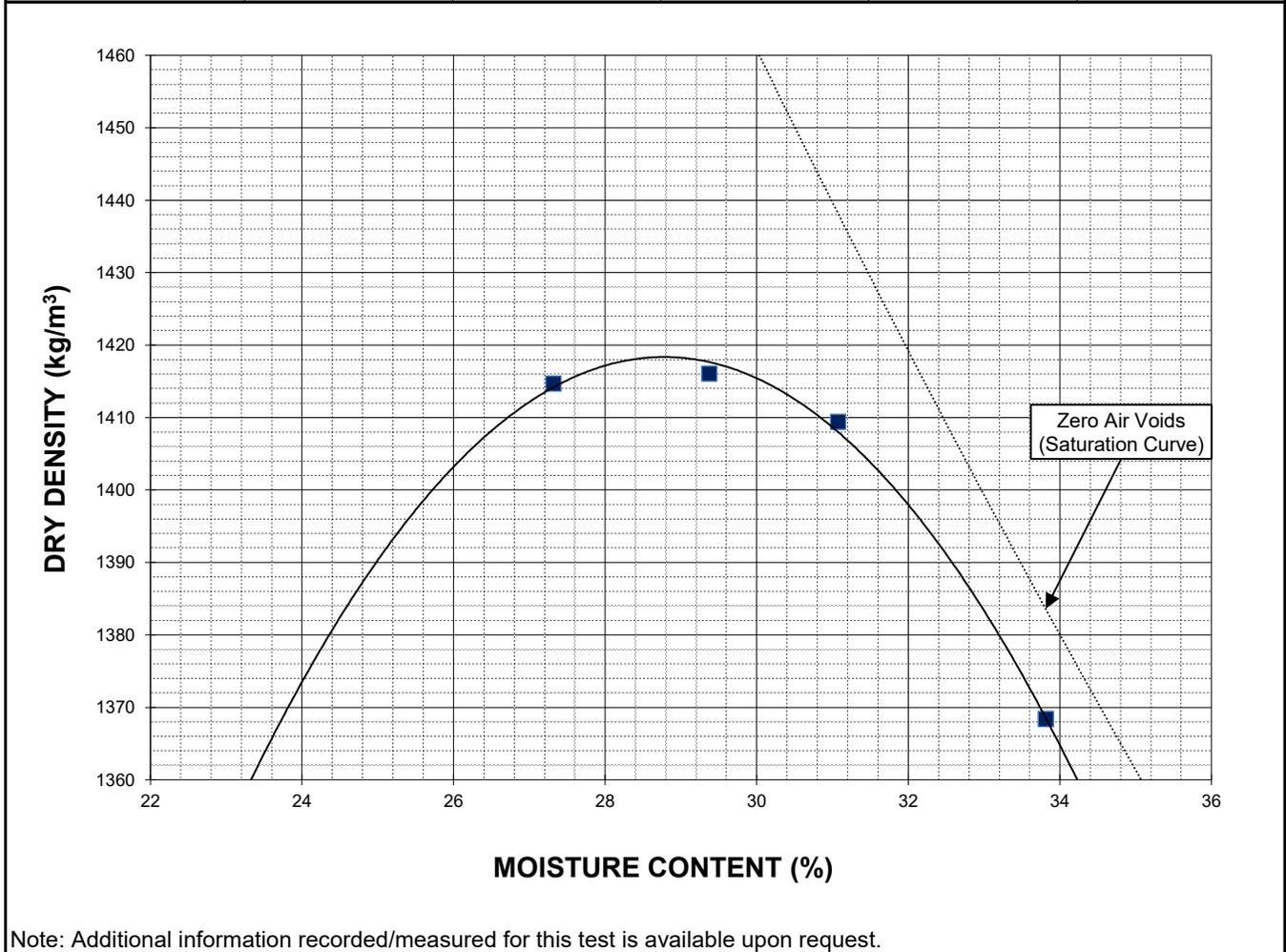
Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09



Sample # B22
Source TH26-07 (St. Vital Park South Road)
Material Clay
Sample Date 14-Jan-26
Test Date 28-Jan-26
Technician A. Dustmamatov

Maximum Dry Density (kg/m³)	1418
Optimum Moisture (%)	28.8

Trial Number	1	2	3	4
Wet Density (kg/m³)	1801	1832	1847	1831
Dry Density (kg/m³)	1415	1416	1409	1368
Moisture Content (%)	27.3	29.4	31.1	33.8

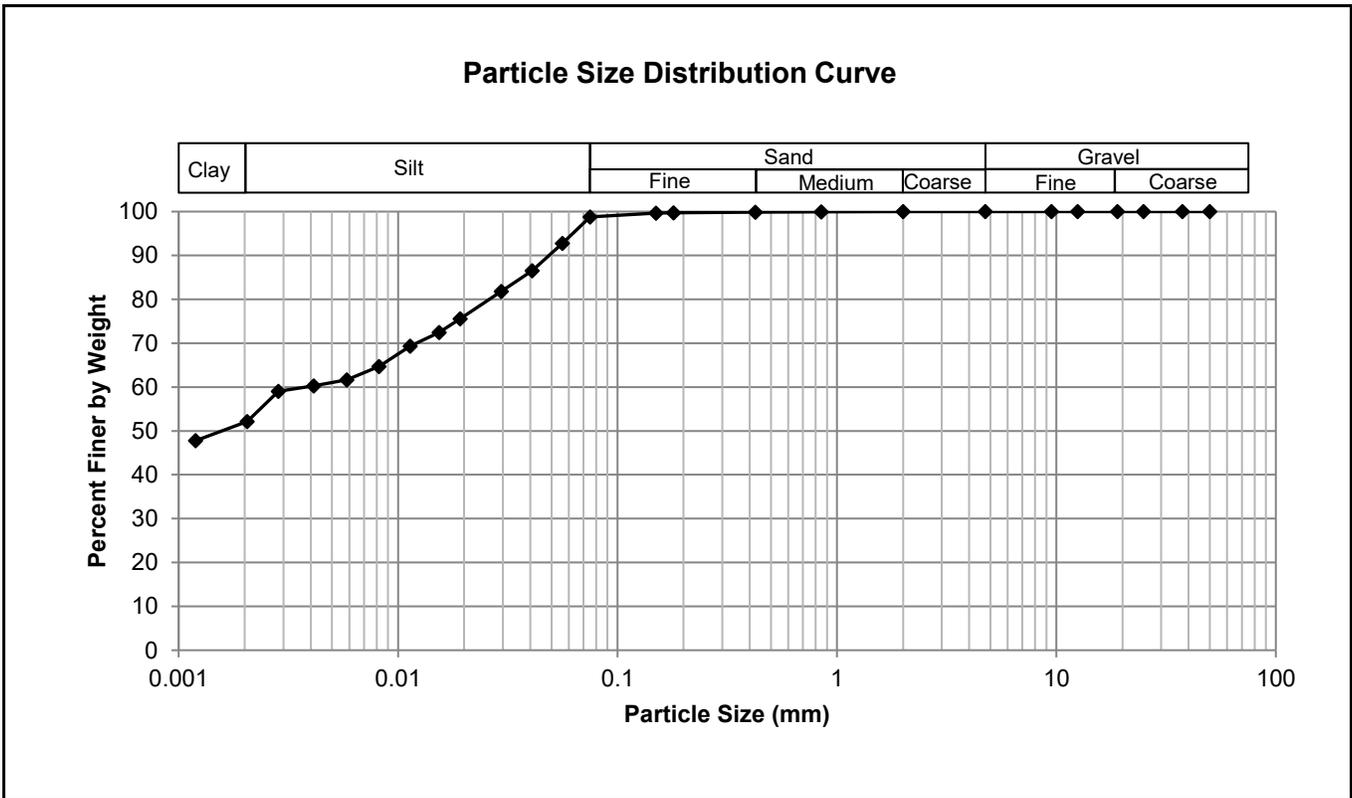


Project No. 1000-240-07
Client Stantec Consulting Ltd.
Project 2026 Local Streets Package 26-R-09 (St Vital Park Roads)

Test Hole TH26-07
Sample # G17
Depth (m) 1.1 - 1.2
Sample Date 14-Jan-26
Test Date 02-Feb-26
Technician Z.Dypiangco



Gravel	0.0%
Sand	1.2%
Silt	46.9%
Clay	51.9%



Gravel		Sand		Silt and Clay	
Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing	Particle Size (mm)	Percent Passing
50.0	100.00	4.75	100.00	0.0750	98.79
37.5	100.00	2.00	100.00	0.0561	92.76
25.0	100.00	0.850	99.94	0.0409	86.51
19.0	100.00	0.425	99.85	0.0295	81.82
12.5	100.00	0.180	99.69	0.0192	75.56
9.50	100.00	0.150	99.63	0.0154	72.44
4.75	100.00	0.075	98.79	0.0114	69.36
				0.0082	64.71
				0.0058	61.68
				0.0041	60.25
				0.0028	59.05
				0.0021	52.14
				0.0012	47.77



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California Bearing Ratio Test Data Sheet

ASTM D1883-21

Project No.	1000-240-07	Source	TH26-07 (St. Vital Park Road)
Client	Stantec Consulting Ltd.	Material	Clay
Project	2026 Local Streets Package 26-R-09	Sample Date	14-Jan-26
Sample #	B22	Test Date	02-Feb-26
		Technician	A. Dustmammatov

Proctor Results (ASTM D698)

Maximum Dry Density	1418 kg/m ³
Optimum Moisture Content	28.8 %
Material Retained on 19 mm Sieve	0.0 %

CBR Sample Compaction

Dry Density	1354 kg/m ³
Initial Moisture Content	28.5 %
Relative Density	95.5 % SPMDD

Soaking Results

Surcharge	4.54 kg
Swell	3.5 %
Moisture Content in top 25 mm	41.1 %
Immersion Period	96 h

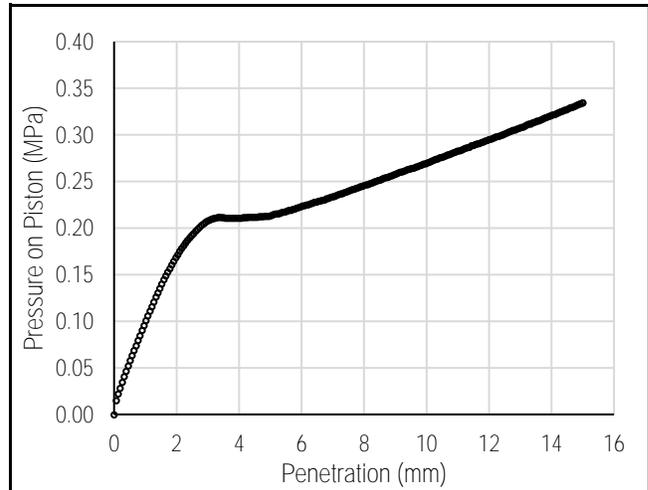
CBR Results

CBR at 2.54 mm	2.8 %
CBR at 5.08 mm	2.1 %
Zero Correction	0 mm

Test Data

Penetration (mm)	Measured Pressure (MPa)	Corrected Pressure (MPa)
0.64	0.07	0.07
1.27	0.12	0.12
1.91	0.16	0.16
2.54	0.19	0.19
3.18	0.21	0.21
3.81	0.21	0.21
4.45	0.21	0.21
5.08	0.21	0.21
7.62	0.24	0.24
10.16	0.27	0.27
12.70	0.30	0.30

Load/Penetration Curve



Comments:



Photo 1: Pavement Core Sample at TH26-05



Photo 2: Pavement Core Sample at TH25-06



Photo 3: Pavement Core Sample at TH25-07



2026 Local Streets Package 26-R-09
St Vital Park Roads - North, Central, South

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-53	UTM : 5521013 m N, 633637 m E; Located at St Vital Park South Roadway, Eastbound median lane, 2.8 m South of North edge of road.	Asphalt	60	Concrete	-
PC26-54	UTM : 5520991 m N, 633477 m E; Located at St Vital Park South Roadway, Eastbound median lane, 2.2 m South of North edge of road.	Asphalt	65	Concrete	-
PC26-55	UTM : 5521025 m N, 633402 m E; Located at St Vital Park South Roadway, Eastbound curb lane, 2.0 m North of South edge of road.	Asphalt	140	Concrete	-
PC26-56	UTM : 5521084 m N, 633346 m E; Located at St Vital Park South Roadway, Eastbound curb lane, 1.7 m North of South edge of road.	Asphalt	115	Concrete	-
PC26-57	UTM : 5521137 m N, 633345 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 3.2 m North of South edge of road.	Asphalt	70	Concrete	-
PC26-58	UTM : 5521189 m N, 633396 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 2.5 m North of South edge of road.	Asphalt	65	Concrete	-
PC26-59	UTM : 5521228 m N, 633463 m E; Located at St Vital Park Central Roadway, Westbound curb lane, 1.6 m South of North curb.	Asphalt	65	Concrete	-
PC26-60	UTM : 5521226 m N, 633526 m E; Located at St Vital Park Central Roadway, Eastbound median lane, 0.7 m South of North edge of road.	Asphalt	90	Concrete	-
PC26-61	UTM : 5521187 m N, 633592 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 1.2 m North of South edge of road.	Asphalt	80	Concrete	-
PC26-62	UTM : 5521170 m N, 633653 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 1.4 m North of South edge of road.	Asphalt	55	Concrete	-
PC26-63	UTM : 5521207 m N, 633804 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 1.3 m North of South edge of road.	Asphalt	65	Concrete	-
PC26-64	UTM : 5521212 m N, 633875 m E; Located at St Vital Park Central Roadway, Eastbound curb lane, 1.7 m North of South edge of road.	Asphalt	65	Concrete	-
PC26-65	UTM : 5521458 m N, 633416 m E; Located at St Vital Park North Roadway, Westbound curb lane, 2.4 m West of East edge of road.	Asphalt	65	Concrete	-



2026 Local Streets Package 26-R-09
St Vital Park Roads - North, Central, South

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-66	UTM : 5521503 m N, 633501 m E; Located at St Vital Park North Roadway, Westbound median lane, 2.4 m North of South edge of road.	Asphalt	65	Concrete	-
PC26-67	UTM : 5521526 m N, 633574 m E; Located at St Vital Park North Roadway, Westbound curb lane, 2.2 m South of North edge of road.	Asphalt	70	Concrete	-
PC26-68	UTM : 5521498 m N, 633638 m E; Located at St Vital Park North Roadway, Westbound curb lane, 2.5 m South of North edge of road.	Asphalt	140	Concrete	200
PC26-69	UTM : 5521432 m N, 633778 m E; Located at St Vital Park North Roadway, Westbound median lane, 1.1 m North of South edge of road.	Asphalt	40	Concrete	-
PC26-70	UTM : 5521423 m N, 633864 m E; Located at St Vital Park North Roadway, Westbound median lane, 1.6 m North of South edge of road.	Asphalt	75	Concrete	-
PC26-71	UTM : 5521401 m N, 633942 m E; Located at St Vital Park North Roadway, Westbound median lane, 1.1 m North of South edge of road.	Asphalt	65	Concrete	-
PC26-72	UTM : 5521380 m N, 633999 m E; Located at St Vital Park North Roadway, Westbound curb lane, 1.2 m Southwest of edge of road.	Asphalt	65	Concrete	-



Photo 1: Pavement Core Sample at PC26-53



Photo 2: Pavement Core Sample at PC26-54



Photo 3: Pavement Core Sample at PC26-55



Photo 4: Pavement Core Sample at PC26-56



Photo 5: Pavement Core Sample at PC26-57



Photo 6: Pavement Core Sample at PC26-58



Photo 7: Pavement Core Sample at PC26-59



Photo 8: Pavement Core Sample at PC26-60



Photo 9: Pavement Core Sample at PC26-61

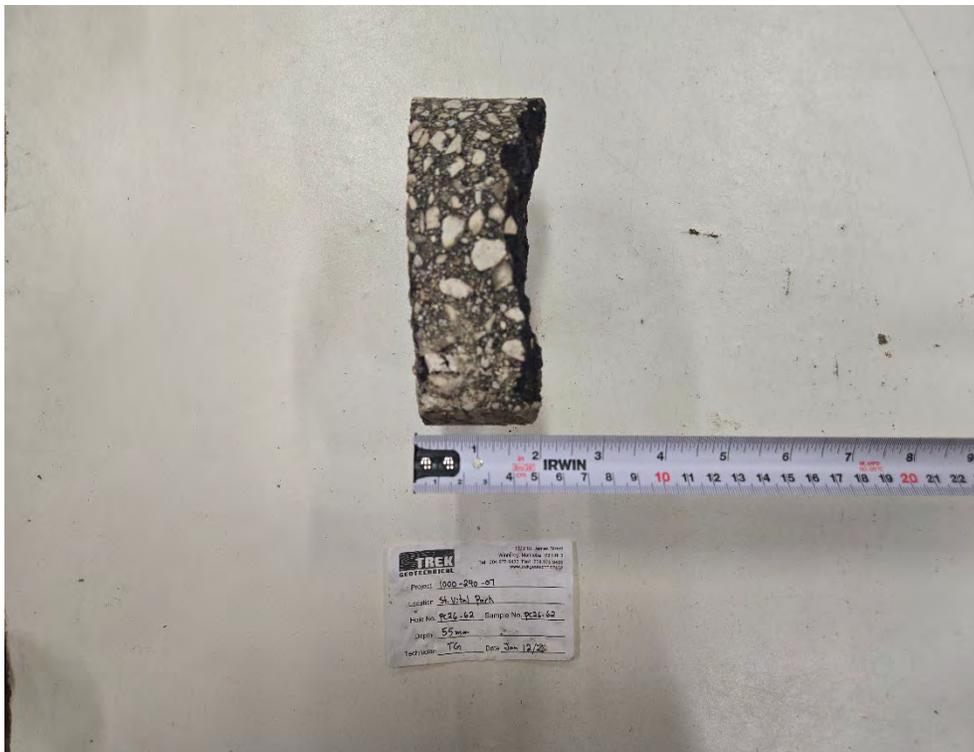


Photo 10: Pavement Core Sample at PC26-62



Photo 11: Pavement Core Sample at PC26-63



Photo 12: Pavement Core Sample at PC26-64



Photo 13: Pavement Core Sample at PC26-65



Photo 14: Pavement Core Sample at PC26-66



Photo 15: Pavement Core Sample at PC26-67



Photo 16: Pavement Core Sample at PC26-68



Photo 17: Pavement Core Sample at PC26-69



Photo 18: Pavement Core Sample at PC26-70



Photo 19: Pavement Core Sample at PC26-71



Photo 20: Pavement Core Sample at PC26-72

Appendix C
Summary Tables and Photographs of Pavement Core Samples
Glen Avenue – Dunkirk Drive to Killarney Street



2026 Local Streets Package 26-R-09
Glen Ave - Dunkirk Dr to Killarney St

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-01	UTM : 5523582 m N, 635243 m E; Located at #63 Glen Ave, Westbound curb lane, 1.9 m South of North curb.	Asphalt	-	Concrete	170
PC26-02	UTM : 5523612 m N, 635300 m E; Located at #55 Glen Ave, Westbound curb lane, 1.8 m South of North curb.	Asphalt	-	Concrete	175
PC26-03	UTM : 5523647 m N, 635357 m E; Located at #47 Glen Ave, Westbound curb lane, 2.5 m South of North curb.	Asphalt	-	Concrete	160
PC26-04	UTM : 5523684 m N, 635426 m E; Located at #39 Glen Ave, Westbound curb lane, 2.5 m South of North curb.	Asphalt	-	Concrete	160
PC26-05	UTM : 5523713 m N, 635478 m E; Located at #33 Glen Ave, Westbound curb lane, 2.5 m South of North curb.	Asphalt	100	Concrete	300

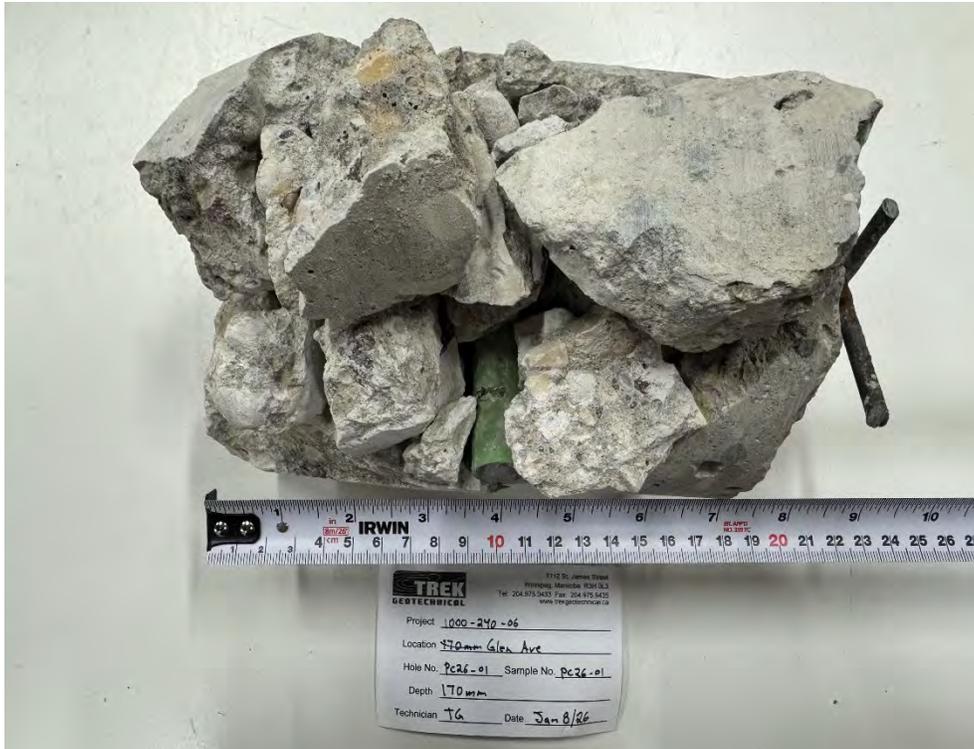


Photo 1: Pavement Core Sample at PC26-01



Photo 2: Pavement Core Sample at PC26-02



Photo 3: Pavement Core Sample at PC26-03



Photo 4: Pavement Core Sample at PC26-04



Photo 5: Pavement Core Sample at PC26-05

Appendix F
Summary Table and Photographs of Pavement Core Samples
Sandralin Bay – Avalon Road to North Limit



2026 Local Streets Package 26-R-09
Sandralin Bay - Avalon Rd to North Limit

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-23	UTM : 5520640 m N, 634226 m E; Located at #23 Sandralin Bay, start of cul de sac, 7.2 m East of West curb.	Asphalt	-	Concrete	170



Photo 1: Pavement Core Sample at PC26-23

Appendix G
Summary Table and Photographs of Pavement Core Samples

English Place – Metz Street to Darwin Street



2026 Local Streets Package 26-R-09

English Pl - Metz St to Darwin St

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-25	UTM : 5520667 m N, 635075 m E; Located at the side of #81 Metz St, Westbound lane, 1.7 m South of North curb.	Asphalt	-	Concrete	160
PC26-26	UTM : 5520702 m N, 635138 m E; Located at #35 English Pl, Westbound lane, 2.5 m South of North curb.	Asphalt	-	Concrete	155
PC26-27	UTM : 5520733 m N, 635199 m E; Located at #19 English Pl, Westbound lane, 2.1 m South of North curb.	Asphalt	-	Concrete	155
PC26-28	UTM : 5520771 m N, 635264 m E; Located 6 m West of Darwin St, Westbound lane, 2.8 m South of North curb.	Asphalt	-	Concrete	150



Photo 1: Pavement Core Sample at PC26-25



Photo 2: Pavement Core Sample at PC26-26



Photo 3: Pavement Core Sample at PC26-27



Photo 4: Pavement Core Sample at PC26-28

Appendix H
Summary Table and Photographs of Pavement Core Samples

Plumtree Place – Meadwood Drive to South Limit



2026 Local Streets Package 26-R-09
Plumtree PI - Meadwood Dr to South Limit

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-24	UTM : 5521269 m N, 636376 m E; Located in the centre of cul de sac at #6 Plumtree PI, 7.2 m East of West curb.	Asphalt	-	Concrete	170



Photo 1: Pavement Core Sample at PC26-24

Appendix I
Summary Table and Photographs of Pavement Core Samples

Dells Crescent – Wales Avenue to Hazelwood Avenue



2026 Local Streets Package 26-R-09
Dells Cr - Wales Ave to Hazelwood Ave

Pavement Core No.	Pavement Core Location	Pavement Surface		Pavement Structure Material	
		Type	Thickness (mm)	Type	Thickness (mm)
PC26-15	UTM : 5521151 m N, 636734 m E; Located 5 m South of Hazelwood Ave, Northbound curb lane, 1.8 m West of East curb.	Asphalt	-	Concrete	160
PC26-16	UTM : 5521228 m N, 636691 m E; Located 6 m South of Greenwood Ave, Northbound curb lane, 1.6 m West of East curb.	Asphalt	-	Concrete	165
PC26-17	UTM : 5521151 m N, 636552 m E; Located 13 m South of Wales Ave, Northbound curb lane, 1.5 m West of East curb.	Asphalt	-	Concrete	160



Photo 1: Pavement Core Sample at PC26-15



Photo 2: Pavement Core Sample at PC26-16



Photo 3: Pavement Core Sample at PC26-17